To: Ms. Karen Sherman, Town Planner From:

Town of Holliston 703 Washington Street Holliston, MA 01746 Project Manager
Transportation Planning Group
Green International Affiliates, Inc.
229 Littleton Pood Suite 3

239 Littleton Road, Suite 3 Westford, MA 01886

Corinne Tobias, PE, PTOE

Cc: Jeffrey S. Dirk., PE, PTOE, FITE

Partner

Vanasse & Associates, Inc

Date: September 16, 2020

**Project Name:** Geoffrey Park Residential Community

Project Number: Green No. 20082

**Subject:** Response to Transportation Peer Review Comments

This memorandum summarizes our revised responses to the Transportation Peer Review Comments prepared by Vanasse & Associates, Inc. (VAI), dated August 24, 2020, and comments received at the Public Hearing on September 02, 2020.

<u>Comment:</u> "A letter should be provided by the Professional Engineer attesting to their oversight in preparing the document and providing their Massachusetts Professional Engineer Registration number and discipline."

<u>Response</u>: The document was prepared with the oversight of a Professional Engineer, Corinne Tobias, Civil Engineering Discipline, License Number 53048. This letter is signed by the same engineer.

#### **Traffic Volumes**

<u>Comment:</u> "The study area should be expanded to include the intersections of Cedar Street at Turner Road and Ashland Street at Turner Road as all Project-related traffic will travel through one of these intersections to access the project site. We note that traffic volumes are provided for both of these intersections on the figures in the July 2020 TIAS."

<u>Response</u>: Cedar Street at Turner Road and Ashland Street at Road have been added to the crash data calculations.

<u>Comment:</u> "The data collection effort and establishment of the seasonal adjustment were completed in accordance with standard Traffic Engineering and Transportation Planning practices; however, the traffic count data needs to be adjusted following the guidance issued by MassDOT for Transportation Impact Assessments (TIAs) conducted during the COVID-19 pandemic and the Governor's phased "Reopening Massachusetts" strategy.1 We would suggest that the Applicant's engineer obtain historic traffic count data for a location proximate to the study area and compare July 2020 traffic count data to historic traffic counts for July in order to develop an appropriate adjustment factor."

<u>Response:</u> Permanent count station data was gathered to determine the affect Covid-19 had on the traffic volumes in the area. Two permanent stations were used for comparison, AET08 located on I-90, east of

Cordaville to the north of the project, and 6125 located on 495 at the Franklin Town Line to the south of the project. The difference between the July 2019 and 2020 traffic volumes were compared and added to the July 1<sup>st</sup>, 2020 traffic counts from PDI. Count station AET08 had a difference of 31%, station 6125 had a difference of 21%. Thirty one percent (31%) was added to the existing volumes at James Road and Turner Road however for a conservative estimate. The volumes and calc sheets are included as an attachment.

Comment: "The traffic count data was performed during the July 4th week, with the turning movement counts (TMCs) performed on July 1st (not July 8th as referenced in the July 2020 TIAS). Traffic data is not usually collected during holiday periods as traffic volumes and trip patterns are not typical of conditions that exist during the majority of the year. Updated traffic volume and travel speed data should be collected or adjustments made to the data that was collected with back-up calculations provided to substantiate the adjustments."

<u>Response:</u> July 4<sup>th</sup> was on a Saturday this year and the Turning Movement Counts were collected on a Wednesday. As school was not in session at this time it is unlikely that the volumes would have been affected by the holiday. Seasonal adjustment data was evaluated.

<u>Comment:</u> "The traffic count data and speed measurements should be provided for all locations. The automatic traffic recorder (ATR) counts were not provided in the Appendix of the July 2020 TIAS."

Response: The ATR is attached.

<u>Comment:</u> "A 48-hour ATR should be performed on Turner Road in the vicinity of Indian Ridge Road on two consecutive weekdays that should include the collection of vehicle travel speed data to be used in evaluating sight distances at the Turner Road/Indian Ridge Road South intersection."

<u>Response</u>: The sight distance was updated for Turner Road and Indian Ridge Road speeds. As the ATR was not calculated at this intersection 10mph was added to the posted speed limit. With this change safe stopping distance is no longer satisfied from the east. The sight distance to the east is limited by foliage and the horizontal curve in the roadway. This foliage is located on private property.

			Sight Distance		
Location	Available	Speed Limi	Speed Limit Plus 10mph (35 mph)		
	Measured (ft)	Minimum Required (ft)	Desirable (ft)	Minimum Required (ft)	Desirable (ft)
Stopping Sight Distance					
James Road (South) approaching from West	350	155	-	250	-
James Road (South) approaching from East	200	155	-	250	-
Intersection Sight Distance					
James Road (South) approaching from West	360	155	280	250	390
James Road (South) approaching from East	205	155	280	250	390

#### **Crash Analysis**

<u>Comment:</u> "The motor vehicle crash analysis should be updated to use reconciled crash data for the most recent 5-year period as available from MassDOT (2013 through 2017) and expanded to include the additional study area intersections."

Response: The crash data was updated for 2013-2017, please see attachments for updated crash reports.

#### No-Build Condition / Background traffic

<u>Comment:</u> "The Applicant's engineer should confirm that there are no roadway improvements by others that are planned to occur within or proximate to the study area that would impact traffic volumes or travel patterns within the study area."

Response: It is listed in section 3.1.2 that the town of Holliston was contacted, and Green was notified that no new developments were being planned.

#### **Trip Generation**

<u>Comment</u>: "The traffic operations analysis should be revised to address the comments provided as a part of this review concerning the COVID-19 traffic volume adjustment, expansion of the study area and refinement of the trip distribution pattern. In addition, the peak-hour factors that are used in the analysis should be based on the measured values and not the default value of 0.92. "

<u>Response:</u> Please see attachments for updated Synchro reports that include the COVID -19 adjustments for the intersection of Turner Road at Indian Ridge Road. With this adjustment, the Level of Service for the intersection remains an "A" under build conditions.

#### **Traffic Assignment**

<u>Comment</u>: "The trip-distribution pattern for the Project should be reviewed and revised. The existing traffic pattern indicates that approximately 60 percent of the trips associated with the Project will be oriented to/from Route 126, with 45 percent of these trips using Elliot Street to/from the east and the remaining 15 percent (of the 60 percent) using Ashland Street."

<u>Response</u>: Trip distribution calculations were based off of existing traffic patterns and census data collected from CTPP(Census Transportation Planning Products). From the existing traffic patterns 72% are using Elliot Street, 20% Ashland Street and 9% are using Cedar Street. When looking at the census data for anticipated traffic patterns 29% are using Elliot Street, 59% Ashland Street and 13% are using Cedar Street. The difference in these values were balanced to determine the trip distribution values.

#### **Site Distance**

<u>Comment</u>: The location of the sight distance measurements that are presented in Table 7 should be clarified as the access to the Project is proposed to be an extension of Indian Ridge Road South and will not create an "intersection" for the purpose of sight distance measurements. Also, the paragraph preceding the sight distance table mentions speed measurements and posted speed limits along Cedar Street. Sight distance

measurements should be provided for the following intersections based on the measured 85th percentile vehicle travel speed on the major roadway or the posted speed limit, whichever is higher:

- Turner Road/Indian Ridge Road South
- Cedar Street/Turner Road
- Ashland Street/Turner Road

<u>Response</u>: Since access to the proposed project is an extension of Indian Ridge Road south table 7 shows the sight distance measurements at Indian Ridge Road South and Turner Road. Sight Distance calculations have been revised to show a higher speed for Turner Road based on comments received at the public hearing.

