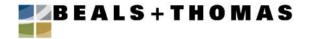
# **Abbreviated Stormwater Management Report**

# **BLAIR SQUARE SITE IMPROVEMENTS**

0 Railroad Bed Holliston, Massachusetts

Prepared for: **Holliston Planning Board** 703 Washington Street Holliston, MA 01746

Prepared by:

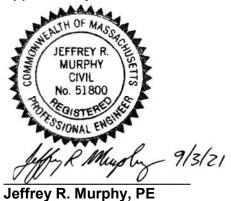


September 3, 2021

Calculated by: Nathaniel Bautz, EIT

Checked by: Jeffrey R. Murphy, PE

Approved by:



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### **LIST OF ATTACHMENTS**

ATTACHMENT 1: SOIL DATA

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# 1.0 INTRODUCTION

The proposed project includes several Low Impact Development (LID) stormwater Best Management Practices (BMPs) to address stormwater runoff from the proposed Blair Square Improvements. The proposed stormwater BMPs will control peak runoff rates, promote groundwater recharge and sediment removal.

The proposed project has been designed to the extent applicable, in accordance with the requirements of the DEP Stormwater Handbook, as well as the Holliston Stormwater Management and Land Disturbance Regulation and associated Stormwater Performance Standards.

The pre- and post-development hydrologic conditions were modeled using HydroCAD<sup>TM</sup> version 10.10 to demonstrate that post-development stormwater runoff rates will be less than or equal to the pre-development rates. Watershed maps with soil types as well as detailed analysis of the model results are also included. The following table summarizes the peak runoff rates for the pre- and post-development conditions.

Table 1: Pre- & Post-development Peak Runoff Rate Comparison, units are in cubic feet per second (cfs).

Storm	2 Year		10 Year		25 Year		50 Year		100 Year	
Event	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post
Design Point 1	0.36	0.07	1.03	0.16	1.47	0.27	1.83	0.35	2.51	0.57
Design Point 2	0.19	0.09	0.56	0.32	0.81	0.48	1.00	0.61	1.39	0.86
Design Point 3	0.09	0.05	0.23	0.17	0.33	0.26	0.41	0.32	0.55	0.46



## 2.0 PRE-DEVELOPMENT CONDITIONS

## 2.1 Site Conditions

The Project site is a parcel of land comprised of a former railroad corridor, which contains a section of the Upper Charles Rail Trail. The parcel is bound to the east by Central Street, to the north by Railroad Street, to the south by Front Street, and to the West by Exchange Street. Along the existing stone dust path is a mix of existing grassed areas with small gardens and various site amenities, as well as strips of wooded areas on either side of the trail mostly to the south of the proposed improvements.

Stormwater from the existing rail trail is relatively self-contained to a series of low-lying areas on the east side of the trail given the limited longitudinal slope of the trail. However, in a macro level view of the site, stormwater drains primarily in three directions which have identified as design points (DPs) for the hydrologic analysis.

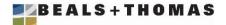
- DP-1: The northern portion of the site drains generally drains to series of lowlying areas along the east side of the Upper Charles Trail, as the trail is flanked on both sides by slightly higher terrain. Overall, this area pitches north towards Central Street.
- o DP-2: The southwest portion of the site drains towards Railroad Street, which generally slopes to the south beyond the limit of work.
- DP-3: A small area of the project site drains southeasterly towards Front Street, which also slopes to the south beyond the limit of work.

These design points have been labeled accordingly on the watershed maps in Attachment 2.

## 2.2 Soil Description

The Natural Resources Conservation Service (NRCS) maps the proposed site as Urban Land Classification, as much of the site is located in previously developed/filled land associated with the construction of the railroad. However, it is bordered to the east and west by Sudbury Fine Sandy Loam and Canton Fine Sandy Loam soils, which are mapped as Hydrologic Soil Class B and A, respectively.

In-situ soil testing was performed by DGT Associates in August of 2020, which was witnessed by the Holliston Board of Health. The results of the testing are contained in a memo from DGT Associates dated September 28, 2020, which is included in Attachment 1. A total of three test pits were performed along with two percolation tests and a permeability test. The test pits appeared to indicate the presence of sandy soil with rapidly permeable sands with ample offset to seasonal high groundwater (7-9'). Each of the percolation tests performed was <2 min/inch and the permeability test result



observed was 49.4 in/hr. As such the soil conditions appear to be very favorable for stormwater infiltration. According the DGT memo, the design RAWLS rate recommended for sand per the Massachusetts DEP Stormwater Handbook is 8.27 in/hr. However, for the hydrology calculations, a more conservative infiltration rate of 2.41 in/hr was used to not overestimate the infiltration occurring in the proposed rain gardens. An infiltration rate of 1.02 in/hr was used for the permeable paver base to account for slightly decreased performance over time.

# 2.3 Existing Hydrologic Analysis

Existing subcatchment areas were delineated based on existing runoff patterns and topographic information. This information is shown on the *Pre-Development Conditions Hydrologic Areas Map* included in Attachment 2. Summaries of each area with respect to Curve Number and peak runoff rates can be found in the model results also in Attachment 2.



# 3.0 POST-DEVELOPMENT CONDITIONS

## 3.1 Design Strategy

During the design of the project, consideration was given to appropriate grading and mitigation of stormwater runoff through the incorporation of various Low Impact Development (LID) strategies.

The proposed project includes improvements consisting of a plaza area and connected walkways comprised of permeable pavers, a picnic pavilion, granite block seating walls, a Veteran's Memorial, various new sitting benches and amenities, and replacement of an existing sidewalk along Central Street.

Overall, the proposed project will constitute a relatively minor increase in impervious area and runoff. To address this, a series of LID stormwater BMPs are proposed. The majority of the proposed hardscape will be the plaza area made from permeable pavers. These pavers will allow water to flow through the gaps between the pavers into a crushed stone base layer beneath to allow for infiltration of stormwater and recharge to groundwater. A covered pavilion area is also proposed and will have permeable pavers beneath and alongside the edge beyond the roof overhang, so that water from the roof can infiltrate through the pavers.

Management of stormwater runoff from the remaining areas of the site will accomplished through the implementation of three rain gardens situated at proposed low points along the east side of the trail. The rain gardens will provide multiple functions of runoff attenuation, infiltration/recharge to groundwater, total suspended solids (TSS) removal and total phosphorus removal (TP). While no impervious surfaces are proposed on the project which would require treatment under the MassDEP stormwater handbook (i.e. no required water quality treatment of paved impervious surfaces with pollutants), the TSS and TP removal functions still would be present given the biological functions of the proposed plantings in the rain gardens.

The proposed project has been designed to the extent applicable, in accordance with the requirements of the DEP Stormwater Handbook.

# 3.2 Proposed Hydrologic Analysis

The established design points used in the pre-development conditions analysis were used in the post-development analysis for direct comparison. The tributary areas and flow paths were modified to reflect post-development conditions. See Attachment 2 for the *Post- Development Conditions Hydrologic Areas Map*. Summaries of each area with respect to Curve Number, BMP modeling, and peak runoff rates can be found in the model results in Attachment 2.



# 3.3 Stormwater Management Controls Sizing

## Rain Gardens

The proposed rain gardens were sized in accordance with the guidance provided in Volume 2, Chapter 2 of the MassDEP Stormwater Handbook. The rain gardens are sized based on roughly 5% of the tributary area to each one, since not specifically required for recharge or water quality treatment.

The proposed rain gardens were modeled using the Simple Dynamic Method, as described in Chapter 3 of the Massachusetts Stormwater Handbook, using a conservative Rawl's exfiltration rate of 2.41 inches per hour. The rain gardens have been designed to fully dewater within 72 hours. Recharge and Drawdown calculations are provided in Attachment 3.

## Permeable Pavers

The proposed plaza area will be constructed of permeable pavers. The proposed footprint of the pavers is approximately 5,190 SF. The proposed stone layer beneath the pavers will be approximately 11-inches deep per the proposed cross-section. The stormwater storage available in the void space of the stone is approximately 1,427 CF assuming a 30% void ratio in the stone. This will provide much more than would be required by MassDEP for recharge from traditional solid impervious areas for Hydrologic Soil Class A sites (0.60 inches x paver surface area = 260 cf). The 1,427 CF of storage beneath the pavers equates to the volume produced by approximately 3.64 inches of rain over that area, which is just above a typical 2-year storm event of 3.10 inches for Middlesex County, MA.

The paver stone has been designed to fully dewater within 72 hours. In addition to exfiltration of water into the underlying soil, an underdrain pipe has also been designed to provide additional outflow from the stone base in the event it becomes saturated. This underdrain is routed into the base stone layer of Rain Garden-1. Recharge and Drawdown calculations are provided in Attachment 3.



# 3.4 Compliance with DEP Stormwater Management Standards

The proposed project was designed to the extent applicable with the ten (10) DEP Stormwater Management Standards. The following summary provides key information related to the proposed stormwater management BMPs and mitigation measures for stormwater runoff.

## **STANDARD 1:**

No new stormwater conveyance (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

There are no wetlands or waters on or near the site and there are no proposed stormwater outfalls; therefore this standard does not apply. However, please note that runoff from proposed hardscape surfaces and surficial runoff from grassed/landscaped areas will be infiltrated through the permeable paver stone base or directed to one of the three rain gardens for infiltration. No stormwater outfalls are proposed.

## **STANDARD 2:**

Stormwater management systems shall be designed so that postdevelopment peak discharge rates do not exceed pre-development peak discharge rates.

The stormwater management design will control post-development peak discharge rates for the 2-, 10-, 25-, 50- and 100-year, 24-hour storms so as to maintain pre-development peak discharge rates. Refer to Section 1.0 Introduction for a summary of the peak runoff rates.

## **STANDARD 3:**

Loss of annual recharge to groundwater shall be eliminated or minimized through the use of environmentally sensitive site design, low impact development techniques, stormwater management practices and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from pre-development conditions based on soil types. This Standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook.

The project includes a relatively small change in overall impervious area. The majority of the proposed plaza area will be constructed of permeable pavers with a porous crushed stone layer beneath the pavers that will allow for infiltration into the underlying soils. A granite block



sitting wall, several small concrete pads, and a brick memorial are the only proposed impervious surfaces, which are small in size and will produce a minimal amount of runoff which will be able to infiltrate into the adjacent soils. Similarly, roof runoff from the proposed pavilion will fall onto the permeable pavers below for infiltration. Ample recharge of stormwater will be provided in combination by the permeable paver stone base and three rain gardens.

#### **STANDARD 4:**

Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS).

The proposed project does not include impervious surfaces that would require treatment under the MassDEP Stormwater Handbook, so water quality treatment is not required. Despite this, TSS removal will still be provided by the proposed rain gardens by nature of their function.

### **STANDARD 5:**

For land uses with higher potential pollutant loads (LUHPPLs), source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable.

The proposed project is not associated with stormwater discharges from land uses with higher potential pollutant loads.

## **STANDARD 6:**

Stormwater discharges to critical areas must utilize certain stormwater management BMPs approved for critical areas. Critical areas are Outstanding Resource Waters, shellfish beds, swimming beaches, coldwater fisheries and recharge areas for public water supplies.

There are no stormwater discharges to critical areas associated with this project.

### STANDARD 7:

Redevelopment of previously developed sites must meet the Stormwater Management Standards to the maximum extent practicable. However, if it is not practicable to meet all the Standards, new (retrofitted or expanded) stormwater management systems must be designed to improve existing conditions.



Although the site is previously developed, this standard does not apply because the proposed project includes a small increase in impervious area.

STANDARD 8: A plan to control construction-related impacts during erosion,

sedimentation and other pollutant sources during construction and

land disturbance activities (construction period erosion,

sedimentation, and pollution prevention plan) shall be developed and

implemented.

A Construction Period Erosion, Sedimentation and Pollution Prevention Plan has been prepared and is included in Attachment 5; therefore the requirements of Standard 8 are fulfilled.

STANDARD 9: A Long-Term Operation and Maintenance (O&M) Plan shall be

developed and implemented to ensure that stormwater management

systems function as designed.

