To: | Ms. Karen Sherman, Town Planner |  |
| :--- | :--- |
|  | Town of Holliston |
|  | 703 Washington Street |
|  | Holliston, MA 01746 |

Cc: Jeffrey S. Dirk., PE, PTOE, FITE
Partner
Vanasse \& Associates, Inc

Date: $\quad$ September 16, 2020
Project Name: Geoffrey Park Residential Community
Project Number: Green No. 20082
Subject: Response to Transportation Peer Review Comments

From: Corinne Tobias, PE, PTOE Project Manager
Transportation Planning Group Green International Affiliates, Inc. 239 Littleton Road, Suite 3 Westford, MA 01886

This memorandum summarizes our revised responses to the Transportation Peer Review Comments prepared by Vanasse \& Associates, Inc. (VAI), dated August 24, 2020, and comments received at the Public Hearing on September 02, 2020.

Comment: "A letter should be provided by the Professional Engineer attesting to their oversight in preparing the document and providing their Massachusetts Professional Engineer Registration number and discipline."

Response: The document was prepared with the oversight of a Professional Engineer, Corinne Tobias, Civil Engineering Discipline, License Number 53048. This letter is signed by the same engineer.

## Traffic Volumes

Comment: "The study area should be expanded to include the intersections of Cedar Street at Turner Road and Ashland Street at Turner Road as all Project-related traffic will travel through one of these intersections to access the project site. We note that traffic volumes are provided for both of these intersections on the figures in the July 2020 TIAS."

Response: Cedar Street at Turner Road and Ashland Street at Road have been added to the crash data calculations.

Comment: "The data collection effort and establishment of the seasonal adjustment were completed in accordance with standard Traffic Engineering and Transportation Planning practices; however, the traffic count data needs to be adjusted following the guidance issued by MassDOT for Transportation Impact Assessments (TIAs) conducted during the COVID-19 pandemic and the Governor's phased "Reopening Massachusetts" strategy. 1 We would suggest that the Applicant's engineer obtain historic traffic count data for a location proximate to the study area and compare July 2020 traffic count data to historic traffic counts for July in order to develop an appropriate adjustment factor."

Response: Permanent count station data was gathered to determine the affect Covid-19 had on the traffic volumes in the area. Two permanent stations were used for comparison, AET08 located on I-90, east of

Cordaville to the north of the project, and 6125 located on 495 at the Franklin Town Line to the south of the project. The difference between the July 2019 and 2020 traffic volumes were compared and added to the July $1^{\text {st }}, 2020$ traffic counts from PDI. Count station AET08 had a difference of $31 \%$, station 6125 had a difference of $21 \%$. Thirty one percent (31\%) was added to the existing volumes at James Road and Turner Road however for a conservative estimate. The volumes and calc sheets are included as an attachment.

Comment: "The traffic count data was performed during the July 4th week, with the turning movement counts (TMCS) performed on July 1st (not July 8th as referenced in the July 2020 TIAS). Traffic data is not usually collected during holiday periods as traffic volumes and trip patterns are not typical of conditions that exist during the majority of the year. Updated traffic volume and travel speed data should be collected or adjustments made to the data that was collected with back-up calculations provided to substantiate the adjustments."

Response: July $4^{\text {th }}$ was on a Saturday this year and the Turning Movement Counts were collected on a Wednesday. As school was not in session at this time it is unlikely that the volumes would have been affected by the holiday. Seasonal adjustment data was evaluated.

Comment: "The traffic count data and speed measurements should be provided for all locations. The automatic traffic recorder (ATR) counts were not provided in the Appendix of the July 2020 TIAS."

Response: The ATR is attached.
Comment: "A 48-hour ATR should be performed on Turner Road in the vicinity of Indian Ridge Road on two consecutive weekdays that should include the collection of vehicle travel speed data to be used in evaluating sight distances at the Turner Road/Indian Ridge Road South intersection."

Response: The sight distance was updated for Turner Road and Indian Ridge Road speeds. As the ATR was not calculated at this intersection 10 mph was added to the posted speed limit. With this change safe stopping distance is no longer satisfied from the east. The sight distance to the east is limited by foliage and the horizontal curve in the roadway. This foliage is located on private property.

| Location | Sight Distance |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Available | Speed Limit (25 mph) |  | Speed Limit Plus 10 mph (35 mph) |  |
|  | Measured (ft) | Minimum Required (ft) | Desirable (ft) | Minimum Required (ft) | Desirable (ft) |
| Stopping Sight Distance |  |  |  |  |  |
| James Road (South) approaching from West | 350 | 155 | - | 250 | - |
| James Road (South) approaching from East | 200 | 155 | - | 250 | - |
| Intersection Sight Distance |  |  |  |  |  |
| James Road (South) approaching from West | 360 | 155 | 280 | 250 | 390 |
| James Road (South) approaching from East | 205 | 155 | 280 | 250 | 390 |

## Crash Analysis

Comment: "The motor vehicle crash analysis should be updated to use reconciled crash data for the most recent 5-year period as available from MassDOT (2013 through 2017) and expanded to include the additional study area intersections."