#### **Traffic Management**

<u>Comment</u>: "The recommendations presented in the July 2020 TIAS should be reviewed, revised and expanded as necessary based on the additional analyses and refinements that have been suggested as a part of this review. The recommendations that were provided should reflect the Site Plans and the context of the Project as it relates to Indian Ridge Road South with regard to pedestrian safety and opportunities to implement traffic calming measures to reduce travel speeds.

#### Response:

- W-2-2 advance warning signage on Turner Road should be provided in advance of the intersection with Indian Ridge Road South/James Road, prior to the horizontal curve that restricts sight distance, indicating the presence of an incoming intersection and need to watch for incoming vehicles.
- "No Parking" areas should be created between 85 Turner Road and 171 Turner Road on the west side of Turner Road, to match the existing "No Parking" area on the east side of Turner Road, improve sight distance, and to allow two-way traffic to travel through the narrow curves without impediment.
- The Speed Limit of Turner Road (25mph) should be posted in advance of the horizontal curves on Turner Road to encourage reduced speeds through the intersection.
- The proposed sidewalk on Indian Ridge Road South should be extended to the intersection of Turner Road and Indian Ridge Road South/James Road and terminated with an accessible crossing across James Road. This will allow safe pedestrian movement throughout the Indian Ridge Road South neighborhood and provide a continuous path for pedestrians utilizing the sidewalk. We do not recommend a crosswalk across Turner Road at this location due to the limited sight distance along Turner Road.
- A Stop bar should be striped for the James Road southbound approach for Turner Road, to reduce intersection creeping and provide a clear location for vehicles to stop.

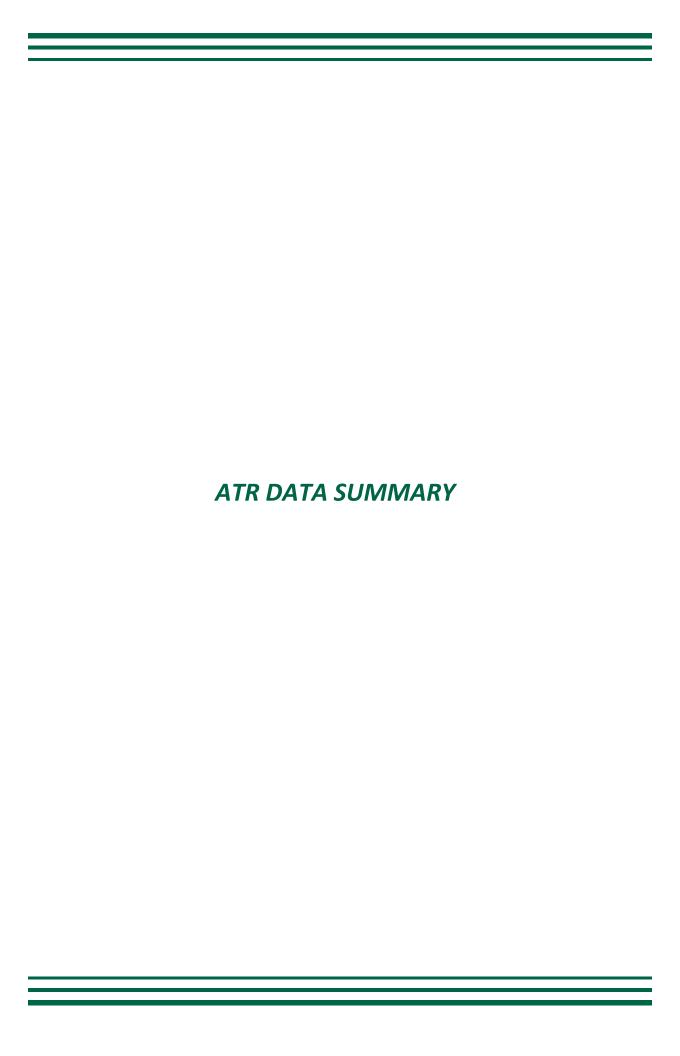
We believe that the responses and updates to the report address all the pertinent comments as well as items discussed during the public hearing. If there is a need to discuss any of these responses further, please feel free to contact me at (978)-923-0400.

Sincerely,

Green International Affiliates, Inc.

Corinne Tobias, P.E.