The Site Owner's Manual included in Attachment 4 complies with the Long-Term Pollution Prevention Plan (Standard 4) and the Long-Term Operation and Maintenance Plan (Standard 9) requirements of the 2008 Massachusetts Department of Environmental Protection (MassDEP) Stormwater Management Standards. The Manual outlines source control and pollution prevention measures and maintenance requirements of the stormwater best management practices (BMPs) associated with the proposed project.

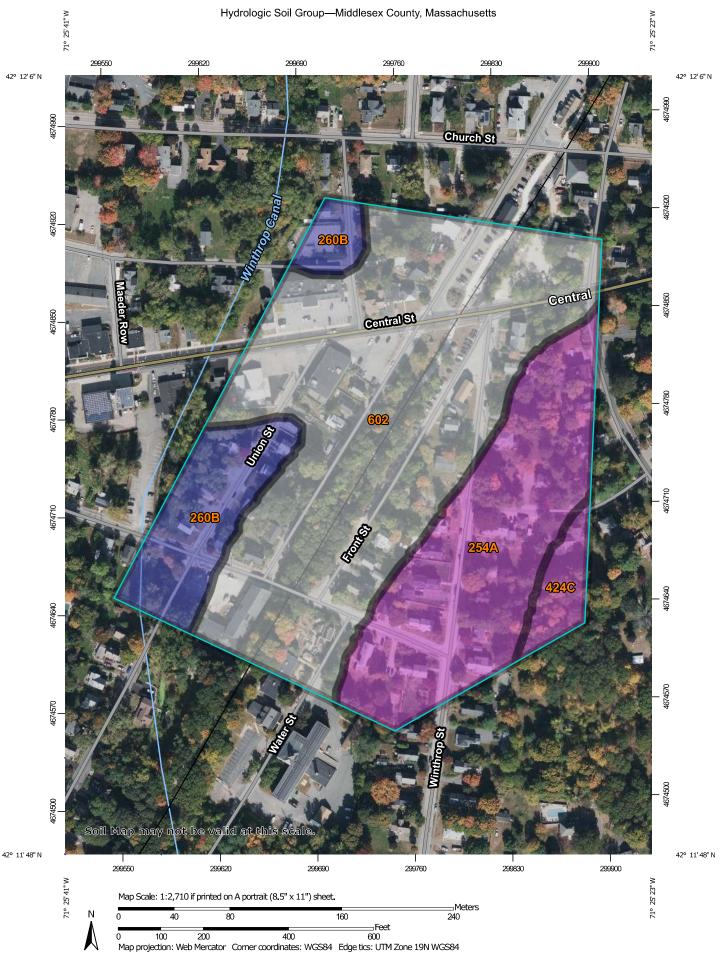
STANDARD 10: All illicit discharges to the stormwater management system are

prohibited.

There will be no illicit discharges to the proposed stormwater management system associated with the proposed project.



Attachment 1
Soil Data



MAP LEGEND

## Not rated or not available Streams and Canals Interstate Highways Aerial Photography Major Roads Local Roads US Routes Rails C/D Water Features Transportation ပ **Background** ŧ Not rated or not available Area of Interest (AOI) Soil Rating Polygons Area of Interest (AOI) Soil Rating Lines ΑP Ш Soils

# MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

contrasting soils that could have been shown at a more detailed misunderstanding of the detail of mapping and accuracy of soil Enlargement of maps beyond the scale of mapping can cause line placement. The maps do not show the small areas of scale

Please rely on the bar scale on each map sheet for map

measurements.

Coordinate System: Web Mercator (EPSG:3857) Web Soil Survey URL:

Source of Map: Natural Resources Conservation Service

distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Middlesex County, Massachusetts Survey Area Data: Version 20, Jun 9, 2020 Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Aug 31, 2020—Oct

Not rated or not available

B/D

Soil Rating Points

⋖

ΑD

B/D

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# **Hydrologic Soil Group**

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI						
254A	Merrimac fine sandy loam, 0 to 3 percent slopes	A	6.1	26.7%						
260B	Sudbury fine sandy loam, 3 to 8 percent slopes	В	2.8	12.5%						
424C	Canton fine sandy loam, 8 to 15 percent slopes, extremely bouldery	A	0.7	3.3%						
602	Urban land		13.1	57.6%						
Totals for Area of Intere	est	·	22.7	100.0%						

# **Description**

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

# **Rating Options**

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher





1071 Wordester Rd Framingham, MA 01703 508 879 0030 www.dgtassociates.com

September 28, 2020

Job: 25639

Town of Holliston – Board of Health Attn: Mr. Scott Moles – Health Director / Agent 703 Washington Street Holliston, MA 01746

RE: Blair Square in Holliston, MA 01746 - Soil Testing

Dear Mr. Moles,

This report contains the results of the on-site soil testing conducted by DGT Associates on September 23, 2020 at the subject site. The testing consisted of three (3) deep observation holes (AKA test pits), two (2) percolation tests, and one (1) permeability test. Rodenhiser Excavating, Septic & Drains and Builders provided the backhoe service. A Soil Test Hole Location Plan is contained in Attachment 3.

The purpose of the testing was to assess the suitability of the soils for the design of a soil absorption system (SAS) and stormwater infiltration systems for the proposed site modifications. Testing was performed by Massachusetts Licensed Soil Evaluator (Joseph A. Losanno, EIT) of DGT and witnessed by Scott Moles from the Town of Holliston Board of Health.

According to the Natural Resources Conservation Service (NRCS) Soils Mapping, the soil in the area of testing is Urban Land. Attachment 2 contains the NRCS Map for the site and descriptions of the soil type.

Generally, the testing revealed a thin topsoil mixed with fill and trash, over a fine sand subsoil, over a fine sand substratum. No signs of Estimated Seasonal High Groundwater Table (ESHGWT), including redoximorphic features, weeping or standing groundwater, were observed during our testing. However, root intrusion at approximately seven to nine feet (7-9') below grade at test hole TH 20-03 were observed. The elevation root intrusion appears to match surrounding sites design ESHGWT. A monitor well was installed in this test hole for future monitoring of the groundwater table. At this time the bottom of the test holes are assumed to be the design ESHGWT. The deep observation hole logs are contained in Attachment 1.

Ledge / Bedrock / Refusal was not observed during the soil exploration.

Two (2) percolation tests were performed within the fine sand substratum layers adjacent to test pits TH 20-01 and TH 20-03. The resulting percolation rates were both less than two (2) minutes per inch. The percolation test logs are contained at the end of Attachment 1.

A permeability test utilizing the US Army Corps of Engineers method "In-situ Permeability Testing in the Vadose Zone" was performed in the substratum of test pit TH 20-02. The permeability rate observed was 49.4 inches per hour. The design permeability rate that will be used for stormwater infiltration in the area of these test pits will be based on the RAWLS rate published in the Massachusetts Stormwater Handbook for sand material (8.27 inches per hour). The tabulation of this test is included at the end of Attachment 1.



1071 Worcester Rd. Framingham, MA 01701 508.879.0030 www.dgtassociates.com

Please contact me if you have any questions regarding this report.

Sincerely, DGT Associates

Joseph A. Losanno, EIT (SE 13870) Project Engineer

## Attachments:

- 1. Form 11 & Form 12 & Permeability Test Log
- 2. NRCS Soils Map & Information
- 3. Soil Test Hole Location Plan
- 4. USGS Surficial Geology Map & Information

Location Address or Lot No. Blair Square - Holliston, MA 01746

# On-site Witnessed Soil Testing Review

	ep Hole Num ation (identif		TH 20-0 <sup>-</sup> ite plan)	Date: _(	08/23/2020	)1	ime: _	A.M.	Weather	60° Sunny
Lan	Land Use Walking Trail Slope (%) 0 - 3 Surface Stones None									
Veg	etationL	_awn / ˈ	Wooded		2					
Lan	dform C	Ground	l Moraines							
Pos	ition on land	dscape	(sketch o	n the back)	see sket	ch				
Dist	ances from:									
	Open W	Vater B	ody	See sketch	Feet	Drainag	jeway	See sk	ketch Feet	
	Possible	e Wet	Area	See sketch	Feet	Propert	y Line	See sk	ketch Feet	
	Drinking	g Wate	r Well	See sketch	Feet	Other				
				DEEP (	OBSE	RVATIO	ON H	OLE	LOG	
	Depth fror Surface (incl		Soil Horizon	Soil Texture	(USDA)	Soil Color (Munsell)	Soil Mo	ottling	(Structure, St	other cones, Boulders, cy, % Gravel)
	0 – 11"		A / Fill	Sandy Lo	am	10 YR 4/2	None Ob	oserved	Massive-Friable – mix roots, and trash	of topsoil w/ brush,
	11 – 120"	,	С	Sand (Fi	ne)	2.5 Y 7/2	None Ob	oserved	Loose-Single Grained	

Parent Material (geologic)	Excavated a	nd filled land		Depth to Bedi	Depth to Bedrock:	
Depth to Groundwater:	Standing Water	er in the Hole:	None Observed	Weeping from Pit Face:	Nor	ne Observed
Estimated Seasonal High G	round Water:	> 120" below gr	rade			



Location Address or Lot No.	Blair Square - Holliston, MA 01746
20041101111441005 01 201110.	Dian Oquale — Homston, MA 01740

# On-site Witnessed Soil Testing Review

Deep Hole N	lumber <u>TH 20-0</u>	2 Date:	08/23/2020	Time:	A.M.	Weather		60° Sunny	
Location (ide	entify on site plan)	see sketch							
Land Use	Walking Trail	Slope	(%) 0 - 3	Surface St	ones	None			
Vegetation	Lawn / Wooded								
Landform	Ground Moraines								
Position on la	andscape (sketch o	n the back)	see sketch						
Distances fro	om:								
Ope	n Water Body	See sketch	Feet	Drainageway	See s	ketch	Feet		
Poss	sible Wet Area	See sketch	Feet	Property Line	See s	ketch	Feet		
Drink	king Water Well	See sketch	Feet	Other			2.		
		DEED	ODCEDY	VATIONIA	IOLI	FIOC			

DEEP OBSERVATION HOLE LOG									
Depth from Surface (Inches)	Soll Horizon	Soil Texture (USDA)	Soll Color (Munsell)	Soil Mottling	Other (Structure, Stones, Boulders, Consistency, % Gravel)				
0 – 8"	A / Fill	Sandy Loam	10 YR 4/2	None Observed	Massive-Friable – mix of topsoil w/ brush, roots, and trash				
8 – 30"	B <sub>w</sub>	Sand (Fine)	10 YR 7/3	None Observed	Loose-Single Grained w/ some roots				
30 – 130"	С	Sand (Fine)	2.5 Y 7/2	None Observed	Loose-Single Grained				

Parent Material (geologic)	Excavated and filled land		Depth to Bedi	rock: None Observed
Depth to Groundwater:	Standing Water in the Hole:	None Observed	Weeping from Pit Face:	None Observed
Estimated Seasonal High Gre	ound Water: > 130" below	grade		



roots, and trash

below grade)

Loose-Single Grained

Loose-Single Grained (root intrusion @ 7-9'

Location Address or Lot No. Blair Square - Holliston, MA 01746

# On-site Witnessed Soil Testing Review

Dee	p Hole N	umber	TH 20-03	B Date:	08/23/2020	)	Γime:	A.M.	Weather		60° Sunny	
Loc	ation (ide	ntify on s	site plan)	see sketch								
Lan	d Use	Walking	Trail	Slope	e (%) 0 - 3	Su Su	rface St	ones	None			
Veg	etation	Lawn /	Wooded									
Lan	andform Ground Moraines											
Pos	Position on landscape (sketch on the back) see sketch											
Dist	ances fro	m:										
	Open	Water E	3ody _	See sketch	Feet	Drainaç	geway	See sl	ketch	Feet		
	Possi	ible Wet	Area	See sketch	Feet	Propert	y Line	See sl	ketch	Feet		
	Drink	ing Wate	er Well	See sketch	Feet	Other						
-												
				DEEP	OBSE	RVATIO	ON F	HOLE	LOG			
	Depth f		Soil	Soil Textur	e (USDA)	Soil Color	Soil N	ottling		Otl	her	
	Surface (i	nches)	Horizon			(Munsell)			· ·		nes, Boulders, y, % Gravel)	
	0 – 1	2"	A / Fill	Sandy I	Loam	10 YR 4/2	None C	Observed			of topsoil w/ brush,	