Response: The crash data was updated for 2013-2017, please see attachments for updated crash reports.

## No-Build Condition / Background traffic

Comment: "The Applicant's engineer should confirm that there are no roadway improvements by others that are planned to occur within or proximate to the study area that would impact traffic volumes or travel patterns within the study area."

Response: It is listed in section 3.1.2 that the town of Holliston was contacted, and Green was notified that no new developments were being planned.

## Trip Generation

Comment: "The traffic operations analysis should be revised to address the comments provided as a part of this review concerning the COVID-19 traffic volume adjustment, expansion of the study area and refinement of the trip distribution pattern. In addition, the peak-hour factors that are used in the analysis should be based on the measured values and not the default value of 0.92. "

Response: Please see attachments for updated Synchro reports that include the COVID -19 adjustments for the intersection of Turner Road at Indian Ridge Road. With this adjustment, the Level of Service for the intersection remains an " $A$ " under build conditions.

## Traffic Assignment

Comment: "The trip-distribution pattern for the Project should be reviewed and revised. The existing traffic pattern indicates that approximately 60 percent of the trips associated with the Project will be oriented to/from Route 126, with 45 percent of these trips using Elliot Street to/from the east and the remaining 15 percent (of the 60 percent) using Ashland Street."

Response: Trip distribution calculations were based off of existing traffic patterns and census data collected from CTPP(Census Transportation Planning Products). From the existing traffic patterns $72 \%$ are using Elliot Street, $20 \%$ Ashland Street and $9 \%$ are using Cedar Street. When looking at the census data for anticipated traffic patterns 29\% are using Elliot Street, 59\% Ashland Street and 13\% are using Cedar Street. The difference in these values were balanced to determine the trip distribution values.

## Site Distance

Comment: The location of the sight distance measurements that are presented in Table 7 should be clarified as the access to the Project is proposed to be an extension of Indian Ridge Road South and will not create an "intersection" for the purpose of sight distance measurements. Also, the paragraph preceding the sight distance table mentions speed measurements and posted speed limits along Cedar Street. Sight distance
measurements should be provided for the following intersections based on the measured 85th percentile vehicle travel speed on the major roadway or the posted speed limit, whichever is higher:

- Turner Road/Indian Ridge Road South
- Cedar Street/Turner Road
- $\quad$ Ashland Street/Turner Road

Response: Since access to the proposed project is an extension of Indian Ridge Road south table 7 shows the sight distance measurements at Indian Ridge Road South and Turner Road. Sight Distance calculations have been revised to show a higher speed for Turner Road based on comments received at the public hearing.

## Traffic Management

Comment: "The recommendations presented in the July 2020 TIAS should be reviewed, revised and expanded as necessary based on the additional analyses and refinements that have been suggested as a part of this review. The recommendations that were provided should reflect the Site Plans and the context of the Project as it relates to Indian Ridge Road South with regard to pedestrian safety and opportunities to implement traffic calming measures to reduce travel speeds.

## Response:

- W-2-2 advance warning signage on Turner Road should be provided in advance of the intersection with Indian Ridge Road South/James Road, prior to the horizontal curve that restricts sight distance, indicating the presence of an incoming intersection and need to watch for incoming vehicles.
- "No Parking" areas should be created between 85 Turner Road and 171 Turner Road on the west side of Turner Road, to match the existing "No Parking" area on the east side of Turner Road, improve sight distance, and to allow two-way traffic to travel through the narrow curves without impediment.
- The Speed Limit of Turner Road ( 25 mph ) should be posted in advance of the horizontal curves on Turner Road to encourage reduced speeds through the intersection.
- The proposed sidewalk on Indian Ridge Road South should be extended to the intersection of Turner Road and Indian Ridge Road South/James Road and terminated with an accessible crossing across James Road. This will allow safe pedestrian movement throughout the Indian Ridge Road South neighborhood and provide a continuous path for pedestrians utilizing the sidewalk. We do not recommend a crosswalk across Turner Road at this location due to the limited sight distance along Turner Road.
- A Stop bar should be striped for the James Road southbound approach for Turner Road, to reduce intersection creeping and provide a clear location for vehicles to stop.


## Response to Transportation Peer Review Comments

We believe that the responses and updates to the report address all the pertinent comments as well as items discussed during the public hearing. If there is a need to discuss any of these responses further, please feel free to contact me at (978)-923-0400.