Project Manager



#### **Cedar Street**

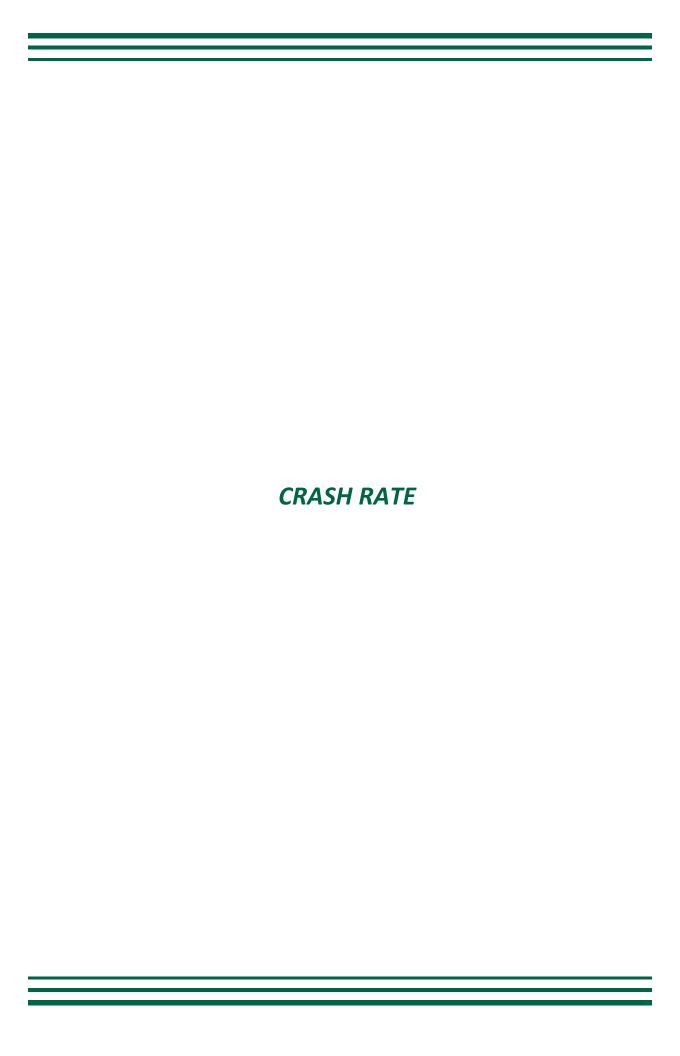
ATR Data Summary

	w	ed, July 8,20	120	Т	hu, Jul 09, 2	2020	Week	day Average		1		ekday Ave urly Volur		Directional D	istribution	
	NB	SB	Both	NB	SB	Both	NB	SB	Both	-	NR	SB	Both	NR	SB	K-Factor
12:00 AM to 12:15 AM	0	1	1	0	1	1	0.0	1.0	1.0	1			201			11.000
12:15 AM to 12:30 AM	0	0	0	0	0	0	0.0	0.0	0.0							†
12:30 AM to 12:45 AM	0	0	0	0	1	1	0.0	0.5	0.5							1
12:45 AM to 01:00 AM	0	0	0	0	0	0	0.0	0.0	0.0	12:00 AM to 01:00 AM	0.0	1.5	1.5	0.000%	100.000%	0.003745
01:00 AM to 01:15 AM	1	0	1	0	0	0	0.5	0.0	0.5	12:15 AM to 01:15 AM	0.5	0.5	1.0	50.000%	50.000%	0.002497
01:15 AM to 01:30 AM	0	0	0	0	0	0	0.0	0.0	0.0	12:30 AM to 01:30 AM	0.5	0.5	1.0	50.000%	50.000%	0.002497
01:30 AM to 01:45 AM	0	0	0	0	0	0	0.0	0.0	0.0	12:45 AM to 01:45 AM	0.5	0.0	0.5	100.000%	0.000%	0.001248
01:45 AM to 02:00 AM	0	0	0	0	0	0	0.0	0.0	0.0	01:00 AM to 02:00 AM	0.5	0.0	0.5	100.000%	0.000%	0.001248
02:00 AM to 02:15 AM	0	0	0	1	0	1	0.5	0.0	0.5	01:15 AM to 02:15 AM	0.5	0.0	0.5	100.000%	0.000%	0.001248
02:15 AM to 02:30 AM	0	0	0	0	0	0	0.0	0.0	0.0	01:30 AM to 02:30 AM	0.5	0.0	0.5	100.000%	0.000%	0.001248
02:30 AM to 02:45 AM	0	0	0	0	0	0	0.0	0.0	0.0	01:45 AM to 02:45 AM	0.5	0.0	0.5	100.000%	0.000%	0.001248
02:45 AM to 03:00 AM	0	0	0	0	0	0	0.0	0.0	0.0	02:00 AM to 03:00 AM	0.5	0.0	0.5	100.000%	0.000%	0.001248
03:00 AM to 03:15 AM	0	0	0 1	0	0	0	0.0	0.0	0.0 1.0	02:15 AM to 03:15 AM	0.0	1.0	0.0	#DIV/0!	#DIV/0!	0.000000
03:15 AM to 03:30 AM 03:30 AM to 03:45 AM	0	0	0	0	0	0	0.0	1.0 0.0	0.0	02:30 AM to 03:30 AM 02:45 AM to 03:45 AM	0.0	1.0	1.0	0.000%	100.000% 100.000%	0.002497
03:45 AM to 04:00 AM	0	0	0	0	0	0	0.0	0.0	0.0	03:00 AM to 04:00 AM	0.0	1.0	1.0	0.000%	100.000%	0.002497
04:00 AM to 04:00 AM	0	0	0	1	1	2	0.0	0.5	1.0	03:00 AM to 04:00 AM 03:15 AM to 04:15 AM	0.0	1.5	2.0	25.000%	75.000%	0.002497
04:15 AM to 04:30 AM	1	1	2	0	0	0	0.5	0.5	1.0	03:30 AM to 04:30 AM	1.0	1.0	2.0	50.000%	50.000%	0.004994
04:30 AM to 04:45 AM	1	0	1	1	1	2	1.0	0.5	1.5	03:45 AM to 04:45 AM	2.0	1.5	3.5	57.143%	42.857%	0.004334
04:45 AM to 05:00 AM	1	0	1	1	0	1	1.0	0.0	1.0	04:00 AM to 05:00 AM	3.0	1.5	4.5	66.667%	33.333%	0.011236
05:00 AM to 05:15 AM	0	0	0	0	0	0	0.0	0.0	0.0	04:15 AM to 05:15 AM	2.5	1.0	3.5	71.429%	28.571%	0.008739
05:15 AM to 05:30 AM	0	3	3	0	2	2	0.0	2.5	2.5	04:30 AM to 05:30 AM	2.0	3.0	5.0	40.000%	60.000%	0.012484
05:30 AM to 05:45 AM	3	0	3	2	2	4	2.5	1.0	3.5	04:45 AM to 05:45 AM	3.5	3.5	7.0	50.000%	50.000%	0.017478
05:45 AM to 06:00 AM	0	1	1	1	0	1	0.5	0.5	1.0	05:00 AM to 06:00 AM	3.0	4.0	7.0	42.857%	57.143%	0.017478
06:00 AM to 06:15 AM	2	0	2	3	0	3	2.5	0.0	2.5	05:15 AM to 06:15 AM	5.5	4.0	9.5	57.895%	42.105%	0.023720
06:15 AM to 06:30 AM	3	1	4	4	2	6	3.5	1.5	5.0	05:30 AM to 06:30 AM	9.0	3.0	12.0	75.000%	25.000%	0.029963
06:30 AM to 06:45 AM	2	3	5	1	2	3	1.5	2.5	4.0	05:45 AM to 06:45 AM	8.0	4.5	12.5	64.000%	36.000%	0.031211
06:45 AM to 07:00 AM	3	0	3	2	0	2	2.5	0.0	2.5	06:00 AM to 07:00 AM	10.0	4.0	14.0	71.429%	28.571%	0.034956
07:00 AM to 07:15 AM	2	1	3	1	3	4	1.5	2.0	3.5	06:15 AM to 07:15 AM	9.0	6.0	15.0	60.000%	40.000%	0.037453
07:15 AM to 07:30 AM 07:30 AM to 07:45 AM	5	0 5	3 10	2	0	2	2.0 3.5	0.5 2.5	2.5 6.0	06:30 AM to 07:30 AM	7.5 9.5	5.0 5.0	12.5 14.5	60.000% 65.517%	40.000% 34.483%	0.031211