Parent Material (geologic)	Excavated and filled land		Depth to Bedro	ck: None Observed
Depth to Groundwater:	Standing Water in the Hole:	None Observed	Weeping from Pit Face:	None Observed
Estimated Seasonal High Gr	round Water: Inconclusive -	Monitor well installed f	or future monitoring	

10 YR 6/4

10 YR 7/4

None Observed

Inconclusive



 $B_w/C_1$ 

 $C_2$ 

Sand (Fine - Medium)

Sand (Fine)

12 - 40"

40 - 118"



# Commonwealth of Massachusetts City/Town of Holliston Percolation Test Form 12

Percolation test results must be submitted with the Soil Suitability Assessment for On-site Sewage Disposal. DEP has provided this form for use by local Boards of Health. Other forms may be used, but the information must be substantially the same as that provided here. Before using this form, check with the local Board of Health to determine the form they use. **A. Site Information** 

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key



Owner Name		
Blair Square		
Street Address or Lot #		
Holliston	MA	01746
City/Town	State	Zip Code

508-879-0030

Telephone Number

## **B. Test Results**

**DGT** Associates

Contact Person (if different from Owner)

	Date	Time	Date	Time	
Observation Hole #	TH 20-01		TH 20-03		
Depth of Perc	36 - 48"		38 - 50"		
Start Pre-Soak	10:30 A.M.		11:06 A.M.		
End Pre-Soak	10:45 P.M.		11:21 A.M.		
Time at 12"	10:45 A.M.		11:21 A.M.		
Time at 9"	10:47 A.M.		11:23 A.M.		
Time at 6"	10:49 A.M.		11:26 A.M.		
Time (9"-6")	2 minutes		3 minutes		
Rate (Min./Inch)	<2 minutes per	inch	<2 Minutes per Inch		
	Test Passed: Test Failed:		Test Passed:		
Joseph Losanno, E.I.T.	rest railed.		Test Failed:	LJ	
Test Performed By:					
Scott Moles					
Nitnessed By:					
Comments:					

# Permeability Test @ TH 20-02

Date Performed:

23-Sep-20

Soil Horizon of Perm Test:

C

Depth to water level =

24"

Depth to bottom of tube =

35"

Start Soak:

11:50 A.M.

Start Test:

12:05 P.M.

	Time Interval	Incrementa
	(Minutes)	Volume(L)
	1	1.500
	1	1.000
	1	14.250
	1	1.250
	1	1.000
	1	1.000
	1	1.000
	1	1.000
	1	1.250
	1	1.250
umulative Time/Volume	10	24.500

Cı

Q=Cumulative Volume cm<sup>3</sup> / Total time in seconds

Q=

40.833 cm<sup>3</sup>/sec

Computation of Permeability(k)

k=Q / 5.5 r Hw=

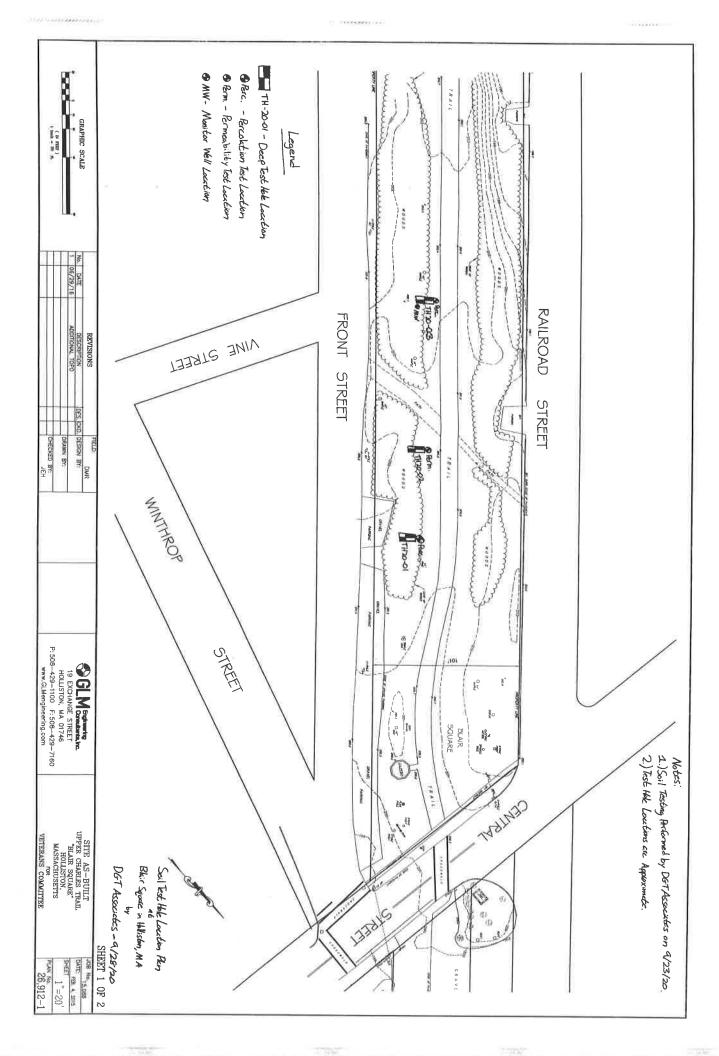
k=coefficient of permeability (cm/sec) r=inside radius of pipe in centimeters= Hw=applied head in centimeters= Q=Computed flow rate in CC/sec=

7.6 (6" DIA.) 28 cm (11 inches) 40.833 cm<sup>3</sup>/sec

k=Q / 5.5 r Hw=

0.03489 cm/sec

49.448 IN/HR



Attachment 2
Hydrologic Analysis





## PRE- & POST-DEVELOPMENT CALCULATION SUMMARY

#### **OBJECTIVE**

To determine the pre- and post-development peak rates of runoff from the site for the 2, 10, 25, 50, & 100-year storm events at the design points.

## CONCLUSION(S)

## Peak Runoff Rates (CFS):

Ctorm Front	2 Y	ear	10 \	/ear	25 \	/ear	50 \	/ear	100	Year
Storm Event	Pre	Post								
Design Point 1	0.36	0.07	1.03	0.16	1.47	0.27	1.83	0.35	2.51	0.57
Design Point 2	0.19	0.09	0.56	0.32	0.81	0.48	1.00	0.61	1.39	0.86
Design Point 3	0.09	0.05	0.23	0.17	0.33	0.26	0.41	0.32	0.55	0.46

**Conclusion:** Overall runoff rates from the project area will be less than or equal to pre-development conditions in accordance with Standard 2 of the MassDEP Stormwater Management Regulations.

### **CALCULATION METHODS**

- 1. Runoff curve numbers (CN), time-of-concentration (Tc), and runoff rates were calculated based on TR-55 methodology.
- 2. AutoCAD 2019 computer program was utilized for digitizing ground cover areas.
- 3. Peak runoff rates were computed using HydroCAD version 10.10

#### **ASSUMPTIONS**

- 1. The ground cover types were determined using aerial imagery. Hydrologic soil class B was used for modeling based on a review of surrounding soil types as shown on the United States Department of Agriculture, NRCS Soil Survey map information. Hydrologic soil group B was used for more conservative sizing of the proposed stormwater management BMPs.
- 2. An exfiltration rate of 2.41 in/hr was used for the rain garden modeling despite the higher infiltration rates observed during in-situ testing as to not overestimate the infiltrative capacity of the soils over time. A rate of 1.02 in/hr was assumed beneath the permeable pavers as it is anticipated there may be some slight compaction of the soils resulting from the installation process.

## **SOURCES OF DATA/ EQUATIONS**

- 1. Pre-and Post-Development Conditions Hydrologic Areas Maps prepared by Beals and Thomas, Inc. File No. 143903P005A-001-002.
- 2. NRCS Soil Survey for Plymouth County, hydrologic soil group report, downloaded from Web Soil Survey on 08/31/2021.
- 3. TR-55 urban Hydrology for Small Watersheds, SCS, 1986.
- 4. Massachusetts DEP Stormwater Management Handbook, February 2008.

## LIST OF APPENDICES

- A. Pre-Development Conditions Hydrologic Analysis
- B. Post-Development Conditions Hydrologic Analysis

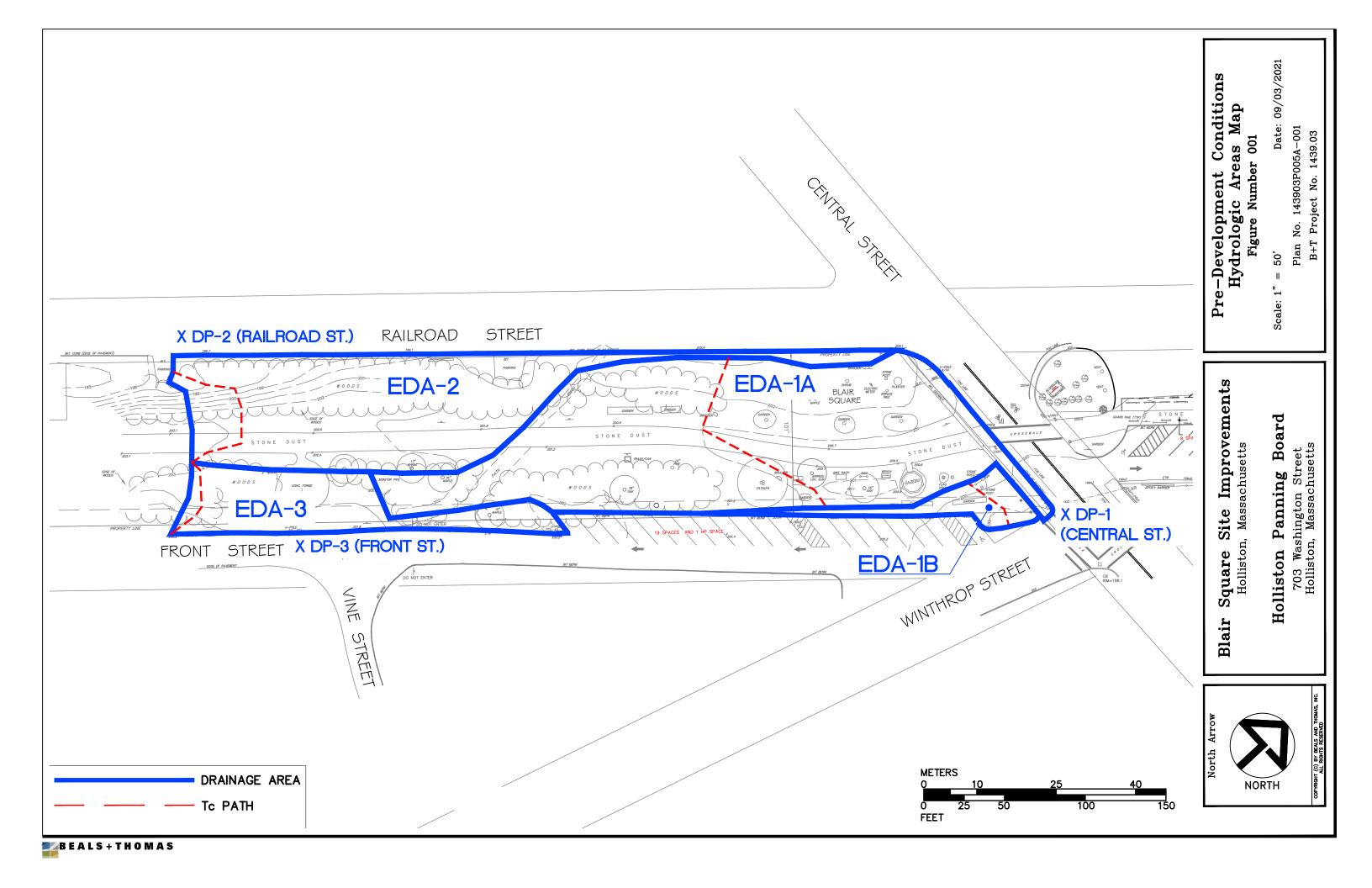
REV	CALC. BY	DATE	CHECKED BY	DATE	APPROVED BY	DATE
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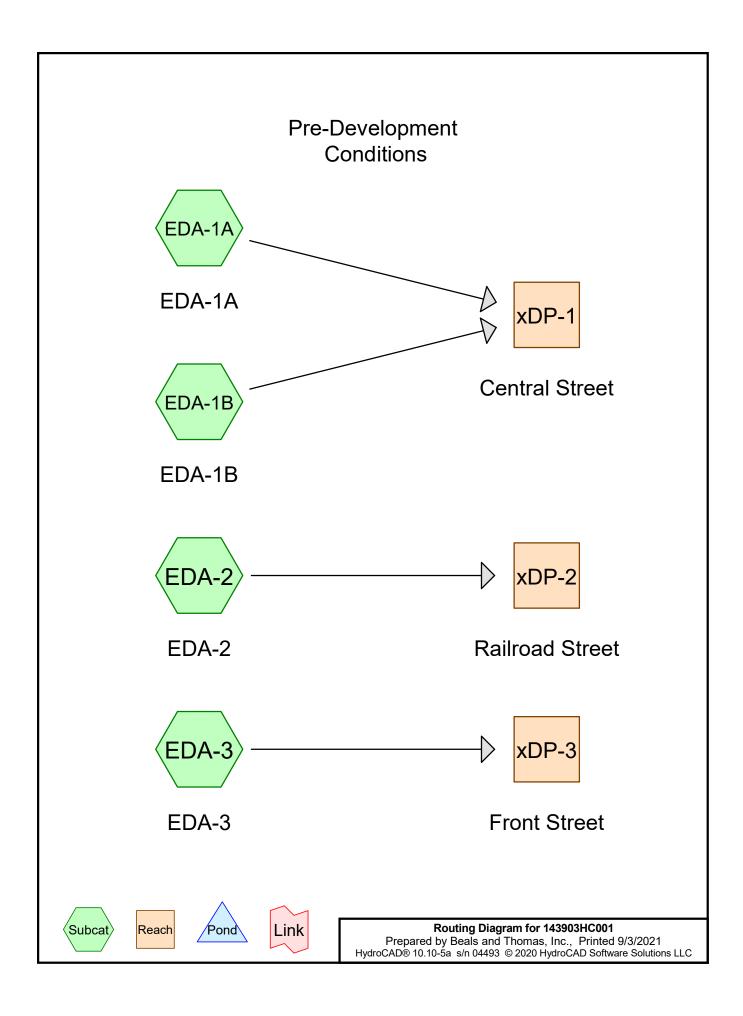
NBB/JRM/143903CS001A