Sincerely,

Green International Affiliates, Inc.
Pres re
Corinne Tobias, P.E.
Project Manager

## ATR DATA SUMMARY

|  |  |  | Wed, July 8,2020 |  |  | Thu, Jul 09, 2020 |  |  | Weekday Average |  |  |  |  |  | Weekday Average Hourly Volumes |  |  | Directional Distribution |  | K-Factor |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | NB | SB | Both | NB | SB | Both | NB | SB | Both |  |  |  | NB | SB | Both | NB | SB |  |
| 12:00 AM | to | 12:15 AM | 0 | 1 | 1 | 0 | 1 | 1 | 0.0 | 1.0 | 1.0 |  |  |  |  |  |  |  |  |  |
| 12:15 AM | to | 12:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |  |
| 12:30 AM | to | 12:45 AM | 0 | 0 | 0 | 0 | 1 | 1 | 0.0 | 0.5 | 0.5 |  |  |  |  |  |  |  |  |  |
| 12:45 AM | to | 01:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 12:00 AM | to | 01:00 AM | 0.0 | 1.5 | 1.5 | 0.000\% | 100.000\% | 0.003745 |
| 01:00 AM | to | 01:15 AM | 1 | 0 | 1 | 0 | 0 | 0 | 0.5 | 0.0 | 0.5 | 12:15 AM | to | 01:15 AM | 0.5 | 0.5 | 1.0 | 50.000\% | 50.000\% | 0.002497 |
| 01:15 AM | to | 01:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 12:30 AM | to | 01:30 AM | 0.5 | 0.5 | 1.0 | 50.000\% | 50.000\% | 0.002497 |
| 01:30 AM | to | 01:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 12:45 AM | to | 01:45 AM | 0.5 | 0.0 | 0.5 | 100.000\% | 0.000\% | 0.001248 |
| 01:45 AM | to | 02:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 01:00 AM | to | 02:00 AM | 0.5 | 0.0 | 0.5 | 100.000\% | 0.000\% | 0.001248 |
| 02:00 AM | to | 02:15 AM | 0 | 0 | 0 | 1 | 0 | 1 | 0.5 | 0.0 | 0.5 | 01:15 AM | to | 02:15 AM | 0.5 | 0.0 | 0.5 | 100.000\% | 0.000\% | 0.001248 |
| 02:15 AM | to | 02:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 01:30 AM | to | 02:30 AM | 0.5 | 0.0 | 0.5 | 100.000\% | 0.000\% | 0.001248 |
| 02:30 AM | to | 02:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 01:45 AM | to | 02:45 AM | 0.5 | 0.0 | 0.5 | 100.000\% | 0.000\% | 0.001248 |
| 02:45 AM | to | 03:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 02:00 AM | to | 03:00 AM | 0.5 | 0.0 | 0.5 | 100.000\% | 0.000\% | 0.001248 |
| 03:00 AM | to | 03:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 02:15 AM | to | 03:15 AM | 0.0 | 0.0 | 0.0 | \#DIV/0! | \#DIV/0! | 0.000000 |
| 03:15 AM | to | 03:30 AM | 0 | 1 | 1 | 0 | 1 | 1 | 0.0 | 1.0 | 1.0 | 02:30 AM | to | 03:30 AM | 0.0 | 1.0 | 1.0 | 0.000\% | 100.000\% | 0.002497 |
| 03:30 AM | to | 03:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 02:45 AM | to | 03:45 AM | 0.0 | 1.0 | 1.0 | 0.000\% | 100.000\% | 0.002497 |
| 03:45 AM | to | 04:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 03:00 AM | to | 04:00 AM | 0.0 | 1.0 | 1.0 | 0.000\% | 100.000\% | 0.002497 |
| 04:00 AM | to | 04:15 AM | 0 | 0 | 0 | 1 | 1 | 2 | 0.5 | 0.5 | 1.0 | 03:15 AM | to | 04:15 AM | 0.5 | 1.5 | 2.0 | 25.000\% | 75.000\% | 0.004994 |
| 04:15 AM | to | 04:30 AM | 1 | 1 | 2 | 0 | 0 | 0 | 0.5 | 0.5 | 1.0 | 03:30 AM | to | 04:30 AM | 1.0 | 1.0 | 2.0 | 50.000\% | 50.000\% | 0.004994 |
| 04:30 AM | to | 04:45 AM | 1 | 0 | 1 | 1 | 1 | 2 | 1.0 | 0.5 | 1.5 | 03:45 AM | to | 04:45 AM | 2.0 | 1.5 | 3.5 | 57.143\% | 42.857\% | 0.008739 |