07:30 AM to 07:45 AM 07:45 AM to 08:00 AM	5	5	10	1	2	3	3.0	3.5	6.5	06:45 AM to 07:45 AM 07:00 AM to 08:00 AM	10.0	8.5	18.5	54.054%	45.946%	0.036205 <b>0.046192</b>
08:00 AM to 08:15 AM	4	1	5	2	2	4	3.0	1.5	4.5	07:15 AM to 08:15 AM	11.5	8.0	19.5	58.974%	41.026%	0.048689
08:15 AM to 08:30 AM	4	2	6	0	0	0	2.0	1.0	3.0	07:30 AM to 08:30 AM	11.5	8.5	20.0	57.500%	42.500%	0.049938
08:30 AM to 08:45 AM	8	4	12	0	0	0	4.0	2.0	6.0	07:45 AM to 08:45 AM	12.0	8.0	20.0	60.000%	40.000%	0.049938
08:45 AM to 09:00 AM	7	3	10	3	2	5	5.0	2.5	7.5	08:00 AM to 09:00 AM	14.0	7.0	21.0	66.667%	33.333%	0.052434
09:00 AM to 09:15 AM	1	2	3	0	0	0	0.5	1.0	1.5	08:15 AM to 09:15 AM	11.5	6.5	18.0	63.889%	36.111%	0.044944
09:15 AM to 09:30 AM	3	4	7	2	1	3	2.5	2.5	5.0	08:30 AM to 09:30 AM	12.0	8.0	20.0	60.000%	40.000%	0.049938
09:30 AM to 09:45 AM	2	5	7	1	0	1	1.5	2.5	4.0	08:45 AM to 09:45 AM	9.5	8.5	18.0	52.778%	47.222%	0.044944
09:45 AM to 10:00 AM	2	2	4	0	3	3	1.0	2.5	3.5	09:00 AM to 10:00 AM	5.5	8.5	14.0	39.286%	60.714%	0.034956
10:00 AM to 10:15 AM	1	0	1	2	2	4	1.5	1.0	2.5	09:15 AM to 10:15 AM	6.5	8.5	15.0	43.333%	56.667%	0.037453
10:15 AM to 10:30 AM	5	2	7	0	0	0	2.5	1.0	3.5	09:30 AM to 10:30 AM	6.5	7.0	13.5	48.148%	51.852%	0.033708
10:30 AM to 10:45 AM	5	0	5	1	1	2	3.0	0.5	3.5	09:45 AM to 10:45 AM	8.0	5.0	13.0	61.538%	38.462%	0.032459
10:45 AM to 11:00 AM	6	2	8	3	2	5	4.5	2.0	6.5	10:00 AM to 11:00 AM	11.5	4.5	16.0	71.875%	28.125%	0.039950
11:00 AM to 11:15 AM	2	2	4	1	2	3	1.5	2.0	3.5	10:15 AM to 11:15 AM	11.5	5.5	17.0	67.647%	32.353%	0.042447
11:15 AM to 11:30 AM	3	5	8	2	4	6	2.5	4.5	7.0	10:30 AM to 11:30 AM	11.5	9.0	20.5	56.098%	40 70001	0.051186
11:30 AM to 11:45 AM	2	2	3	2	2	4	2.0	1.5	3.5 5.0	10:45 AM to 11:45 AM	10.5	10.0	20.5	51.220%	48.780%	0.051186
11:45 AM to 12:00 PM 12:00 PM to 12:15 PM	0	3	3	4	1	6	3.0 1.5	2.0 2.0	3.5	<b>11:00 AM to 12:00 PM</b> 11:15 AM to 12:15 PM	<b>9.0</b> 9.0	<b>10.0</b> 10.0	<b>19.0</b> 19.0	<b>47.368%</b> 47.368%	<b>52.632%</b> 52.632%	0.047441 0.047441
12:15 PM to 12:15 PM	1	1	2	2	1	3	1.5	1.0	2.5	11:15 AM to 12:15 PM	8.0	6.5	14.5	55.172%	44.828%	0.036205
12:30 PM to 12:45 PM	1	4	5	2	2	4	1.5	3.0	4.5	11:45 AM to 12:45 PM	7.5	8.0	15.5	48.387%	51.613%	0.038702
12:45 PM to 01:00 PM	5	2	7	2	1	3	3.5	1.5	5.0	12:00 PM to 01:00 PM	8.0	7.5	15.5	51.613%	48.387%	0.038702
01:00 PM to 01:15 PM	6	3	9	4	2	6	5.0	2.5	7.5	12:15 PM to 01:15 PM	11.5	8.0	19.5	58.974%	41.026%	0.048689
01:15 PM to 01:30 PM	4	2	6	1	3	4	2.5	2.5	5.0	12:30 PM to 01:30 PM	12.5	9.5	22.0	56.818%	43.182%	0.054931
01:30 PM to 01:45 PM	2	1	3	2	5	7	2.0	3.0	5.0	12:45 PM to 01:45 PM	13.0	9.5	22.5	57.778%	42.222%	0.056180
01:45 PM to 02:00 PM	2	2	4	5	2	7	3.5	2.0	5.5	01:00 PM to 02:00 PM	13.0	10.0	23.0	56.522%	43.478%	0.057428
02:00 PM to 02:15 PM	1	6	7	2	0	2	1.5	3.0	4.5	01:15 PM to 02:15 PM	9.5	10.5	20.0	47.500%	52.500%	0.049938
02:15 PM to 02:30 PM	1	4	5	4	6	10	2.5	5.0	7.5	01:30 PM to 02:30 PM	9.5	13.0	22.5	42.222%	57.778%	0.056180
02:30 PM to 02:45 PM	1	3	4	5	1	6	3.0	2.0	5.0	01:45 PM to 02:45 PM	10.5	12.0	22.5	46.667%	53.333%	0.056180
02:45 PM to 03:00 PM	4	3	7	5	4	9	4.5	3.5	8.0	02:00 PM to 03:00 PM	11.5	13.5	25.0	46.000%	54.000%	0.062422