# Appendix A

Pre-Development Conditions Hydrologic Analysis







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# **Area Listing (selected nodes)**

Area	CN	Description
(acres)		(subcatchment-numbers)
0.724	61	>75% Grass cover, Good, HSG B (EDA-1A, EDA-1B, EDA-2, EDA-3)
0.053	96	Compacted Earth, HSG B (EDA-1A, EDA-2, EDA-3)
0.017	98	Impervious Surface, HSG B (EDA-1A, EDA-2)
0.003	98	Roofs, HSG B (EDA-1A)
0.131	96	Stone Dust Path, HSG B (EDA-1A, EDA-2)
0.281	55	Woods, Good, HSG B (EDA-1A, EDA-2, EDA-3)
1.209	66	TOTAL AREA

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1A: EDA-1A Runoff Area=0.633 ac 2.05% Impervious Runoff Depth>0.59"

Tc=6.0 min CN=66 Runoff=0.35 cfs 0.031 af

Subcatchment EDA-1B: EDA-1B Runoff Area=0.048 ac 0.00% Impervious Runoff Depth>0.40"

Tc=6.0 min CN=61 Runoff=0.01 cfs 0.002 af

Subcatchment EDA-2: EDA-2 Runoff Area=0.385 ac 1.82% Impervious Runoff Depth>0.55"

Tc=6.0 min CN=65 Runoff=0.19 cfs 0.018 af

Subcatchment EDA-3: EDA-3 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth>0.63"

Tc=6.0 min CN=67 Runoff=0.09 cfs 0.008 af

Reach xDP-1: Central Street Inflow=0.36 cfs 0.033 af

Outflow=0.36 cfs 0.033 af

Reach xDP-2: Railroad Street Inflow=0.19 cfs 0.018 af

Outflow=0.19 cfs 0.018 af

Reach xDP-3: Front Street Inflow=0.09 cfs 0.008 af

Outflow=0.09 cfs 0.008 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.058 af Average Runoff Depth = 0.58" 98.35% Pervious = 1.189 ac 1.65% Impervious = 0.020 ac

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# Summary for Subcatchment EDA-1A: EDA-1A

Runoff = 0.35 cfs @ 12.11 hrs, Volume= 0.031 af, Depth> 0.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

Area	(ac)	CN	Desc	cription					
0.	111	55	Woo	/oods, Good, HSG B					
0.	417	61	>759	% Grass co	over, Good,	, HSG B			
0.	016	96	Com	pacted Ea	rth, HSG B	3			
0.	076	96	Ston	e Dust Pat	h, HSG B				
0.	010	98	Impe	ervious Sui	face, HSG	BB			
0.	003	98	Roof	s, HSG B					
0.	0.633 66 Weighted Average				age				
0.	620		97.9	5% Pervio	us Area				
0.	013		2.05	% Impervi	ous Area				
Tc	Leng	th	Slope	Velocity	Capacity	Description			
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)				
6.0						Direct Entry, Tc Min.			
	0. 0. 0. 0. 0. 0. 0. Tc (min)	(min) (fee	0.111 55 0.417 61 0.016 96 0.076 96 0.010 98 0.003 98 0.633 66 0.620 0.013  Tc Length (min) (feet)	0.111 55 Woo 0.417 61 >759 0.016 96 Com 0.076 96 Ston 0.010 98 Impe 0.003 98 Roof 0.633 66 Weig 0.620 97.9 0.013 2.05  Tc Length Slope (min) (feet) (ft/ft)	0.111         55         Woods, Good, O.417         61         >75% Grass cool,	0.111         55         Woods, Good, HSG B           0.417         61         >75% Grass cover, Good           0.016         96         Compacted Earth, HSG B           0.076         96         Stone Dust Path, HSG B           0.010         98         Impervious Surface, HSG           0.003         98         Roofs, HSG B           0.633         66         Weighted Average           0.620         97.95% Pervious Area           0.013         2.05% Impervious Area           Tc         Length         Slope         Velocity         Capacity           (min)         (feet)         (ft/ft)         (ft/sec)         (cfs)			

# **Summary for Subcatchment EDA-1B: EDA-1B**

Runoff = 0.01 cfs @ 12.13 hrs, Volume= 0.002 af, Depth> 0.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

Area	(ac)	CN	Desc	ription		
0.	.048	61	>75%	√ Grass co	over, Good	, HSG B
0.	.048		100.0	00% Pervi	ous Area	
Tc (min)	Leng		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0						Direct Entry, Tc Min.

# **Summary for Subcatchment EDA-2: EDA-2**

Runoff = 0.19 cfs @ 12.11 hrs, Volume= 0.018 af, Depth> 0.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

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	Area (	ac)	CN	Desc	cription		
	0.1	131	55	Woo	ds, Good,	HSG B	
	0.1	188	61	>759	% Grass co	ver, Good,	, HSG B
*	0.0	004	96	Com	pacted Ea	rth, HSG B	3
*	0.0	)55	96	Ston	e Dust Pat	h, HSG B	
*	0.0	007	98	Impe	rvious Sur	face, HSG	В
	0.3	385	65	Weig	ghted Aver	age	
	0.3	378		98.1	8% Pervio	us Area	
	0.0	007		1.82	% Impervi	ous Area	
	Тс	Lengi	th	Slope	Velocity	Capacity	Description
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

# **Summary for Subcatchment EDA-3: EDA-3**

Runoff = 0.09 cfs @ 12.11 hrs, Volume= 0.008 af, Depth> 0.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

	Area	(ac)	CN	Desc	cription						
	0.	039	55	Woo	ds, Good,	HSG B					
	0.	071	61	>759	% Grass cover, Good, HSG B						
*	0.	033	96	Com	pacted Ea	rth, HSG B	3				
	0.	143	67								
	0.	0.143 67 Weighted Average 0.143 100.00% Pervious Area				ous Area					
	Tc (min)	Leng (fe	,	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	6.0						Direct Entry, Tc Min.				

# **Summary for Reach xDP-1: Central Street**

Inflow Are	a =	0.681 ac,	1.91% Impervious, Infl	ow Depth > 0.58"	for Middlesex-002yr event
Inflow	=	0.36 cfs @	12.11 hrs, Volume=	0.033 af	·
Outflow	=	0.36 cfs @	12.11 hrs, Volume=	0.033 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

# Summary for Reach xDP-2: Railroad Street

Inflow Are	ea =	0.385 ac,	1.82% Impervious, Inflow I	Depth > 0.55"	for Middlesex-002yr event
Inflow	=	0.19 cfs @	12.11 hrs, Volume=	0.018 af	
Outflow	=	0.19 cfs @	12.11 hrs, Volume=	0.018 af, Att	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

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# **Summary for Reach xDP-3: Front Street**

Inflow Area = 0.143 ac, 0.00% Impervious, Inflow Depth > 0.63" for Middlesex-002yr event

Inflow = 0.09 cfs @ 12.11 hrs, Volume= 0.008 af

Outflow = 0.09 cfs @ 12.11 hrs, Volume= 0.008 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1A: EDA-1A Runoff Area=0.633 ac 2.05% Impervious Runoff Depth>1.39"

Tc=6.0 min CN=66 Runoff=0.97 cfs 0.074 af

Subcatchment EDA-1B: EDA-1B Runoff Area=0.048 ac 0.00% Impervious Runoff Depth>1.08"

Tc=6.0 min CN=61 Runoff=0.05 cfs 0.004 af

Subcatchment EDA-2: EDA-2 Runoff Area=0.385 ac 1.82% Impervious Runoff Depth>1.33"

Tc=6.0 min CN=65 Runoff=0.56 cfs 0.043 af

Subcatchment EDA-3: EDA-3 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth>1.46"

Tc=6.0 min CN=67 Runoff=0.23 cfs 0.017 af

Reach xDP-1: Central Street Inflow=1.03 cfs 0.078 af

Outflow=1.03 cfs 0.078 af

Reach xDP-2: Railroad Street Inflow=0.56 cfs 0.043 af

Outflow=0.56 cfs 0.043 af

Reach xDP-3: Front Street Inflow=0.23 cfs 0.017 af

Outflow=0.23 cfs 0.017 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.138 af Average Runoff Depth = 1.37" 98.35% Pervious = 1.189 ac 1.65% Impervious = 0.020 ac Prepared by Beals and Thomas, Inc.