| 04:45 AM | to | 05:00 AM | 1 | 0 | 1 | 1 | 0 | 1 | 1.0 | 0.0 | 1.0 | 04:00 AM | to | 05:00 AM | 3.0 | 1.5 | 4.5 | 66.667\% | 33.333\% | 0.011236 |
| 05:00 AM | to | 05:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0.0 | 0.0 | 0.0 | 04:15 AM | to | 05:15 AM | 2.5 | 1.0 | 3.5 | 71.429\% | 28.571\% | 0.008739 |
| 05:15 AM | to | 05:30 AM | 0 | 3 | 3 | 0 | 2 | 2 | 0.0 | 2.5 | 2.5 | 04:30 AM | to | 05:30 AM | 2.0 | 3.0 | 5.0 | 40.000\% | 60.000\% | 0.012484 |
| 05:30 AM | to | 05:45 AM | 3 | 0 | 3 | 2 | 2 | 4 | 2.5 | 1.0 | 3.5 | 04:45 AM | to | 05:45 AM | 3.5 | 3.5 | 7.0 | 50.000\% | 50.000\% | 0.017478 |
| 05:45 AM | to | 06:00 AM | 0 | 1 | 1 | 1 | 0 | 1 | 0.5 | 0.5 | 1.0 | 05:00 AM | to | 06:00 AM | 3.0 | 4.0 | 7.0 | 42.857\% | 57.143\% | 0.017478 |
| 06:00 AM | to | 06:15 AM | 2 | 0 | 2 | 3 | 0 | 3 | 2.5 | 0.0 | 2.5 | 05:15 AM | to | 06:15 AM | 5.5 | 4.0 | 9.5 | 57.895\% | 42.105\% | 0.023720 |
| 06:15 AM | to | 06:30 AM | 3 | 1 | 4 | 4 | 2 | 6 | 3.5 | 1.5 | 5.0 | 05:30 AM | to | 06:30 AM | 9.0 | 3.0 | 12.0 | 75.000\% | 25.000\% | 0.029963 |
| 06:30 AM | to | 06:45 AM | 2 | 3 | 5 | 1 | 2 | 3 | 1.5 | 2.5 | 4.0 | 05:45 AM | to | 06:45 AM | 8.0 | 4.5 | 12.5 | 64.000\% | 36.000\% | 0.031211 |
| 06:45 AM | to | 07:00 AM | 3 | 0 | 3 | 2 | 0 | 2 | 2.5 | 0.0 | 2.5 | 06:00 AM | to | 07:00 AM | 10.0 | 4.0 | 14.0 | 71.429\% | 28.571\% | 0.034956 |
| 07:00 AM | to | 07:15 AM | 2 | 1 | 3 | 1 | 3 | 4 | 1.5 | 2.0 | 3.5 | 06:15 AM | to | 07:15 AM | 9.0 | 6.0 | 15.0 | 60.000\% | 40.000\% | 0.037453 |
| 07:15 AM | to | 07:30 AM | 3 | 0 | 3 | 1 | 1 | 2 | 2.0 | 0.5 | 2.5 | 06:30 AM | to | 07:30 AM | 7.5 | 5.0 | 12.5 | 60.000\% | 40.000\% | 0.031211 |
| 07:30 AM | to | 07:45 AM | 5 | 5 | 10 | 2 | 0 | 2 | 3.5 | 2.5 | 6.0 | 06:45 AM | to | 07:45 AM | 9.5 | 5.0 | 14.5 | 65.517\% | 34.483\% | 0.036205 |
| 07:45 AM | to | 08:00 AM | 5 | 5 | 10 | 1 | 2 | 3 | 3.0 | 3.5 | 6.5 | 07:00 AM | to | 08:00 AM | 10.0 | 8.5 | 18.5 | 54.054\% | 45.946\% | 0.046192 |
| 08:00 AM | to | 08:15 AM | 4 | 1 | 5 | 2 | 2 | 4 | 3.0 | 1.5 | 4.5 | 07:15 AM | to | 08:15 AM | 11.5 | 8.0 | 19.5 | 58.974\% | 41.026\% | 0.048689 |
| 08:15 AM | to | 08:30 AM | 4 | 2 | 6 | 0 | 0 | 0 | 2.0 | 1.0 | 3.0 | 07:30 AM | to | 08:30 AM | 11.5 | 8.5 | 20.0 | 57.500\% | 42.500\% | 0.049938 |
| 08:30 AM | to | 08:45 AM | 8 | 4 | 12 | 0 | 0 | 0 | 4.0 | 2.0 | 6.0 | 07:45 AM | to | 08:45 AM | 12.0 | 8.0 | 20.0 | 60.000\% | 40.000\% | 0.049938 |
| 08:45 AM | to | 09:00 AM | 7 | 3 | 10 | 3 | 2 | 5 | 5.0 | 2.5 | 7.5 | 08:00 AM | to | 09:00 AM | 14.0 | 7.0 | 21.0 | 66.667\% | 33.333\% | 0.052434 |
| 09:00 AM | to | 09:15 AM | 1 | 2 | 3 | 0 | 0 | 0 | 0.5 | 1.0 | 1.5 | 08:15 AM | to | 09:15 AM | 11.5 | 6.5 | 18.0 | 63.889\% | 36.111\% | 0.044944 |
| 09:15 AM | to | 09:30 AM | 3 | 4 | 7 | 2 | 1 | 3 | 2.5 | 2.5 | 5.0 | 08:30 AM | to | 09:30 AM | 12.0 | 8.0 | 20.0 | 60.000\% | 40.000\% | 0.049938 |
| 09:30 AM | to | 09:45 AM | 2 | 5 | 7 | 1 | 0 | 1 | 1.5 | 2.5 | 4.0 | 08:45 AM | to | 09:45 AM | 9.5 | 8.5 | 18.0 | 52.778\% | 47.222\% | 0.044944 |