#### **Cedar Street**

#### ATR Data Summary

											Wee	kday Ave	rage			
	W	ed, July 8,20	020	1	hu, Jul 09, 2	2020	Week	day Average		1	Ho	urly Volun	nes	Directional D	istribution	
	NB	SB	Both	NB	SB	Both	NB	SB	Both		NB	SB	Both	NB	SB	K-Factor
03:00 PM to 03:15 PM	6	12	18	2	4	6	4.0	8.0	12.0	02:15 PM to 03:15 PM	14.0	18.5	32.5	43.077%	56.923%	0.081149
03:15 PM to 03:30 PM	3	3	6	1	4	5	2.0	3.5	5.5	02:30 PM to 03:30 PM	13.5	17.0	30.5	44.262%	55.738%	0.076155
03:30 PM to 03:45 PM	6	7	13	3	5	8	4.5	6.0	10.5	02:45 PM to 03:45 PM	15.0	21.0	36.0	41.667%	58.333%	0.089888
03:45 PM to 04:00 PM	4	8	12	3	6	9	3.5	7.0	10.5	03:00 PM to 04:00 PM	14.0	24.5	38.5	36.364%	63.636%	0.096130
04:00 PM to 04:15 PM	1	5	6	5	7	12	3.0	6.0	9.0	03:15 PM to 04:15 PM	13.0	22.5	35.5	36.620%	63.380%	0.088639
04:15 PM to 04:30 PM	3	9	12	2	4	6	2.5	6.5	9.0	03:30 PM to 04:30 PM	13.5	25.5	39.0	34.615%	65.385%	0.097378
04:30 PM to 04:45 PM	1	12	13	1	6	7	1.0	9.0	10.0	03:45 PM to 04:45 PM	10.0	28.5	38.5	25.974%	74.026%	0.096130
04:45 PM to 05:00 PM	3	4	7	4	8	12	3.5	6.0	9.5	04:00 PM to 05:00 PM	10.0	27.5	37.5	26.667%	73.333%	0.093633
05:00 PM to 05:15 PM	3	9	12	3	6	9	3.0	7.5	10.5	04:15 PM to 05:15 PM	10.0	29.0	39.0	25.641%	74.359%	0.097378
05:15 PM to 05:30 PM	4	10	14	3	6	9	3.5	8.0	11.5	04:30 PM to 05:30 PM	11.0	30.5	41.5	26.506%	73.494%	0.103620
05:30 PM to 05:45 PM	4	3	7	8	3	11	6.0	3.0	9.0	04:45 PM to 05:45 PM	16.0	24.5	40.5	39.506%	60.494%	0.101124
05:45 PM to 06:00 PM	4	2	6	3	6	9	3.5	4.0	7.5	05:00 PM to 06:00 PM	16.0	22.5	38.5	41.558%	58.442%	0.096130
06:00 PM to 06:15 PM	5	4	9	3	0	3	4.0	2.0	6.0	05:15 PM to 06:15 PM	17.0	17.0	34.0	50.000%	50.000%	0.084894
06:15 PM to 06:30 PM	6	4	10	5	3	8	5.5	3.5	9.0	05:30 PM to 06:30 PM	19.0	12.5	31.5	60.317%	39.683%	0.078652
06:30 PM to 06:45 PM	6	6	12	3	5	8	4.5	5.5	10.0	05:45 PM to 06:45 PM	17.5	15.0	32.5	53.846%	46.154%	0.081149
06:45 PM to 07:00 PM	4	2	6	2	4	6	3.0	3.0	6.0	06:00 PM to 07:00 PM	17.0	14.0	31.0	54.839%	45.161%	0.077403
07:00 PM to 07:15 PM	2	4	6	2	7	9	2.0	5.5	7.5	06:15 PM to 07:15 PM	15.0	17.5	32.5	46.154%	53.846%	0.081149
07:15 PM to 07:30 PM	0	4	4	3	2	5	1.5	3.0	4.5	06:30 PM to 07:30 PM	11.0	17.0	28.0	39.286%	60.714%	0.069913
07:30 PM to 07:45 PM	1	8	9	3	4	7	2.0	6.0	8.0	06:45 PM to 07:45 PM	8.5	17.5	26.0	32.692%	67.308%	0.064919
07:45 PM to 08:00 PM	3	1	4	4	2	6	3.5	1.5	5.0	07:00 PM to 08:00 PM	9.0	16.0	25.0	36.000%	64.000%	0.062422
08:00 PM to 08:15 PM	2	4	6	1	6	7	1.5	5.0	6.5	07:15 PM to 08:15 PM	8.5	15.5	24.0	35.417%	64.583%	0.059925
08:15 PM to 08:30 PM	0	3	3	2	5	7	1.0	4.0	5.0	07:30 PM to 08:30 PM	8.0	16.5	24.5	32.653%	67.347%	0.061174
08:30 PM to 08:45 PM	1	5	6	0	5	5	0.5	5.0	5.5	07:45 PM to 08:45 PM	6.5	15.5	22.0	29.545%	70.455%	0.054931
08:45 PM to 09:00 PM	1	4	5	2	6	8	1.5	5.0	6.5	08:00 PM to 09:00 PM	4.5	19.0	23.5	19.149%	80.851%	0.058677
09:00 PM to 09:15 PM	1	6	7	2	4	6	1.5	5.0	6.5	08:15 PM to 09:15 PM	4.5	19.0	23.5	19.149%	80.851%	0.058677
09:15 PM to 09:30 PM	1	1	2	3	5	8	2.0	3.0	5.0	08:30 PM to 09:30 PM	5.5	18.0	23.5	23.404%	76.596%	0.058677
09:30 PM to 09:45 PM	2	1	3	3	1	4	2.5	1.0	3.5	08:45 PM to 09:45 PM	7.5	14.0	21.5	34.884%	65.116%	0.053683
09:45 PM to 10:00 PM	0	2	2	0	4	4	0.0	3.0	3.0	09:00 PM to 10:00 PM	6.0	12.0	18.0	33.333%	66.667%	0.044944
10:00 PM to 10:15 PM	1	0	1	0	2	2	0.5	1.0	1.5	09:15 PM to 10:15 PM	5.0	8.0	13.0	38.462%	61.538%	0.032459
10:15 PM to 10:30 PM	2	2	4	2	2	4	2.0	2.0	4.0	09:30 PM to 10:30 PM	5.0	7.0	12.0	41.667%	58.333%	0.029963
10:30 PM to 10:45 PM	0	0	0	0	0	0	0.0	0.0	0.0	09:45 PM to 10:45 PM	2.5	6.0	8.5	29.412%	70.588%	0.021223
10:45 PM to 11:00 PM	1	2	3	0	0	0	0.5	1.0	1.5	10:00 PM to 11:00 PM	3.0	4.0	7.0	42.857%	57.143%	0.017478
11:00 PM to 11:15 PM	0	0	0	0	0	0	0.0	0.0	0.0	10:15 PM to 11:15 PM	2.5	3.0	5.5	45.455%	54.545%	0.013733
11:15 PM to 11:30 PM	0	0	0	0	0	0	0.0	0.0	0.0	10:30 PM to 11:30 PM	0.5	1.0	1.5	33.333%	66.667%	0.003745
11:30 PM to 11:45 PM	0	0	0	0	0	0	0.0	0.0	0.0	10:45 PM to 11:45 PM	0.5	1.0	1.5	33.333%	66.667%	0.003745
11:45 PM to 12:00 AM	0	1	1	0	0	0	0.0	0.5	0.5	11:00 PM to 12:00 AM	0.0	0.5	0.5	0.000%	100.000%	0.001248
Daily Totals	198.0	241.0	439.0	160.0	202.0	362.0	179.0	221.5	400.5				•		•	





TOWN :	Holliston				COUNT DA	TE:	2/26/2020
DISTRICT :	3	UNSIGN	IALIZED :	٧	SIGNA	ALIZED :	
			~ IN	TERSECTION	I DATA ~		
MAJOR STRE	EET:	Ashland Stre	eet				
MINOR STRE	ET(S):	Turner Road	<u> </u>				
INTERSE DIAGR		North			Turner Road Ashland	Street	
				PEAK HOUI	R VOLUMES		
APPRO	ACH :	1	2	3	4	5	Total Peak Hourly
DIRECT	TON:	NB	SB	EB	WB		Approach Volume
PEAK HC		32	0	101	152		285
"K" FAC	CTOR:	0.08	INTERS	ECTION ADT APPROACH	( <b>V</b> ) = TOTA H VOLUME :	AL DAILY	3,563
FOTAL # OF (	CRASHES :	0	# OF YEARS :	5	CRASHES	GE#OF PERYEAR(	0.20
CRASH RA	ATE CALCU	LATION :	0.00	RATE =	<u>( A * 1,0</u>	000,000 ) * 365 )	
Comments:	The average Date:	ge crash rate Geoffery Pai	for an unsigna	alized interse	ction in Distric	ot is 0.57	



TOWN:	Holliston				COUNT DA	TE:	2/26/2020
DISTRICT :	3	UNSIGN	IALIZED :	٧	SIGNA	LIZED :	
			~ IN]	TERSECTION	N DATA ~		
MAJOR STRE	EET:	Cedar Stree	t				
MINOR STRE	EET(S):	Turner Road	I				
INTERSE DIAGF		North			Cedar Street Turner R	oad	
				PEAK HOU	R VOLUMES		
APPRO	ACH :	1	2	3	4	5	Total Peak Hourly
DIRECT	TION :	NB	SB	EB	WB		Approach Volume
PEAK HO		25	44	4	0		73
"K" FAC	CTOR:	0.08	INTERSI		( <b>V</b> ) = TOTA H VOLUME :	AL DAILY	913
TOTAL # OF (	CRASHES :	0	# OF YEARS :	5	CRASHES	GE#OF PERYEAR( .):	0.00
CRASH RA	ATE CALCU	LATION :	0.00	RATE =	- (A * 1,0	000,000 ) * 365 )	
Comments :	The averag	ge crash rate	for an unsigna	alized interse	ction in Distric	ot is 0.57	