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# **Summary for Subcatchment EDA-1A: EDA-1A**

Runoff = 0.97 cfs @ 12.10 hrs, Volume= 0.074 af, Depth> 1.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

Area	(ac)	CN	Desc	cription			
0.	111	55	Woo	ds, Good,	HSG B		
0.	417	61	>759	% Grass co	over, Good	, HSG B	
0.	016	96	Com	pacted Ea	rth, HSG B	3	
0.	076	96	Ston	e Dust Pat	th, HSG B		
0.	010	98	Impe	ervious Sui	face, HSG	i B	
0.	003	98	Root	s, HSG B			
0.	0.633 66 Weighted Average				age		
0.	620		97.9	5% Pervio	us Area		
0.	013		2.05	% Impervi	ous Area		
Тс	_		•	,		Description	
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)		
6.0						Direct Entry, Tc Min.	
	0. 0. 0. 0. 0. 0. Tc (min)	(min) (fee	0.111 55 0.417 61 0.016 96 0.076 96 0.010 98 0.003 98 0.633 66 0.620 0.013  Tc Length (min) (feet)	0.111 55 Woo 0.417 61 >759 0.016 96 Com 0.076 96 Ston 0.010 98 Impe 0.003 98 Roof 0.633 66 Weig 0.620 97.9 0.013 2.05  Tc Length Slope (min) (feet) (ft/ft)	0.111         55         Woods, Good, O.417         61         >75% Grass cool         0.417         61         >75% Grass cool         0.016         96         Compacted Ea         0.076         96         Stone Dust Pate         0.010         98         Impervious Sure         0.003         98         Roofs, HSG B         Neighted Averence         0.633         66         Weighted Averence         0.620         97.95% Pervious         0.013         2.05% Impervious         2.05% Impervious         To Length         Slope         Velocity         (ft/ft)         (ft/sec)	0.111         55         Woods, Good, HSG B           0.417         61         >75% Grass cover, Good           0.016         96         Compacted Earth, HSG B           0.076         96         Stone Dust Path, HSG B           0.010         98         Impervious Surface, HSG           0.003         98         Roofs, HSG B           0.633         66         Weighted Average           0.620         97.95% Pervious Area           0.013         2.05% Impervious Area           Tc         Length         Slope         Velocity         Capacity           (min)         (feet)         (ft/ft)         (ft/sec)         (cfs)	0.111         55         Woods, Good, HSG B           0.417         61         >75% Grass cover, Good, HSG B           0.016         96         Compacted Earth, HSG B           0.076         96         Stone Dust Path, HSG B           0.010         98         Impervious Surface, HSG B           0.003         98         Roofs, HSG B           0.633         66         Weighted Average           0.620         97.95% Pervious Area           0.013         2.05% Impervious Area           Tc         Length         Slope         Velocity         Capacity         Description           (min)         (feet)         (ft/ft)         (ft/sec)         (cfs)

# **Summary for Subcatchment EDA-1B: EDA-1B**

Runoff = 0.05 cfs @ 12.10 hrs, Volume= 0.004 af, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

Area	(ac)	CN	Desc	ription			
0.	.048	61	>75%	√ Grass co	over, Good	, HSG B	
0.	0.048 100.00% Pervious Area						
Tc (min)	Leng		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.0						Direct Entry, Tc Min.	

# Summary for Subcatchment EDA-2: EDA-2

Runoff = 0.56 cfs @ 12.10 hrs, Volume= 0.043 af, Depth> 1.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

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	Area (ac) CN Description									
	0.1	131	55	Woo	ds, Good,	HSG B				
	0.1	188	61	>759	>75% Grass cover, Good, HSG B					
*	0.0	004	96	Com	Compacted Earth, HSG B					
*	0.0	)55	96	Ston	Stone Dust Path, HSG B					
*	0.0	007	98	Impe	Impervious Surface, HSG B					
	0.3	385	65	Weig	ghted Aver	age				
	0.378			98.18% Pervious Area						
	0.007			1.82	% Impervi	ous Area				
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	6.0						Direct Entry, Tc Min.			

# **Summary for Subcatchment EDA-3: EDA-3**

Runoff = 0.23 cfs @ 12.10 hrs, Volume= 0.017 af, Depth> 1.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

	Area (ac) CN			Desc	Description				
0.039 55 Woods, Good, HSG B									
0.071 61 >75% Grass cover, Good, HSG B						, HSG B			
*	0.	0.033 96 Compacted Earth, HSG B							
	0.	0.143 67 Weighted Average							
0.143				100.00% Pervious Area					
	Tc (min)	3		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	6.0						Direct Entry, Tc Min.		

# **Summary for Reach xDP-1: Central Street**

Inflow Area = 0.681 ac, 1.91% Impervious, Inflow Depth > 1.37" for Middlesex-010yr event Inflow = 1.03 cfs @ 12.10 hrs, Volume= 0.078 af Outflow = 1.03 cfs @ 12.10 hrs, Volume= 0.078 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

# Summary for Reach xDP-2: Railroad Street

Inflow Area = 0.385 ac, 1.82% Impervious, Inflow Depth > 1.33" for Middlesex-010yr event Inflow = 0.56 cfs @ 12.10 hrs, Volume= 0.043 af Outflow = 0.56 cfs @ 12.10 hrs, Volume= 0.043 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

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## Summary for Reach xDP-3: Front Street

Inflow Area = 0.143 ac, 0.00% Impervious, Inflow Depth > 1.46" for Middlesex-010yr event

Inflow = 0.23 cfs @ 12.10 hrs, Volume= 0.017 af

Outflow = 0.23 cfs @ 12.10 hrs, Volume= 0.017 af, Atten= 0%, Lag= 0.0 min

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1A: EDA-1A Runoff Area=0.633 ac 2.05% Impervious Runoff Depth>1.93"

Tc=6.0 min CN=66 Runoff=1.39 cfs 0.102 af

Subcatchment EDA-1B: EDA-1B Runoff Area=0.048 ac 0.00% Impervious Runoff Depth>1.55"

Tc=6.0 min CN=61 Runoff=0.08 cfs 0.006 af

**Subcatchment EDA-2: EDA-2** Runoff Area=0.385 ac 1.82% Impervious Runoff Depth>1.85"

Tc=6.0 min CN=65 Runoff=0.81 cfs 0.059 af

Subcatchment EDA-3: EDA-3 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth>2.01"

Tc=6.0 min CN=67 Runoff=0.33 cfs 0.024 af

Reach xDP-1: Central Street Inflow=1.47 cfs 0.108 af

Outflow=1.47 cfs 0.108 af

Reach xDP-2: Railroad Street Inflow=0.81 cfs 0.059 af

Outflow=0.81 cfs 0.059 af

Reach xDP-3: Front Street Inflow=0.33 cfs 0.024 af

Outflow=0.33 cfs 0.024 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.192 af Average Runoff Depth = 1.90" 98.35% Pervious = 1.189 ac 1.65% Impervious = 0.020 ac

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## **Summary for Subcatchment EDA-1A: EDA-1A**

Runoff = 1.39 cfs @ 12.09 hrs, Volume= 0.102 af, Depth> 1.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

_	Area	(ac)	CN	Desc	cription						
	0.	111	55	Woo	ds, Good,	HSG B					
	0.	417	61	>759	% Grass co	over, Good	, HSG B				
*	0.	016	96	96 Compacted Earth, HSG B							
*	0.	076	96	Ston	e Dust Pa	th, HSG B					
*	0.	010	98	Impe	ervious Su	face, HSG	В				
	0.	003	98	Root	s, HSG B						
	0.	633	66	Weig	ghted Aver	age					
	0.	620		97.9	5% Pervio	us Area					
	0.	013		2.05	% Impervi	ous Area					
	Тс	Leng	th	Slope	Velocity	Capacity	Description				
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0						Direct Entry, Tc Min.				

### **Summary for Subcatchment EDA-1B: EDA-1B**

Runoff = 0.08 cfs @ 12.10 hrs, Volume= 0.006 af, Depth> 1.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

	Area	(ac)	CN	Desc	cription					
	0.	0.048 61 >75% Grass cover, Good, HSG B								
	0.	048		100.0	00% Pervi	ous Area				
_	Tc (min)	Leng		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	6.0						Direct Entry, Tc Min.			

## Summary for Subcatchment EDA-2: EDA-2

Runoff = 0.81 cfs @ 12.09 hrs, Volume= 0.059 af, Depth> 1.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

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	Area (a	ac)	CN	Desc	cription								
	0.1	31	55	Woo	Voods, Good, HSG B								
	0.1	88	61	>75%	% Grass co	over, Good,	I, HSG B						
*	0.0	04	96	Com	pacted Ea	rth, HSG B	3						
*	0.0	)55	96	Ston	e Dust Pat	th, HSG B							
*	0.0	07	98	Impe	rvious Sui	face, HSG	BB						
<u> </u>	0.3	85	65	Weig	ghted Aver	age							
	0.3	378		98.1	8% Pervio	us Area							
	0.0	07		1.82	% Impervi	ous Area							
	Tc	Lengt		Slope	Velocity	Capacity	Description						
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)							
	6.0						Direct Entry, Tc Min.						

### **Summary for Subcatchment EDA-3: EDA-3**

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 0.024 af, Depth> 2.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

	Area	(ac)	CN	Desc	Description							
	0.	0.039 55 Woods, Good, HSG B										
	0.071 61 >75% Grass cover, Good, HSG B											
*	0.	0.033 96 Compacted Earth, HSG B										
	0.	0.143 67 Weighted Average										
	0.	143		100.	00% Pervi	ous Area						
	Tc (min)	Leng (fe	,	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	6.0						Direct Entry, Tc Min.					

### **Summary for Reach xDP-1: Central Street**

Inflow Area = 0.681 ac, 1.91% Impervious, Inflow Depth > 1.91" for Middlesex-025yr event Inflow = 1.47 cfs @ 12.09 hrs, Volume= 0.108 af Outflow = 1.47 cfs @ 12.09 hrs, Volume= 0.108 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Summary for Reach xDP-2: Railroad Street

Inflow Area = 0.385 ac, 1.82% Impervious, Inflow Depth > 1.85" for Middlesex-025yr event 
Inflow = 0.81 cfs @ 12.09 hrs, Volume= 0.059 af 
Outflow = 0.81 cfs @ 12.09 hrs, Volume= 0.059 af, Atten= 0%, Lag= 0.0 min

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## Summary for Reach xDP-3: Front Street

Inflow Area = 0.143 ac, 0.00% Impervious, Inflow Depth > 2.01" for Middlesex-025yr event

Inflow = 0.33 cfs @ 12.09 hrs, Volume= 0.024 af

Outflow = 0.33 cfs @ 12.09 hrs, Volume= 0.024 af, Atten= 0%, Lag= 0.0 min

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1A: EDA-1A Runoff Area=0.633 ac 2.05% Impervious Runoff Depth>2.36"

Tc=6.0 min CN=66 Runoff=1.72 cfs 0.125 af

Subcatchment EDA-1B: EDA-1B Runoff Area=0.048 ac 0.00% Impervious Runoff Depth>1.94"

Tc=6.0 min CN=61 Runoff=0.10 cfs 0.008 af

Subcatchment EDA-2: EDA-2 Runoff Area=0.385 ac 1.82% Impervious Runoff Depth>2.28"

Tc=6.0 min CN=65 Runoff=1.00 cfs 0.073 af

Subcatchment EDA-3: EDA-3 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth>2.45"

Tc=6.0 min CN=67 Runoff=0.41 cfs 0.029 af

Reach xDP-1: Central Street Inflow=1.83 cfs 0.132 af

Outflow=1.83 cfs 0.132 af

Reach xDP-2: Railroad Street Inflow=1.00 cfs 0.073 af

Outflow=1.00 cfs 0.073 af

Reach xDP-3: Front Street Inflow=0.41 cfs 0.029 af

Outflow=0.41 cfs 0.029 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.235 af Average Runoff Depth = 2.33" 98.35% Pervious = 1.189 ac 1.65% Impervious = 0.020 ac

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## Summary for Subcatchment EDA-1A: EDA-1A

Runoff = 1.72 cfs @ 12.09 hrs, Volume= 0.125 af, Depth> 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

	Area	(ac)	CN	Desc	cription							
	0.	111	55	Woo	/oods, Good, HSG B							
	0.	417	61	>759	% Grass co	over, Good	I, HSG B					
*	0.	016	96	Com	pacted Ea	rth, HSG B	3					
*	0.	076	·									
*	0.	010	98	Impe	ervious Sui	face, HSG	BB					
	0.	003	003 98 Roofs, HSG B									
	0.	633	66	Weig	ghted Aver	age						
	0.	620		97.9	5% Pervio	us Area						
	0.	013		2.05	% Impervi	ous Area						
					-							
	Tc	Leng	th	Slope	Velocity	Capacity	Description					
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)						
	6.0						Direct Entry, Tc Min.					

### **Summary for Subcatchment EDA-1B: EDA-1B**

Runoff = 0.10 cfs @ 12.10 hrs, Volume= 0.008 af, Depth> 1.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

	Area	(ac)	CN	Desc	cription					
	0.	0.048 61 >75% Grass cover, Good, HSG B								
	0.	048		100.0	00% Pervi	ous Area				
_	Tc (min)	Leng		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	6.0						Direct Entry, Tc Min.			