| 09:45 AM | to | 10:00 AM | 2 | 2 | 4 | 0 | 3 | 3 | 1.0 | 2.5 | 3.5 | 09:00 AM | to | 10:00 AM | 5.5 | 8.5 | 14.0 | 39.286\% | 60.714\% | 0.034956 |
| 10:00 AM | to | 10:15 AM | 1 | 0 | 1 | 2 | 2 | 4 | 1.5 | 1.0 | 2.5 | 09:15 AM | to | 10:15 AM | 6.5 | 8.5 | 15.0 | 43.333\% | 56.667\% | 0.037453 |
| 10:15 AM | to | 10:30 AM | 5 | 2 | 7 | 0 | 0 | 0 | 2.5 | 1.0 | 3.5 | 09:30 AM | to | 10:30 AM | 6.5 | 7.0 | 13.5 | 48.148\% | 51.852\% | 0.033708 |
| 10:30 AM | to | 10:45 AM | 5 | 0 | 5 | 1 | 1 | 2 | 3.0 | 0.5 | 3.5 | 09:45 AM | to | 10:45 AM | 8.0 | 5.0 | 13.0 | 61.538\% | 38.462\% | 0.032459 |
| 10:45 AM | to | 11:00 AM | 6 | 2 | 8 | 3 | 2 | 5 | 4.5 | 2.0 | 6.5 | 10:00 AM | to | 11:00 AM | 11.5 | 4.5 | 16.0 | 71.875\% | 28.125\% | 0.039950 |
| 11:00 AM | to | 11:15 AM | 2 | 2 | 4 | 1 | 2 | 3 | 1.5 | 2.0 | 3.5 | 10:15 AM | to | 11:15 AM | 11.5 | 5.5 | 17.0 | 67.647\% | 32.353\% | 0.042447 |
| 11:15 AM | to | 11:30 AM | 3 | 5 | 8 | 2 | 4 | 6 | 2.5 | 4.5 | 7.0 | 10:30 AM | to | 11:30 AM | 11.5 | 9.0 | 20.5 | 56.098\% |  | 0.051186 |
| 11:30 AM | to | 11:45 AM | 2 | 1 | 3 | 2 | 2 | 4 | 2.0 | 1.5 | 3.5 | 10:45 AM | to | 11:45 AM | 10.5 | 10.0 | 20.5 | 51.220\% | 48.780\% | 0.051186 |
| 11:45 AM | to | 12:00 PM | 2 | 2 | 4 | 4 | 2 | 6 | 3.0 | 2.0 | 5.0 | 11:00 AM | to | 12:00 PM | 9.0 | 10.0 | 19.0 | 47.368\% | 52.632\% | 0.047441 |
| 12:00 PM | to | 12:15 PM | 0 | 3 | 3 | 3 | 1 | 4 | 1.5 | 2.0 | 3.5 | 11:15 AM | to | 12:15 PM | 9.0 | 10.0 | 19.0 | 47.368\% | 52.632\% | 0.047441 |
| 12:15 PM | to | 12:30 PM | 1 | 1 | 2 | 2 | 1 | 3 | 1.5 | 1.0 | 2.5 | 11:30 AM | to | 12:30 PM | 8.0 | 6.5 | 14.5 | 55.172\% | 44.828\% | 0.036205 |
| 12:30 PM | to | 12:45 PM | 1 | 4 | 5 | 2 | 2 | 4 | 1.5 | 3.0 | 4.5 | 11:45 AM | to | 12:45 PM | 7.5 | 8.0 | 15.5 | 48.387\% | 51.613\% | 0.038702 |
| 12:45 PM | to | 01:00 PM | 5 | 2 | 7 | 2 | 1 | 3 | 3.5 | 1.5 | 5.0 | 12:00 PM | to | 01:00 PM | 8.0 | 7.5 | 15.5 | 51.613\% | 48.387\% | 0.038702 |
| 01:00 PM | to | 01:15 PM | 6 | 3 | 9 | 4 | 2 | 6 | 5.0 | 2.5 | 7.5 | 12:15 PM | to | 01:15 PM | 11.5 | 8.0 | 19.5 | 58.974\% | 41.026\% | 0.048689 |
| 01:15 PM | to | 01:30 PM | 4 | 2 | 6 | 1 | 3 | 4 | 2.5 | 2.5 | 5.0 | 12:30 PM | to | 01:30 PM | 12.5 | 9.5 | 22.0 | 56.818\% | 43.182\% | 0.054931 |
| 01:30 PM | to | 01:45 PM | 2 | 1 | 3 | 2 | 5 | 7 | 2.0 | 3.0 | 5.0 | 12:45 PM | to | 01:45 PM | 13.0 | 9.5 | 22.5 | 57.778\% | 42.222\% | 0.056180 |
| 01:45 PM | to | 02:00 PM | 2 | 2 | 4 | 5 | 2 | 7 | 3.5 | 2.0 | 5.5 | 01:00 PM | to | 02:00 PM | 13.0 | 10.0 | 23.0 | 56.522\% | 43.478\% | 0.057428 |
| 02:00 PM | to | 02:15 PM | 1 | 6 | 7 | 2 | 0 | 2 | 1.5 | 3.0 | 4.5 | 01:15 PM | to | 02:15 PM | 9.5 | 10.5 | 20.0 | 47.500\% | 52.500\% | 0.049938 |
| 02:15 PM | to | 02:30 PM | 1 | 4 | 5 | 4 | 6 | 10 | 2.5 | 5.0 | 7.5 | 01:30 PM | to | 02:30 PM | 9.5 | 13.0 | 22.5 | 42.222\% | 57.778\% | 0.056180 |
| 02:30 PM | to | 02:45 PM | 1 | 3 | 4 | 5 | 1 | 6 | 3.0 | 2.0 | 5.0 | 01:45 PM | to | 02:45 PM | 10.5 | 12.0 | 22.5 | 46.667\% | 53.333\% | 0.056180 |
| 02:45 PM | to | 03:00 PM | 4 | 3 | 7 | 5 | 4 | 9 | 4.5 | 3.5 | 8.0 | 02:00 PM | to | 03:00 PM | 11.5 | 13.5 | 25.0 | 46.000\% | 54.000\% | 0.062422 |