TOWN:	Holliston	_			COUNT DA	TE:	2/26/2020
DISTRICT :	3	UNSIGN	IALIZED :	٧	SIGNA	LIZED :	
			~ IN	TERSECTION	I DATA ~		
MAJOR STRE	ET:	Main Street					
MINOR STRE	ET(S):	Shrewsbury	Street				
INTERSE DIAGF		North	Cedar Stre	eet Elliot Street	Driveway		
		<u> </u>		PEAK HOUR	R VOLUMES		
APPRO	ACH:	1	2	3	4	5	Total Peak Hourly
DIRECT	ION :	NB	SB	EB	WB	SWB	Approach Volume
PEAK HO VOLUMES		87	40	367	496	0	990
"K" FAC	CTOR:	0.08	INTERSI	ECTION ADT APPROACH		AL DAILY	12,375
ΓΟΤΑL # OF (	CRASHES :	10	# OF YEARS :	5	CRASHES	GE#OF PERYEAR( .):	2.20
CRASH RA	ATE CALCU	ILATION :	0.44	RATE =	( A * 1,0	000,000 ) * 365 )	
Comments :		ge crash rate Geoffery Par	for an unsigna	alized intersec	tion in Distric	ot 5 is 0.57	



OWN:	Holliston	_			COUNT DA	TE:	6/24/2020
DISTRICT :	3	UNSIGN	ALIZED :	V	SIGNA	LIZED :	
			~ IN	TERSECTION	I DATA ~		
MAJOR STRE	EET:	Cedar Street					
MINOR STRE	EET(S):	Indian Ridge	Road				
INTERSE DIAGE		North		Indian Ridge Road	Turner St	reet	
				PEAK HOUF	R VOLUMES		
APPRO	ACH :	1	2	3	4	5	Total Peak Hourly
DIRECT	TON:	NB	WB	EB			Approach Volume
PEAK HO		7	19	7			33
"K" FAC	CTOR:	0.08	INTERS	ECTION ADT APPROACH		AL DAILY	413
OTAL # OF (	CRASHES :	0	# OF YEARS :	5	CRASHES	GE # OF PER YEAR ( ):	0.00
CRASH RA	ATE CALCU	ILATION :	0.00	RATE =	( A * 1,0	000,000 ) * 365 )	



TOWN :	Holliston	<u>-</u>			COUNT DA	TE:	2/26/2020
DISTRICT :	3	UNSIGN	IALIZED :	٧	SIGNA	LIZED :	
			~ IN	TERSECTION	N DATA ~		
MAJOR STRE	EET:	Ashland Stre	et				
MINOR STRE	ET(S):	Cedar Street	:				
INTERSE DIAGE		North			Ashland	Street	
				PEAK HOUI	R VOLUMES		
APPRO	ACH :	1	2	3	4	5	Total Peak Hourly
DIRECT	TON:	NB	SB	EB	WB		Approach Volume
PEAK HO VOLUMES		14	17	101	130		262
"K" FAC	CTOR:	0.08	INTERS		( <b>V</b> ) = TOTA H VOLUME :	AL DAILY	3,275
ΓΟΤΑL # OF (	CRASHES :	2	# OF YEARS :	5	CRASHES	GE#OF PERYEAR( .):	0.60
CRASH RA	ATE CALCU	ILATION :	0.33	RATE =	<u>( A * 1,0</u>	000,000 ) * 365 )	
Comments :		ge crash rate Geoffery Par		alized interse	ction in Distric	et is 0.57	



2019 and 2020 Traffic counts from AET08 on Mass. Turnpike East of Cordaville

Date	2019 Volume	Date	2020 Volume	Difference
7/1/2019	105563	7/1/2020	73210	31%
7/2/2019	105017	7/2/2020	83300	21%
7/3/2019	106142	7/3/2020	67685	36%
7/4/2019	69418	7/4/2020	48483	30%
7/5/2019	90134	7/5/2020	64076	29%
7/6/2019	84654	7/6/2020	71464	16%
7/7/2019	97124	7/7/2020	71677	26%
7/8/2019	105386	7/8/2020	70383	33%
7/9/2019	104237	7/9/2020	74393	29%
7/10/2019	107614	7/10/2020	75562	30%
7/11/2019	113328	7/11/2020	63587	44%
7/12/2019	115899	7/12/2020	62530	46%
7/13/2019	101600	7/13/2020	69112	32%
7/14/2019	100543	7/14/2020	70405	30%
7/15/2019	108733	7/15/2020	73266	33%
7/16/2019	0	7/16/2020	76900	
7/17/2019	0	7/17/2020	81473	
7/18/2019	115489	7/18/2020	70039	39%
7/19/2019	117940	7/19/2020	63858	46%
7/20/2019	93672	7/20/2020	71497	24%
7/21/2019	97052	7/21/2020	72618	25%
7/22/2019	105869	7/22/2020	73063	31%
7/23/2019	102394	7/23/2020	75443	26%
7/24/2019	111605	7/24/2020	85461	23%
7/25/2019	117202	7/25/2020	73071	38%
7/26/2019	121135	7/26/2020	68144	44%
7/27/2019	100017	7/27/2020	72351	28%
7/28/2019	100373	7/28/2020	72181	28%
7/29/2019	108173	7/29/2020	76131	30%
7/30/2019	107333	7/30/2020	78934	26%
7/31/2019	0	7/31/2020	88944	

2019 and 2020 Traffic counts from 6125 Interstate 495 at Franklin Town Line

Date	2019 Volume	Date	2020 Volume	Difference
7/1/2019	97674	7/1/2020	76863	21%
7/2/2019	102610	7/2/2020	0	
7/3/2019	103349	7/3/2020	67142	35%
7/4/2019	56071	7/4/2020	50934	9%
7/5/2019	79197	7/5/2020	58765	26%
7/6/2019	68733	7/6/2020	74218	-8%
7/7/2019	74086	7/7/2020	74183	0%
7/8/2019	100091	7/8/2020	76306	24%
7/9/2019	106395	7/9/2020	79929	25%
7/10/2019	103771	7/10/2020	79738	23%
7/11/2019	108274	7/11/2020	66062	39%
7/12/2019	111604	7/12/2020	61913	45%
7/13/2019	94329	7/13/2020	72920	23%
7/14/2019	80753	7/14/2020	74613	8%
7/15/2019	103392	7/15/2020	79348	23%
7/16/2019	104269	7/16/2020	81909	21%
7/17/2019	98771	7/17/2020	85496	13%
7/18/2019	105531	7/18/2020	72912	31%
7/19/2019	0	7/19/2020	62758	
7/20/2019	83141	7/20/2020	77144	7%
7/21/2019	72727	7/21/2020	78292	-8%
7/22/2019	107603	7/22/2020	79452	26%
7/23/2019	94447	7/23/2020	80329	15%
7/24/2019	105919	7/24/2020	88496	16%
7/25/2019	106186	7/25/2020	74851	30%
7/26/2019	110650	7/26/2020	64631	42%
7/27/2019	90669	7/27/2020	77350	15%
7/28/2019	77538	7/28/2020	76899	1%
7/29/2019	101149	7/29/2020	81001	20%
7/30/2019	102960	7/30/2020	82783	20%
7/31/2019	103332	7/31/2020	90270	13%

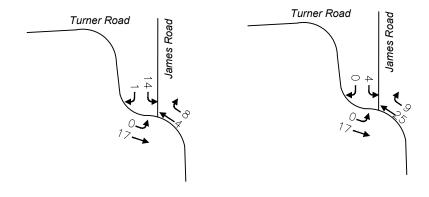
2020 20082 Traffic Counts at James Road and Turner Road