### Summary for Subcatchment EDA-2: EDA-2

Runoff = 1.00 cfs @ 12.09 hrs, Volume= 0.073 af, Depth> 2.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

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	Area	(ac)	ac) CN Description									
	0.	131	55	Woo	Voods, Good, HSG B							
	0.	188	61	>759	% Grass co	over, Good,	I, HSG B					
*	0.	004	96	Com	pacted Ea	rth, HSG B	3					
*	0.	055	96	Ston	e Dust Pat	h, HSG B						
*	0.	007	98 Impervious Surface, HSG B									
	0.	385	65	Wei	ghted Aver	age						
	0.	378		98.1	8% Pervio	us Area						
	0.	007		1.82	% Impervi	ous Area						
	Тс	Leng	•	Slope	Velocity	Capacity	Description					
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)						
	6.0						Direct Entry, Tc Min.					

## **Summary for Subcatchment EDA-3: EDA-3**

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 0.029 af, Depth> 2.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

	Area	(ac)	CN	Desc	Description							
	0.	0.039 55 Woods, Good, HSG B										
	0.071 61 >75% Grass cover, Good, HSG B											
*	0.	0.033 96 Compacted Earth, HSG B										
	0.	0.143 67 Weighted Average										
	0.	143		100.	00% Pervi	ous Area						
	Tc (min)	Leng (fe	,	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	6.0						Direct Entry, Tc Min.					

### **Summary for Reach xDP-1: Central Street**

Inflow Area = 0.681 ac, 1.91% Impervious, Inflow Depth > 2.33" for Middlesex-050yr event Inflow = 1.83 cfs @ 12.09 hrs, Volume= 0.132 af Outflow = 1.83 cfs @ 12.09 hrs, Volume= 0.132 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Summary for Reach xDP-2: Railroad Street

Inflow Area = 0.385 ac, 1.82% Impervious, Inflow Depth > 2.28" for Middlesex-050yr event Inflow = 1.00 cfs @ 12.09 hrs, Volume= 0.073 af Outflow = 1.00 cfs @ 12.09 hrs, Volume= 0.073 af, Atten= 0%, Lag= 0.0 min

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## **Summary for Reach xDP-3: Front Street**

Inflow Area = 0.143 ac, 0.00% Impervious, Inflow Depth > 2.45" for Middlesex-050yr event

Inflow = 0.41 cfs @ 12.09 hrs, Volume= 0.029 af

Outflow = 0.41 cfs (a) 12.09 hrs, Volume= 0.029 af, Atten= 0%, Lag= 0.0 min

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1A: EDA-1A Runoff Area=0.633 ac 2.05% Impervious Runoff Depth>3.20"

Tc=6.0 min CN=66 Runoff=2.36 cfs 0.169 af

Subcatchment EDA-1B: EDA-1B Runoff Area=0.048 ac 0.00% Impervious Runoff Depth>2.70"

Tc=6.0 min CN=61 Runoff=0.15 cfs 0.011 af

Subcatchment EDA-2: EDA-2 Runoff Area=0.385 ac 1.82% Impervious Runoff Depth>3.10"

Tc=6.0 min CN=65 Runoff=1.39 cfs 0.099 af

Subcatchment EDA-3: EDA-3 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth>3.30"

Tc=6.0 min CN=67 Runoff=0.55 cfs 0.039 af

Reach xDP-1: Central Street Inflow=2.51 cfs 0.180 af

Outflow=2.51 cfs 0.180 af

Reach xDP-2: Railroad Street Inflow=1.39 cfs 0.099 af

Outflow=1.39 cfs 0.099 af

Reach xDP-3: Front Street Inflow=0.55 cfs 0.039 af

Outflow=0.55 cfs 0.039 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.318 af Average Runoff Depth = 3.16" 98.35% Pervious = 1.189 ac 1.65% Impervious = 0.020 ac

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## **Summary for Subcatchment EDA-1A: EDA-1A**

Runoff = 2.36 cfs @ 12.09 hrs, Volume= 0.169 af, Depth> 3.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

_	Area	(ac)	CN	Desc	cription						
	0.	111	55	Woo	ds, Good,	HSG B					
	0.	417	61	>759	% Grass co	over, Good	, HSG B				
*	0.	016	96	96 Compacted Earth, HSG B							
*	0.	076	96	Ston	e Dust Pa	th, HSG B					
*	0.	010	98	Impe	ervious Su	face, HSG	В				
	0.	003	98	Root	s, HSG B						
	0.	633	66	Weig	ghted Aver	age					
	0.	620		97.9	5% Pervio	us Area					
	0.	013		2.05	% Impervi	ous Area					
	Тс	Leng	th	Slope	Velocity	Capacity	Description				
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0						Direct Entry, Tc Min.				

### **Summary for Subcatchment EDA-1B: EDA-1B**

Runoff = 0.15 cfs @ 12.09 hrs, Volume= 0.011 af, Depth> 2.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

Area	(ac)	CN	Desc	ription						
0.	.048	048 61 >75% Grass cover, Good, HSG B								
0.	.048		100.0	00% Pervi	ous Area					
Tc (min)	Leng		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0						Direct Entry, Tc Min.				

## Summary for Subcatchment EDA-2: EDA-2

Runoff = 1.39 cfs @ 12.09 hrs, Volume= 0.099 af, Depth> 3.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

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	Area (a	ıc)	CN	N Description									
	0.13	31	55	Woo	Voods, Good, HSG B								
	0.18	88	61	>75%	75% Grass cover, Good, HSG B								
*	0.00	04	96	Com	pacted Ea	rth, HSG B	}						
*	0.05	55	96	Ston	e Dust Pat	th, HSG B							
*	0.00	07	· ·										
	0.38	85	65	Weig	hted Aver	age							
	0.37	78		98.1	8% Pervio	us Area							
	0.00	07		1.82	% Impervi	ous Area							
	Tc L	_ength	า -	Slope	Velocity	Capacity	Description						
_	(min)	(feet	)	(ft/ft)	(ft/sec)	(cfs)							
	6.0						Direct Entry, Tc Min.						

## **Summary for Subcatchment EDA-3: EDA-3**

Runoff = 0.55 cfs @ 12.09 hrs, Volume= 0.039 af, Depth> 3.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

_	Area	(ac)	CN	Desc	Description							
	0.039 55 Woods, Good, HSG B											
	0.071 61 >75% Grass cover, Good, HSG B											
*	0.	0.033 96 Compacted Earth, HSG B										
	0.	0.143 67 Weighted Average										
	0.	143		100.	00% Pervi	ous Area						
	Tc (min)	Leng (fee	,	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	6.0	·			,	,	Direct Entry, Tc Min.					

### **Summary for Reach xDP-1: Central Street**

Inflow Area = 0.681 ac, 1.91% Impervious, Inflow Depth > 3.16" for Middlesex-100yr event 
Inflow = 2.51 cfs @ 12.09 hrs, Volume= 0.180 af 
Outflow = 2.51 cfs @ 12.09 hrs, Volume= 0.180 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Summary for Reach xDP-2: Railroad Street

Inflow Area = 0.385 ac, 1.82% Impervious, Inflow Depth > 3.10" for Middlesex-100yr event Inflow = 1.39 cfs @ 12.09 hrs, Volume= 0.099 af Outflow = 1.39 cfs @ 12.09 hrs, Volume= 0.099 af, Atten= 0%, Lag= 0.0 min

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## Summary for Reach xDP-3: Front Street

Inflow Area = 0.143 ac, 0.00% Impervious, Inflow Depth > 3.30" for Middlesex-100yr event

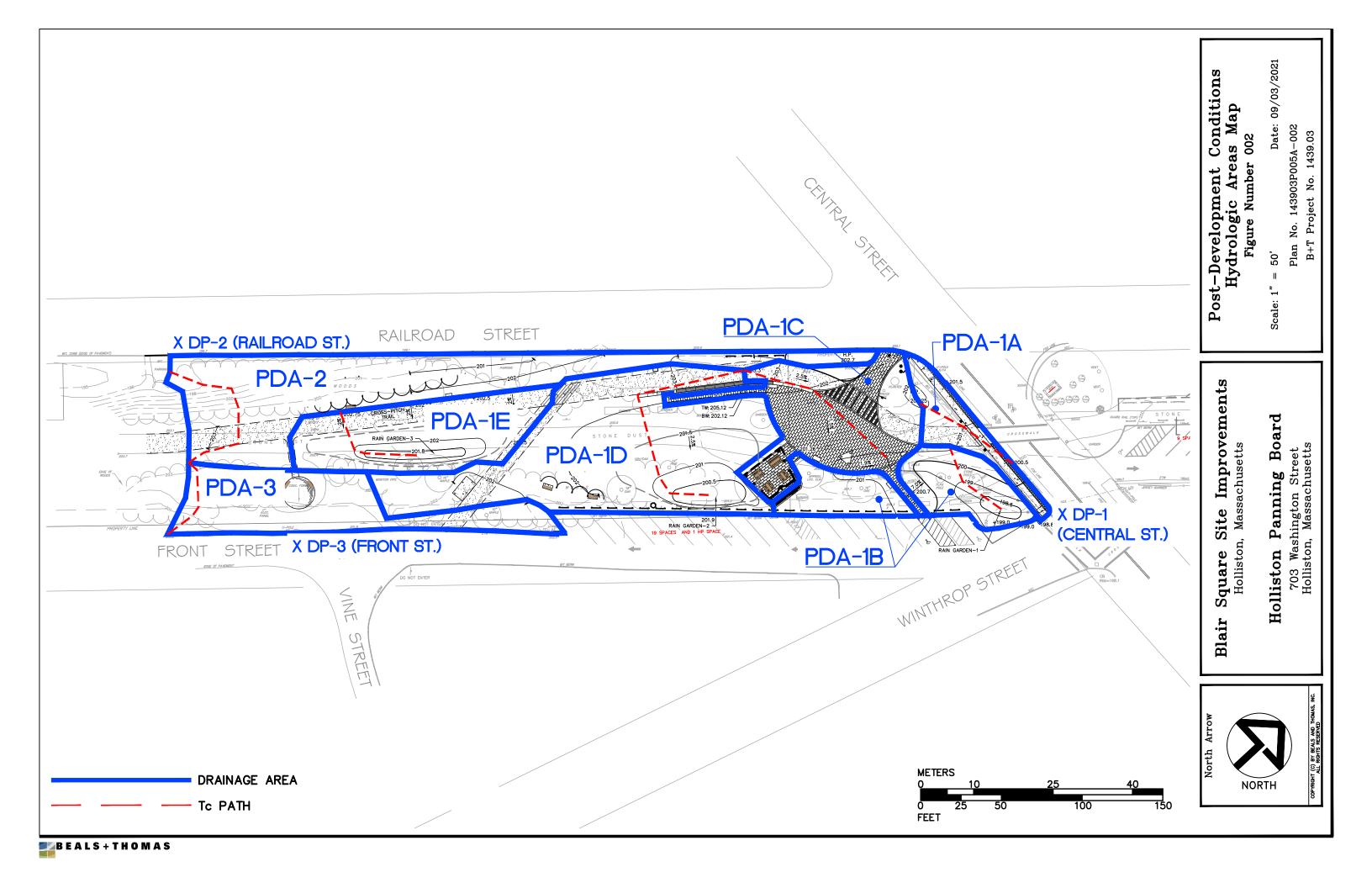
Inflow = 0.55 cfs @ 12.09 hrs, Volume= 0.039 af