## CRASH RATE

## INTERSECTION CRASH RATE WORKSHEET



Comments : The average crash rate for an unsignalized intersection in District is 0.57
Project Title \& Date:
Geoffery Park 20082

## INTERSECTION CRASH RATE WORKSHEET



Comments : The average crash rate for an unsignalized intersection in District is 0.57

## INTERSECTION CRASH RATE WORKSHEET

| TOWN : DISTRICT: | Holliston |  |  | COUNT DATE : <br> SIGNALIZED <br> DATA ~ | 2/26/2020 |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3 | UNSIGNALIZED : | V |  |  |
|  |  |  | EC |  |  |
| MAJOR STREET : |  | Main Street |  |  |  |
| MINOR STREET(S) |  | Shrewsbury Street |  |  |  |



|  | PEAK HOUR VOLUMES |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| APPROACH | 1 | 2 | 3 | 4 | 5 | Total Peak Hourly |
| DIRECTION : | NB | SB | EB | WB | SWB | Approach Volume |
| PEAK HOURLY VOLUMES (PM) | 87 | 40 | 367 | 496 | 0 | 990 |
| " K " FACTOR : | 0.08 |  | $\begin{aligned} & \text { ION A } \\ & \hline \text { PRRO } \end{aligned}$ | $)=T$ 'LUM | DAILY | 12,375 |



CRASH RATE CALCULATION :

### 0.44

RATE $=\frac{(\mathrm{A} * 1,000,000)}{(\mathrm{V} * 365)}$
Comments : The average crash rate for an unsignalized intersection in District 5 is 0.57
Project Title \& Date:
Geoffery Park 20082

## INTERSECTION CRASH RATE WORKSHEET



Comments : The average crash rate for an unsignalized intersection in District is 0.57
Project Title \& Date: Geoffrey Park 20082

## INTERSECTION CRASH RATE WORKSHEET



Comments : The average crash rate for an unsignalized intersection in District is 0.57
Project Title \& Date:
Geoffery Park 20082

## CORONA VIRUS VOLUME COMPARISONS

2019 and 2020 Traffic counts from AET08 on Mass. Turnpike East of Cordaville

| Date | 2019 Volume | Date | 2020 Volume | Difference |
| :---: | ---: | ---: | ---: | ---: |
| $7 / 1 / 2019$ | 105563 | $7 / 1 / 2020$ | 73210 | $31 \%$ |
| $7 / 2 / 2019$ | 105017 | $7 / 2 / 2020$ | 83300 | $21 \%$ |
| $7 / 3 / 2019$ | 106142 | $7 / 3 / 2020$ | 67685 | $36 \%$ |
| $7 / 4 / 2019$ | 69418 | $7 / 4 / 2020$ | 48483 | $30 \%$ |
| $7 / 5 / 2019$ | 90134 | $7 / 5 / 2020$ | 64076 | $29 \%$ |
| $7 / 6 / 2019$ | 84654 | $7 / 6 / 2020$ | 71464 | $16 \%$ |
| $7 / 7 / 2019$ | 97124 | $7 / 7 / 2020$ | 71677 | $26 \%$ |
| $7 / 8 / 2019$ | 105386 | $7 / 8 / 2020$ | 70383 | $33 \%$ |
| $7 / 9 / 2019$ | 104237 | $7 / 9 / 2020$ | 74393 | $29 \%$ |
| $7 / 10 / 2019$ | 107614 | $7 / 10 / 2020$ | 75562 | $30 \%$ |
| $7 / 11 / 2019$ | 113328 | $7 / 11 / 2020$ | 63587 | $44 \%$ |
| $7 / 12 / 2019$ | 115899 | $7 / 12 / 2020$ | 62530 | $46 \%$ |
| $7 / 13 / 2019$ | 101600 | $7 / 13 / 2020$ | 69112 | $32 \%$ |
| $7 / 14 / 2019$ | 100543 | $7 / 14 / 2020$ | 70405 | $30 \%$ |
| $7 / 15 / 2019$ | 108733 | $7 / 15 / 2020$ | 73266 | $33 \%$ |
| $7 / 16 / 2019$ | 0 | $7 / 16 / 2020$ | 76900 |  |
| $7 / 17 / 2019$ | 0 | $7 / 17 / 2020$ | 81473 |  |
| $7 / 18 / 2019$ | 115489 | $7 / 18 / 2020$ | 70039 | $39 \%$ |
| $7 / 19 / 2019$ | 117940 | $7 / 19 / 2020$ | 63858 | $46 \%$ |
| $7 / 20 / 2019$ | 93672 | $7 / 20 / 2020$ | 71497 | $24 \%$ |
| $7 / 21 / 2019$ | 97052 | $7 / 21 / 2020$ | 72618 | $25 \%$ |
| $7 / 22 / 2019$ | 105869 | $7 / 22 / 2020$ | 73063 | $31 \%$ |
| $7 / 23 / 2019$ | 102394 | $7 / 23 / 2020$ | 75443 | $26 \%$ |
| $7 / 24 / 2019$ | 111605 | $7 / 24 / 2020$ | 85461 | $23 \%$ |
| $7 / 25 / 2019$ | 117202 | $7 / 25 / 2020$ | 73071 |  |
| $7 / 26 / 2019$ | 121135 | $7 / 26 / 2020$ | 68144 | $38 \%$ |
| $7 / 27 / 2019$ | 100017 | $7 / 27 / 2020$ | 72351 | $44 \%$ |
| $7 / 28 / 2019$ | 100373 | $7 / 28 / 2020$ | 72181 | $28 \%$ |
| $7 / 29 / 2019$ | 108173 | $7 / 29 / 2020$ | 76131 | 289 |
| $7 / 30 / 2019$ | 107333 | $7 / 30 / 2020$ | 78934 |  |
| $7 / 31 / 2019$ | 0 | $7 / 31 / 2020$ | 88944 |  |
|  |  |  |  | $26 \%$ |