	2020 Co	unt Data	2020 Volumes with o	lifference from 2019	2020 Traffic Volumes	with seasonal factor	Background	Growth Rate	2020 with seasonal	and/or Background
James Road SB I	1	0	1	0	1	0	0	0	1	0
James Road SB I	11	3	14	4	11	3	0	0	11	3
Turner Road WI	6	7	8	9	6	7	0	0	6	7
Turner Road WI	3	19	4	25	3	19	0	0	3	19
Turner Road EB	13	4	17	5	13	4	0	0	13	4
Turner Road EB	0	0	0	0	0	0	0	0	0	0

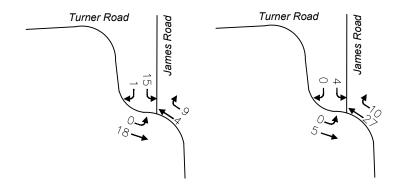
Seasonal Factor	0.00%
Annual Growth Rate	0.00%
Number of years	0



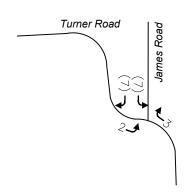
# AM/PM EXISTING

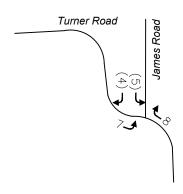


# 2027 AM/PM NO BUILD



## SITE GENERATED TRIPS

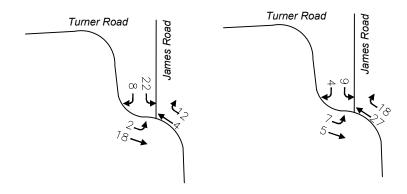


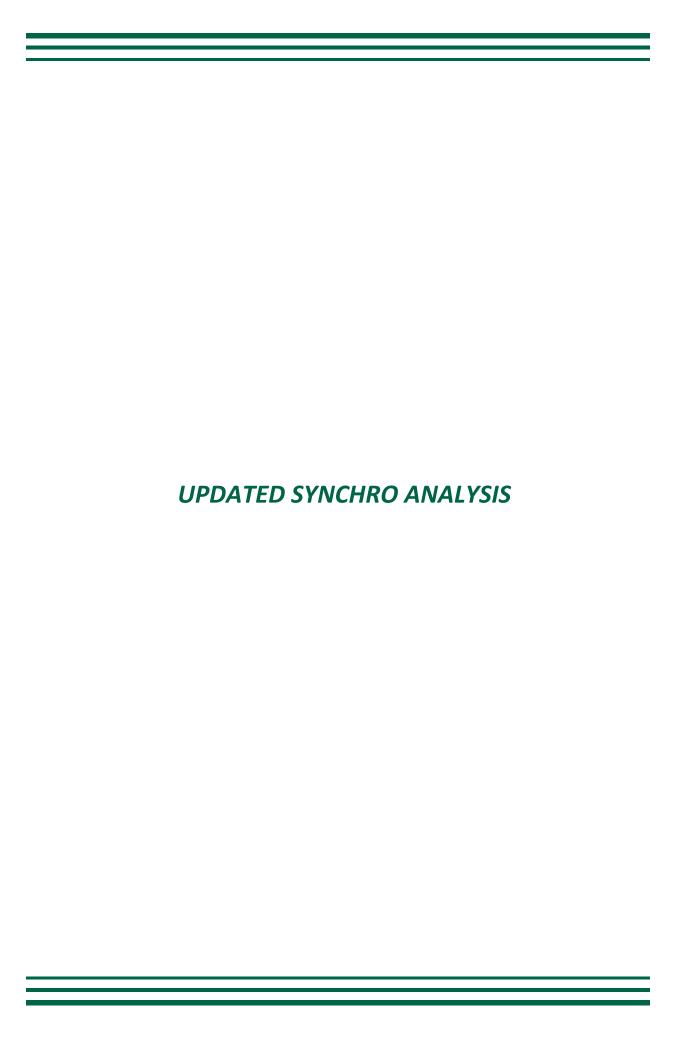


	ENTER	EXIT	TOTAL
SITE-GENERATED TRIPS	5	14	19

	ENTER	EXIT	TOTAL
SITE-GENERATED TRIPS	15	9	24

# 2027 AM/PM BUILD





Intersection						
Int Delay, s/veh	3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LUL	4	₩ <u>₩</u>	WDR	→ Y	JUIC
Traffic Vol, veh/h	0	17	4	8	14	1
Future Vol, veh/h	0	17	4	8	14	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	78	78	78	78	78	78
Heavy Vehicles, %	0	8	14	0	6	0
Mvmt Flow	0	22	5	10	18	1
Major/Minor N	/lajor1	N	Major2		Minor2	
Conflicting Flow All	15	0	<u>viajui 2</u> -	0	32	10
Stage 1	-	U	-		10	-
Stage 2		-	-	-	22	
	4.1	-	-	-	6.46	6.2
Critical Hdwy Critical Hdwy Stg 1	4.1	-	-	-	5.46	0.2
Critical Hdwy Stg 2		-	-		5.46	-
, ,	2.2	-	-	-	3.554	3.3
Follow-up Hdwy	1616	-	-		972	1077
Pot Cap-1 Maneuver	1010	-	-	-	1003	1077
Stage 1		-	-	-	990	-
Stage 2 Platoon blocked, %	-	-	-	-	990	-
	1616	-	-	-	972	1077
Mov Cap 3 Manager		-	-	-	972	1077
Mov Cap-2 Maneuver	-	-	-	-	1003	
Stage 1	-	-	-	-	990	-
Stage 2	-	-	-	-	990	-
					SB	
Approach	EB		WB		JU	
HCM Control Delay, s	EB 0		WB 0		8.8	
HCM Control Delay, s					8.8	
HCM Control Delay, s HCM LOS	0	FRI	0	WRT	8.8 A	SRI n1
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm	0	EBL 1616		WBT	8.8 A WBR S	
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h)	0	1616	0 EBT	-	8.8 A WBR :	978
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	0 t	1616 -	0 EBT -	-	8.8 A WBR 9	978 0.02
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	0 t	1616 - 0	0 EBT - -	- - -	8.8 A WBR :	978 0.02 8.8
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	0 t	1616 -	0 EBT -	-	8.8 A WBR 9	978 0.02

0.8					
FRI	FRT	WRT	WRR	SRI	SBR
LDL			WUIN		JUIN
0			0		0
					0
					0
					Stop
					None
		_			-
		٥			
					-
					83
					0
					0
U	U	30	11	J	U
		Major2			
41	0	-	0		36
-	-	-	-	36	-
-	-	-	-	6	-
4.1	-	-	-		6.2
-	-	-	-		-
-	-	-	-		-
	-	-	-		3.3
1581	-	-	-		1042
-	-	-	-		-
-	-	-	-	1022	-
	-	-	-		
1581	-	-	-		1042
-	-	-	-		-
-	-	-	-		-
-	-	-	-	1022	-
FR		WR		SB	
U		U			
				А	
nt	EBL	EBT	WBT	WBR S	SBLn1
	1581	-	-	-	974
	1001				0.005
	-	-	-	-	0.005
		-	-	-	8.7
)	-	- - -			
	EBL  0 0 0 Free - 83 0 0  Major1 41 2.2 1581 1581  EBB 0	EBL EBT  0 5 0 5 0 0 5 0 0 0 Free Free - None - 0 83 83 0 0 0 6  Major1 N 41 0 4.1 2.2 - 1581 1581  1581  EB 0	EBL EBT WBT  0 5 25 0 0 0 0 Free Free Free - None 0, # - 0 0 83 83 83 0 0 5 0 6 30  Major1 Major2 41 0 4.1 2.2 1581 1581  EB WB 0 0 0	EBL EBT WBT WBR	EBL         EBT         WBT         WBR         SBL           0         5         25         9         4           0         5         25         9         4           0         0         0         0         0           Free         Free         Free         Stop         -           None         -         None         -         0           0         0         0         -         0           0         0         0         -         0           83         83         83         83         83           0         0         5         0         0           0         6         30         11         5    Major1  Major2  Minor2  Minor2  Minor2   Minor2   Minor2   Minor2