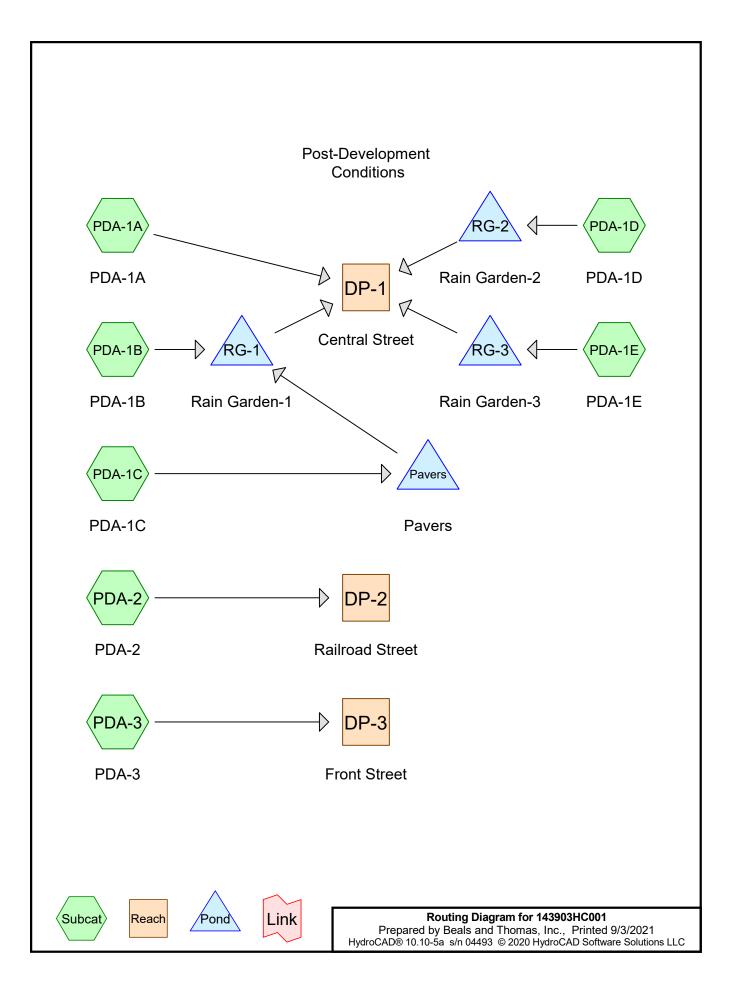
Outflow = 0.55 cfs @ 12.09 hrs, Volume= 0.039 af, Atten= 0%, Lag= 0.0 min

# Appendix B

Post-Development Conditions Hydrologic Analysis







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## **Area Listing (selected nodes)**

	Area	CN	Description
(a	cres)		(subcatchment-numbers)
(	0.736	61	>75% Grass cover, Good, HSG B (PDA-1A, PDA-1B, PDA-1C, PDA-1D, PDA-1E,
			PDA-2, PDA-3)
(	0.029	98	Impervious Surface, HSG B (PDA-1A, PDA-1D, PDA-3)
(	0.119	98	Pavers, HSG B (PDA-1C)
(	0.156	96	Stone Dust Path, HSG B (PDA-1A, PDA-1C, PDA-1D, PDA-1E, PDA-2, PDA-3)
(	0.169	55	Woods, Good, HSG B (PDA-1D, PDA-1E, PDA-2, PDA-3)
•	1.209	69	TOTAL AREA

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-1A: PDA-1A Runoff Area=0.048 ac 27.08% Impervious Runoff Depth>1.20"

Tc=6.0 min CN=78 Runoff=0.07 cfs 0.005 af

Subcatchment PDA-1B: PDA-1B Runoff Area=0.112 ac 0.00% Impervious Runoff Depth>0.40"

Tc=6.0 min CN=61 Runoff=0.03 cfs 0.004 af

Subcatchment PDA-1C: PDA-1C Runoff Area=0.164 ac 72.56% Impervious Runoff Depth>2.35"

Tc=6.0 min CN=93 Runoff=0.44 cfs 0.032 af

Subcatchment PDA-1D: PDA-1D Runoff Area=0.334 ac 2.69% Impervious Runoff Depth>0.59"

Tc=6.0 min CN=66 Runoff=0.18 cfs 0.016 af

Subcatchment PDA-1E: PDA-1E Runoff Area=0.141 ac 0.00% Impervious Runoff Depth>0.92"

Tc=6.0 min CN=73 Runoff=0.14 cfs 0.011 af

Subcatchment PDA-2: PDA-2 Runoff Area=0.267 ac 0.00% Impervious Runoff Depth>0.44"

Tc=6.0 min CN=62 Runoff=0.09 cfs 0.010 af

Subcatchment PDA-3: PDA-3 Runoff Area=0.143 ac 4.90% Impervious Runoff Depth>0.44"

Tc=6.0 min CN=62 Runoff=0.05 cfs 0.005 af

Reach DP-1: Central Street Inflow=0.07 cfs 0.005 af

Outflow=0.07 cfs 0.005 af

Reach DP-2: Railroad Street Inflow=0.09 cfs 0.010 af

Outflow=0.09 cfs 0.010 af

Reach DP-3: Front Street Inflow=0.05 cfs 0.005 af

Outflow=0.05 cfs 0.005 af

Pond Payers: Payers Peak Elev=199.49' Storage=194 cf Inflow=0.44 cfs 0.032 af

Discarded=0.12 cfs 0.027 af Primary=0.06 cfs 0.005 af Outflow=0.18 cfs 0.032 af

Pond RG-1: Rain Garden-1 Peak Elev=198.65' Storage=140 cf Inflow=0.08 cfs 0.009 af

Discarded=0.02 cfs 0.009 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.009 af

Pond RG-2: Rain Garden-2 Peak Elev=198.91' Storage=179 cf Inflow=0.18 cfs 0.016 af

Outflow=0.03 cfs 0.016 af

Pond RG-3: Rain Garden-3 Peak Elev=201.82' Storage=151 cf Inflow=0.14 cfs 0.011 af

Outflow=0.02 cfs 0.011 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.083 af Average Runoff Depth = 0.82" 87.76% Pervious = 1.061 ac 12.24% Impervious = 0.148 ac

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## Summary for Subcatchment PDA-1A: PDA-1A

Runoff = 0.07 cfs @ 12.09 hrs, Volume= 0.005 af, Depth> 1.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

_	Area	(ac)	CN	Desc	ription		
	0.	026	61	>75%	√ Grass co	over, Good	I, HSG B
*	0.	013	98	Impe	rvious Su	face, HSG	BB
*	0.	009	96	Ston	e Dust Pa	th, HSG B	
	0.	048	78	Weig	hted Aver	age	
	0.	035		72.9	2% Pervio	us Area	
	0.	013		27.0	8% Imperv	ious Area	
	_	_					
	Tc	Leng	jth	Slope	Velocity	Capacity	Description
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-1B: PDA-1B**

Runoff = 0.03 cfs @ 12.13 hrs, Volume= 0.004 af, Depth> 0.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

	Area	(ac)	CN	Desc	cription		
	0.	112	61	>75%	% Grass co	over, Good	, HSG B
	0.	112		100.	00% Pervi	ous Area	
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	6.0						Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-1C: PDA-1C**

Runoff = 0.44 cfs @ 12.09 hrs, Volume= 0.032 af, Depth> 2.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

	Area (ac)	CN	Description
	0.021	61	>75% Grass cover, Good, HSG B
*	0.024	96	Stone Dust Path, HSG B
*	0.119	98	Pavers, HSG B
	0.164	93	Weighted Average
	0.045		27.44% Pervious Area
	0.119		72.56% Impervious Area

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	Length	•	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0			Direct Entry, Tc Min.		

### **Summary for Subcatchment PDA-1D: PDA-1D**

Runoff = 0.18 cfs @ 12.11 hrs, Volume= 0.016 af, Depth> 0.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

	Area	(ac)	CN	Desc	cription			
	0.	046	55	Woo	ds, Good,	HSG B		
	0.	233	61	>759	% Grass co	over, Good	, HSG B	
*	0.	046	96	Ston	e Dust Pat	th, HSG B		
*	0.	009	98	Impe	Impervious Surface, HSG B			
-	0.	334	66	Weig	ghted Aver	age		
	0.	325		97.3	1% Pervio	us Area		
	0.	009		2.69	% Impervi	ous Area		
	Тс	Leng	jth	Slope	Velocity	Capacity	Description	
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)		
	6.0	•		•			Direct Entry, Tc Min.	

## **Summary for Subcatchment PDA-1E: PDA-1E**

Runoff = 0.14 cfs @ 12.10 hrs, Volume= 0.011 af, Depth> 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

	Area	(ac)	CN	Desc	cription				
	0.	003	55	Woo	Woods, Good, HSG B				
	0.	880	61	>759	% Grass co	over, Good	, HSG B		
*	0.	050	96	Ston	e Dust Pa	th, HSG B			
	0.	0.141 73 Weighted Average							
	0.141 100.00% Pervious Area			00% Pervi	ous Area				
	Tc	Leng	•	Slope	Velocity	Capacity	Description		
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)			
	6.0						Direct Entry, Tc Min.		

## **Summary for Subcatchment PDA-2: PDA-2**

Runoff = 0.09 cfs @ 12.12 hrs, Volume= 0.010 af, Depth> 0.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

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	Area	(ac)	CN	Desc	cription			
	0.	081	55	Woo	Woods, Good, HSG B			
	0.	164	61	>759	% Grass co	over, Good	d, HSG B	
*	0.	022	96	Ston	e Dust Pat	h, HSG B		
	0.	0.267 62 Weighted Average						
	0.	267		100.	00% Pervi	ous Area		
	Тс	Leng	ıth	Slope	Velocity	Capacity	Description	
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	•	
_	6.0			_			Direct Entry To Min	

6.0

Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-3: PDA-3**

Runoff = 0.05 cfs @ 12.12 hrs, Volume= 0.005 af, Depth> 0.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-002yr Rainfall=3.10"

	Area	(ac)	CN	Desc	cription		
	0.	039	55	Woo	ds, Good,	HSG B	
	0.	092	61	>759	% Grass co	over, Good	I, HSG B
*	0.	005	96	Ston	e Dust Pat	th, HSG B	
*	0.	007	98	Impe	ervious Sui	face, HSG	G B
0.143 62 Weighted Average							
	0.	136		95.1	0% Pervio	us Area	
	0.	007		4.90	% Impervi	ous Area	
	Тс	Leng	th	Slope	Velocity	Capacity	Description
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

### **Summary for Reach DP-1: Central Street**

Inflow Area = 0.799 ac, 17.65% Impervious, Inflow Depth > 0.07" for Middlesex-002yr event

Inflow = 0.07 cfs @ 12.09 hrs, Volume= 0.005 af

Outflow = 0.07 cfs @ 12.09 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Summary for Reach DP-2: Railroad Street

Inflow Area = 0.267 ac, 0.00% Impervious, Inflow Depth > 0.44" for Middlesex-002yr event

Inflow = 0.09 cfs @ 12.12 hrs, Volume= 0.010 af

Outflow = 0.09 cfs @ 12.12 hrs, Volume= 0.010 af, Atten= 0%, Lag= 0.0 min

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## **Summary for Reach DP-3: Front Street**

Inflow Area = 0.143 ac, 4.90% Impervious, Inflow Depth > 0.44" for Middlesex-002yr event

Inflow = 0.05 cfs @ 12.12 hrs, Volume= 0.005 af

Outflow = 0.05 cfs @ 12.12 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

### **Summary for Pond Pavers: Pavers**

Inflow Area = 0.164 ac, 72.56% Impervious, Inflow Depth > 2.35" for Middlesex-002yr event 
0.44 cfs @ 12.09 hrs, Volume= 0.032 af

Outflow = 0.18 cfs @ 12.29 hrs, Volume= 0.032 af, Atten= 58%, Lag= 12.5 min

Discarded = 0.12 cfs @ 11.86 hrs, Volume= 0.027 af Primary = 0.06 cfs @ 12.29 hrs, Volume= 0.005 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 199.49' @ 12.29 hrs Surf.Area= 5,190 sf Storage= 194 cf

Plug-Flow detention time= 6.2 min calculated for 0.032 af (100% of inflow)

Center-of-Mass det. time= 6.0 min ( 799.2 - 793.1 )

Volume	Invert Av	ail.Storage	Storage Descrip	tion	
#1	199.37'	1,448 cf	Custom Stage I	Data (Prismatic)	Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
199.37 199.38 200.30	5,190 5,190 5,190	30.0	0 16 1,432	0 16 1,448	

Device	Routing	Invert	Outlet Devices
#1	Discarded	199.37'	1.020 in/hr Exfiltration over Surface area
#2	Primary	199.30'	<b>4.0" Round Culvert</b> L= 17.0' Ke= 1.000
	-		Inlet / Outlet Invert= 199.30' / 198.90' S= 0.0235 '/' Cc= 0.900
			n= 0.020 Corrugated PE, corrugated interior, Flow Area= 0.09 sf