2019 and 2020 Traffic counts from 6125 Interstate 495 at Franklin Town Line

| Date | 2019 Volume | Date | 2020 Volume | Difference |
| :---: | :---: | :---: | :---: | :---: |
| 7/1/2019 | 97674 | 7/1/2020 | 76863 | 21\% |
| 7/2/2019 | 102610 | 7/2/2020 | 0 |  |
| 7/3/2019 | 103349 | 7/3/2020 | 67142 | 35\% |
| 7/4/2019 | 56071 | 7/4/2020 | 50934 | 9\% |
| 7/5/2019 | 79197 | 7/5/2020 | 58765 | 26\% |
| 7/6/2019 | 68733 | 7/6/2020 | 74218 | -8\% |
| 7/7/2019 | 74086 | 7/7/2020 | 74183 | 0\% |
| 7/8/2019 | 100091 | 7/8/2020 | 76306 | 24\% |
| 7/9/2019 | 106395 | 7/9/2020 | 79929 | 25\% |
| 7/10/2019 | 103771 | 7/10/2020 | 79738 | 23\% |
| 7/11/2019 | 108274 | 7/11/2020 | 66062 | 39\% |
| 7/12/2019 | 111604 | 7/12/2020 | 61913 | 45\% |
| 7/13/2019 | 94329 | 7/13/2020 | 72920 | 23\% |
| 7/14/2019 | 80753 | 7/14/2020 | 74613 | 8\% |
| 7/15/2019 | 103392 | 7/15/2020 | 79348 | 23\% |
| 7/16/2019 | 104269 | 7/16/2020 | 81909 | 21\% |
| 7/17/2019 | 98771 | 7/17/2020 | 85496 | 13\% |
| 7/18/2019 | 105531 | 7/18/2020 | 72912 | 31\% |
| 7/19/2019 | 0 | 7/19/2020 | 62758 |  |
| 7/20/2019 | 83141 | 7/20/2020 | 77144 | 7\% |
| 7/21/2019 | 72727 | 7/21/2020 | 78292 | -8\% |
| 7/22/2019 | 107603 | 7/22/2020 | 79452 | 26\% |
| 7/23/2019 | 94447 | 7/23/2020 | 80329 | 15\% |
| 7/24/2019 | 105919 | 7/24/2020 | 88496 | 16\% |
| 7/25/2019 | 106186 | 7/25/2020 | 74851 | 30\% |
| 7/26/2019 | 110650 | 7/26/2020 | 64631 | 42\% |
| 7/27/2019 | 90669 | 7/27/2020 | 77350 | 15\% |
| 7/28/2019 | 77538 | 7/28/2020 | 76899 | 1\% |
| 7/29/2019 | 101149 | 7/29/2020 | 81001 | 20\% |
| 7/30/2019 | 102960 | 7/30/2020 | 82783 | 20\% |
| 7/31/2019 | 103332 | 7/31/2020 | 90270 | 13\% |



| Seasonal Factor | $0.00 \%$ |
| :---: | ---: |
| Annual Growth Rate | $0.00 \%$ |
| Number of years | 0 |