Intersection						
Int Delay, s/veh	3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	f <sub>a</sub>		Y	JJK
Traffic Vol, veh/h	0	18	4	9	15	1
Future Vol, veh/h	0	18	4	9	15	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		- -	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage	.# -	0	0	-	0	_
Grade, %	-	0	0	_	0	_
Peak Hour Factor	78	78	78	78	78	78
Heavy Vehicles, %	0	8	14	0	6	0
Mvmt Flow	0	23	5	12	19	1
WWITH FIOW	U	23	5	12	19	ı
Major/Minor N	/lajor1	ľ	Major2	ľ	Minor2	
Conflicting Flow All	17	0	-	0	34	11
Stage 1	-	-	-	-	11	-
Stage 2	-	-	-	-	23	-
Critical Hdwy	4.1	_	-	-	6.46	6.2
Critical Hdwy Stg 1			-	_	5.46	-
Critical Hdwy Stg 2	_	_	_	_	5.46	_
Follow-up Hdwy	2.2	_	_		3.554	3.3
Pot Cap-1 Maneuver	1613	_	_	_	969	1076
Stage 1	-	_	_	<u>-</u>	1002	1070
Stage 2	_		_		989	-
Platoon blocked, %	-	-	-		707	-
	1613	-	-	-	040	1076
Mov Cap-1 Maneuver		-	-	-	969	
Mov Cap-2 Maneuver	-	-	-	-	969	-
Stage 1	-	-	-	-	1002	-
Stage 2	-	-	-	-	989	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.8	
HCM LOS	U		U		A	
HOW EOS					, , , , , , , , , , , , , , , , , , ,	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1613	-	-	-	975
HCM Lane V/C Ratio		-	-	-	-	0.021
HCM Control Delay (s)		0	-	-	-	8.8
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q(veh)		0	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	LDL	<u> </u>	T <sub>P</sub>	WDK	JDL W	SDR
Lane Configurations	0			10		0
Traffic Vol, veh/h	0	5	27	10	4	0
Future Vol, veh/h	0	5	27	10	4	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	0	0	5	0	0	0
Mvmt Flow	0	6	33	12	5	0
WWW.C TOW	U				U	U
	Major1		Major2		Minor2	
Conflicting Flow All	45	0	-	0	45	39
Stage 1	-	-	-	-	39	-
Stage 2	-	-	-	-	6	-
Critical Hdwy	4.1	-	-	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	_	-	-	_	5.4	-
Follow-up Hdwy	2.2	_	_	-	3.5	3.3
Pot Cap-1 Maneuver	1576	_	_	_	970	1038
Stage 1	-		_	_	989	-
Stage 2	_	-		_	1022	_
Platoon blocked, %	_		_	_	1022	_
	1576		_		970	1038
Mov Cap-1 Maneuver		-	-	-		
Mov Cap-2 Maneuver	-	-	-	-	970	-
Stage 1	-	-	-	-	989	-
Stage 2	-	-		-	1022	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.7	
HCM LOS	U		U		Α	
HOW LOS					A	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1576	-	_	_	970
HCM Lane V/C Ratio		-	-	_	-	0.005
HCM Control Delay (s)		0		_	_	8.7
HCM Lane LOS		A	_	-	-	Α
HCM 95th %tile Q(veh)	)	0		_		0
HOW JOHN JOHN QUELL		U				U

Intersection						
Int Delay, s/veh	4.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	LDL	EDI	WDI	NOK	SBL W	אמכ
Lane Configurations	2			10		0
Traffic Vol, veh/h	2	18	4	12	22	8
Future Vol, veh/h	2	18	4	12	22	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	78	78	78	78	78	78
Heavy Vehicles, %	0	8	14	0	6	0
Mvmt Flow	3	23	5	15	28	10
WWW. Tiow	J	20	U	10	20	10
Major/Minor N	/lajor1	Ν	/lajor2	N	Minor2	
Conflicting Flow All	20	0	-	0	42	13
Stage 1	-	-	-	-	13	-
Stage 2	-	-	-	-	29	-
Critical Hdwy	4.1	-	-	-	6.46	6.2
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	_	_	-	_	5.46	_
Follow-up Hdwy	2.2	_	_	_	3.554	3.3
Pot Cap-1 Maneuver	1609	_	_	_	959	1073
Stage 1	1007	_	_	_	1000	-
Stage 2	_	_	-	-	983	_
Platoon blocked, %	-	-	-		903	-
	1/00	-	-	-	057	1072
Mov Cap-1 Maneuver	1609	-	-	-	957	1073
Mov Cap-2 Maneuver	-	-	-	-	957	-
Stage 1	-	-	-	-	998	-
Stage 2	-	-	-	-	983	-
Approach	EB		WB		SB	
	0.7		0		8.8	
HCM Control Delay, s	0.7		U			
HCM LOS					Α	
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1609	_	_	_	985
HCM Lane V/C Ratio		0.002	_	_	_	0.039
HCM Control Delay (s)		7.2	0	_	-	8.8
HCM Lane LOS		Α.2	A	-	-	Α
HCM 95th %tile Q(veh)	١	0	А	-	-	0.1
HOW FOUT MILE Q(VEH)	1	U	-		-	U. I

Intersection						
Int Delay, s/veh	2.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	1	WDIX	Y	ODIC
Traffic Vol, veh/h	7	5	27	18	9	4
Future Vol, veh/h	7	5	27	18	9	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	0	0	5	0	0	0
Mvmt Flow	8	6	33	22	11	5
Major/Minor N	1ajor1	ı	Major2	N	Minor2	
Conflicting Flow All	55	0	<u>viajui 2</u> -	0	66	44
Stage 1	- 33	U	-	-	44	- 44
Stage 2	-	-	-	-	22	-
Critical Hdwy	4.1	-	-	-	6.4	6.2
Critical Hdwy Stg 1	4.1	-	_	-	5.4	0.2
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	2.2	-	_	-	3.5	3.3
Pot Cap-1 Maneuver	1563	-	-	-	944	1032
Stage 1	1303	-	_	_	984	1032
Stage 2				-	1006	-
Platoon blocked, %	-	-	-	-	1000	-
Mov Cap-1 Maneuver	1563			-	939	1032
Mov Cap-1 Maneuver	1303	-	-	_	939	1032
Stage 1				_	979	_
Stage 2		-	_	_	1006	-
Stage 2	-	-	-		1000	
Approach	EB		WB		SB	
HCM Control Delay, s	4.3		0		8.8	
HCM LOS					Α	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SBI n1
Capacity (veh/h)		1563		-	-	966
HCM Lane V/C Ratio		0.005	_	_		0.016
HCM Control Delay (s)		7.3	0	_	_	8.8
HCM Lane LOS		Α.5	A	_	_	A
HCM 95th %tile Q(veh)		0	-		_	0
HUVI YOUN WILLE COMEN						