UPDATED FIGURES

## AM/PM EXISTING



## 2027 AM/PM NO BUILD



## SITE GENERATED TRIPS



|  | ENTER | EXIT | TOTAL |
| :---: | :---: | :---: | :---: |
| SITE-GENERATED <br> TRIPS | 5 | 14 | 19 |


|  | ENTER | EXIT | TOTAL |
| :---: | :---: | :---: | :---: |
| SITE-GENERATED <br> TRIPS | 15 | 9 | 24 |

$$
2027 \text { AM/PM BUILD }
$$



## UPDATED SYNCHRO ANALYSIS

3: Turner Road \& Indian Ridge Road (S)


| Major/Minor M | Major1 |  |  |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 15 | 0 | - | 0 | 32 | 10 |
| Stage 1 | - | - | - | - | 10 | - |
| Stage 2 | - | - | - | - | 22 | - |
| Critical Hdwy | 4.1 | - | - | - | 6.46 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.46 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.46 | - |
| Follow-up Hdwy | 2.2 | - | - | - | 3.554 | 3.3 |
| Pot Cap-1 Maneuver | 1616 | - | - | - | 972 | 1077 |
| Stage 1 | - | - | - | - | 1003 | - |
| Stage 2 | - | - | - | - | 990 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1616 | - | - | - | 972 | 1077 |
| Mov Cap-2 Maneuver | - | - | - | - | 972 | - |
| Stage 1 | - | - | - | - | 1003 | - |
| Stage 2 | - | - | - | - | 990 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  |  |  | SB |  |
| HCM Control Delay, s | 0 |  | 0 |  | 8.8 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL EBT WBT WBR SBLn1 |  |  |  |  |
| Capacity (veh/h) |  | 1616 | - | - | - | 978 |
| HCM Lane V/C Ratio |  | - | - | - | - | 0.02 |
| HCM Control Delay (s) |  | 0 | - | - | - | 8.8 |
| HCM Lane LOS |  | A | - | - | - | A |
| HCM 95th \%tile Q(veh) |  | 0 | - | - |  | 0.1 |

3: Turner Road \& Indian Ridge Road (S)



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | A |  | 1 |  |
| Traffic Vol, veh/h | 0 | 18 | 4 | 9 | 15 | 1 |
| Future Vol, veh/h | 0 | 18 | 4 | 9 | 15 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, $\#$ | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 0 | 8 | 14 | 0 | 6 | 0 |
| Mvmt Flow | 0 | 23 | 5 | 12 | 19 | 1 |




| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 45 | 0 | - | 0 | 45 | 39 |
| Stage 1 | - | - | - |  | 39 | - |
| Stage 2 | - | - | - | - | 6 |  |
| Critical Hdwy | 4.1 | - | - | - | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.4 | - |
| Critical Hdwy Stg 2 | - | - | - |  | 5.4 | - |
| Follow-up Hdwy | 2.2 | - | - | - | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | 1576 | - | - | - | 970 | 1038 |
| Stage 1 | - | - | - |  | 989 | - |
| Stage 2 | - | - | - |  | 1022 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1576 | - | - | - | 970 | 1038 |
| Mov Cap-2 Maneuver | - | - | - | - | 970 | - |
| Stage 1 | - | - | - |  | 989 | - |
| Stage 2 | - | - | - |  | 1022 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 0 |  | 0 |  | 8.7 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBL 1 |  |
| Capacity (veh/h) |  | 1576 | - | - | - | 970 |
| HCM Lane V/C Ratio |  | - | - | - | - | 0.005 |
| HCM Control Delay (s) |  | 0 | - | - | - | 8.7 |
| HCM Lane LOS |  | A | - | - | - | A |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0 |



| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 20 | 0 | - | 0 | 42 | 13 |
| Stage 1 | - | - | - |  | 13 | - |
| Stage 2 | - | - | - |  | 29 |  |
| Critical Hdwy | 4.1 | - | - |  | 6.46 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - |  | 5.46 | - |
| Critical Hdwy Stg 2 | - | - | - |  | 5.46 | - |
| Follow-up Hdwy | 2.2 | - | - |  | 3.554 | 3.3 |
| Pot Cap-1 Maneuver | 1609 | - | - |  | 959 | 1073 |
| Stage 1 | - | - | - |  | 1000 | - |
| Stage 2 | - | - | - |  | 983 | - |
| Platoon blocked, \% |  | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1609 | - | - |  | 957 | 1073 |
| Mov Cap-2 Maneuver | - | - | - |  | 957 | - |
| Stage 1 | - | - | - |  | 998 | - |
| Stage 2 | - | - | - |  | 983 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 0.7 |  | 0 |  | 8.8 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT WBT WBRSBLn1 |  |  |  |
| Capacity (veh/h) |  | 1609 | - |  | - | 985 |
| HCM Lane V/C Ratio |  | 0.002 | - |  | - | 0.039 |
| HCM Control Delay (s) |  | 7.2 | 0 |  | - | 8.8 |
| HCM Lane LOS |  | A | A | - | - | A |
| HCM 95th \%tile Q(veh) |  | 0 | - |  | - | 0.1 |