**Discarded OutFlow** Max=0.12 cfs @ 11.86 hrs HW=199.38' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.06 cfs @ 12.29 hrs HW=199.49' (Free Discharge) 2=Culvert (Inlet Controls 0.06 cfs @ 1.13 fps)

### **Summary for Pond RG-1: Rain Garden-1**

Inflow Area =	0.276 ac, 43.12% Impervious, Inflow De	epth > 0.37" for Middlesex-002yr event
Inflow =	0.08 cfs @ 12.26 hrs, Volume=	0.009 af
Outflow =	0.02 cfs @ 12.91 hrs, Volume=	0.009 af, Atten= 75%, Lag= 39.1 min
Discarded =	0.02 cfs @ 12.91 hrs, Volume=	0.009 af
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

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Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 198.65' @ 12.91 hrs Surf.Area= 372 sf Storage= 140 cf

Plug-Flow detention time= 76.9 min calculated for 0.009 af (100% of inflow)

Center-of-Mass det. time= 76.7 min ( 915.1 - 838.4 )

Volume	Inve	rt Ava	il.Storage	Storage Description						
#1	195.7	4'	339 c	f Custom Stage	Custom Stage Data (Prismatic) Listed below (Recalc)					
Elevation	on S	Surf.Area	Voids	Inc.Store	Cum.Store					
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)					
195.7	74	202	0.0	0	0					
195.7	75	202	30.0	1	1					
196.7	<b>7</b> 5	202	30.0	61	61					
196.7	76	202	10.0	0	61					
198.5	50	202	10.0	35	97					
198.5	51	202	100.0	2	99					
199.0	00	780	100.0	241	339					
Device	Routing	In	vert Ou	utlet Devices						
#1	Discarded	d 195	5.74' <b>2.</b> 4	410 in/hr Exfiltrati	on over Surface	area				
#2	Primary	198	3.80' <b>3.</b> 0	0' long x 4.0' brea	dth Broad-Crest	ed Rectangular Weir				
	-		He	ead (feet) 0.20 0.4	40 0.60 0.80 1.0	00 1.20 1.40 1.60 1.80 2.00				
				2.50 3.00 3.50 4.00 4.50 5.00 5.50						
			Co	oef. (English) 2.38	2.54 2.69 2.68	2.67 2.67 2.65 2.66 2.66				
	7 3.32									

**Discarded OutFlow** Max=0.02 cfs @ 12.91 hrs HW=198.65' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=195.74' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## **Summary for Pond RG-2: Rain Garden-2**

Inflow Area = 0.334 ac, 2.69% Impervious, Inflow Depth > 0.59" for Middlesex-002yr event 
Inflow = 0.18 cfs @ 12.11 hrs, Volume= 0.016 af 
Outflow = 0.03 cfs @ 11.96 hrs, Volume= 0.016 af, Atten= 83%, Lag= 0.0 min 
Discarded = 0.03 cfs @ 11.96 hrs, Volume= 0.016 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 198.91' @ 12.97 hrs Surf.Area= 561 sf Storage= 179 cf

Plug-Flow detention time= 45.5 min calculated for 0.016 af (100% of inflow) Center-of-Mass det. time= 45.3 min (938.4 - 893.1)

Volume	Invert	Avail.Storage	Storage Description
#1	197.74'	1,891 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
	<u> </u>			
197.74	561	0.0	0	0
197.75	561	30.0	2	2
198.75	561	30.0	168	170
198.76	561	10.0	1	171
200.50	561	10.0	98	268
200.51	561	100.0	6	274
201.00	1,248	100.0	443	717
201.50	3,450	100.0	1,175	1,891

Device Routing Invert Outlet Devices

#1 Discarded

197.74' 2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.03 cfs @ 11.96 hrs HW=197.75' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

### **Summary for Pond RG-3: Rain Garden-3**

0.141 ac, 0.00% Impervious, Inflow Depth > 0.92" for Middlesex-002yr event Inflow Area =

Inflow 0.14 cfs @ 12.10 hrs, Volume= 0.011 af

0.02 cfs @ 12.95 hrs, Volume= Outflow 0.011 af, Atten= 87%, Lag= 51.4 min =

Discarded = 0.02 cfs @ 12.95 hrs, Volume= 0.011 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 201.82' @ 12.95 hrs Surf.Area= 341 sf Storage= 151 cf

Plug-Flow detention time= 77.2 min calculated for 0.011 af (100% of inflow)

Center-of-Mass det. time= 77.0 min ( 943.6 - 866.6 )

Volume	Invert Ava	il.Storage	Storage Description							
#1	199.04'	944 cf	Custom Stage I	<b>Data (Prismatic)</b> Lis	sted below (Recalc)					
Elevation	Surf.Area	Voids	Inc.Store	Cum.Store						
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)						
199.04	301	0.0	0	0						
199.05	301	30.0	1	1						
200.05	301	30.0	90	91						
200.06	301	10.0	0	92						
201.80	301	10.0	52	144						
201.81	301	100.0	3	147						
202.00	945	100.0	118	265						
202.40	2,450	100.0	679	944						
Device Ro	outing In	vert Outl	et Devices							

199.04' 2.410 in/hr Exfiltration over Surface area #1 Discarded

Discarded OutFlow Max=0.02 cfs @ 12.95 hrs HW=201.82' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-1A: PDA-1A Runoff Area=0.048 ac 27.08% Impervious Runoff Depth>2.29"

Tc=6.0 min CN=78 Runoff=0.13 cfs 0.009 af

Subcatchment PDA-1B: PDA-1B Runoff Area=0.112 ac 0.00% Impervious Runoff Depth>1.08"

Tc=6.0 min CN=61 Runoff=0.12 cfs 0.010 af

Subcatchment PDA-1C: PDA-1C Runoff Area=0.164 ac 72.56% Impervious Runoff Depth>3.70"

Tc=6.0 min CN=93 Runoff=0.67 cfs 0.051 af

Subcatchment PDA-1D: PDA-1D Runoff Area=0.334 ac 2.69% Impervious Runoff Depth>1.39"

Tc=6.0 min CN=66 Runoff=0.51 cfs 0.039 af

Subcatchment PDA-1E: PDA-1E Runoff Area=0.141 ac 0.00% Impervious Runoff Depth>1.89"

Tc=6.0 min CN=73 Runoff=0.31 cfs 0.022 af

Subcatchment PDA-2: PDA-2 Runoff Area=0.267 ac 0.00% Impervious Runoff Depth>1.14"

Tc=6.0 min CN=62 Runoff=0.32 cfs 0.025 af

Subcatchment PDA-3: PDA-3 Runoff Area=0.143 ac 4.90% Impervious Runoff Depth>1.14"

Tc=6.0 min CN=62 Runoff=0.17 cfs 0.014 af

Reach DP-1: Central Street Inflow=0.16 cfs 0.015 af

Outflow=0.16 cfs 0.015 af

Reach DP-2: Railroad Street Inflow=0.32 cfs 0.025 af

Outflow=0.32 cfs 0.025 af

Reach DP-3: Front Street Inflow=0.17 cfs 0.014 af

Outflow=0.17 cfs 0.014 af

Pond Pavers: Pavers Peak Elev=199.62' Storage=395 cf Inflow=0.67 cfs 0.051 af

Discarded=0.12 cfs 0.040 af Primary=0.13 cfs 0.011 af Outflow=0.25 cfs 0.051 af

Pond RG-1: Rain Garden-1 Peak Elev=198.87' Storage=247 cf Inflow=0.22 cfs 0.021 af

Discarded=0.03 cfs 0.015 af Primary=0.13 cfs 0.006 af Outflow=0.16 cfs 0.021 af

Pond RG-2: Rain Garden-2 Peak Elev=200.91' Storage=611 cf Inflow=0.51 cfs 0.039 af

Outflow=0.06 cfs 0.039 af

Pond RG-3: Rain Garden-3 Peak Elev=202.05' Storage=319 cf Inflow=0.31 cfs 0.022 af

Outflow=0.06 cfs 0.022 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.170 af Average Runoff Depth = 1.69" 87.76% Pervious = 1.061 ac 12.24% Impervious = 0.148 ac Prepared by Beals and Thomas, Inc.

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## Summary for Subcatchment PDA-1A: PDA-1A

Runoff = 0.13 cfs @ 12.09 hrs, Volume= 0.009 af, Depth> 2.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

	Area	(ac)	CN	Desc	cription		
	0.	026	61	>75%	√ Grass co	over, Good	, HSG B
*	0.	013	98	Impe	rvious Su	rface, HSG	i B
*	0.	009	96	Ston	e Dust Pa	th, HSG B	
	0.048 78 Weighted Average						
	0.035 72.92% Pervious Area					us Area	
	0.	013		27.0	8% Imperv	ious Area	
	Тс	Leng	jth	Slope	Velocity	Capacity	Description
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-1B: PDA-1B**

Runoff = 0.12 cfs @ 12.10 hrs, Volume= 0.010 af, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

	Area	(ac)	CN	Desc	cription				
	0.	0.112 61 >75% Grass cover, Good, HSG B							
	0.112 100.00% Pervious Area								
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
_	6.0						Direct Entry, Tc Min.		

### **Summary for Subcatchment PDA-1C: PDA-1C**

Runoff = 0.67 cfs @ 12.08 hrs, Volume= 0.051 af, Depth> 3.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

	Area (ac)	CN	Description
	0.021	61	>75% Grass cover, Good, HSG B
*	0.024	96	Stone Dust Path, HSG B
*	0.119	98	Pavers, HSG B
	0.164	93	Weighted Average
	0.045		27.44% Pervious Area
	0.119		72.56% Impervious Area

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-1D: PDA-1D**

Runoff = 0.51 cfs @ 12.10 hrs, Volume= 0.039 af, Depth> 1.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

	Area	(ac)	CN	Desc	Description						
	0.	046	6 55 Woods, Good, HSG B								
	0.	233	61	>759	>75% Grass cover, Good, HSG B						
*	0.	046	96	Ston	itone Dust Path, HSG B						
*	0.	009	, ,								
-	0.	0.334 66 Weighted Average									
	0.325 97.31% Pervious Area					us Area					
	0.009			2.69	% Impervi	ous Area					
	Тс	Leng	jth	Slope	Velocity	Capacity	Description				
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0	•		•			Direct Entry, Tc Min.				

## **Summary for Subcatchment PDA-1E: PDA-1E**

Runoff = 0.31 cfs @ 12.09 hrs, Volume= 0.022 af, Depth> 1.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

	Area	(ac)	CN	Desc	cription		
0.003 55 Woods, Good, HSG B							
	0.088 61 >75% Grass cover, Good, HSG B				, HSG B		
*	0.	0.050 96 Stone Dust Path, HSG B					
	0.	0.141 73 Weighted Average					
	0.	0.141 100.00% Pe			00% Pervi	ous Area	
	Тс	Leng	•	Slope	Velocity	Capacity	Description
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-2: PDA-2**

Runoff = 0.32 cfs @ 12.10 hrs, Volume= 0.025 af, Depth> 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

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	Area	(ac)	CN	Desc	Description					
	0.	0.081 55 Woods, Good, HSG B								
	0.164 61 >75% Grass cover, Good, HSG B					, HSG B				
*	0.	.022 96 Stone Dust Path, HSG B								
	0.	0.267 62 Weighted Average								
	0.	0.267 100.00% Pervious Area								
	Тс	Tc Length		Slope	Velocity	Capacity	Description			
	(min)	(fee	,	(ft/ft)	(ft/sec)	(cfs)				
	6.0						Direct Entry, Tc Min.			

### **Summary for Subcatchment PDA-3: PDA-3**

Runoff = 0.17 cfs @ 12.10 hrs, Volume= 0.014 af, Depth> 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-010yr Rainfall=4.50"

_	Area (	(ac)	CN	Desc	cription						
	0.0	039	55	Woo	Woods, Good, HSG B						
	0.0	092	61	>75%	>75% Grass cover, Good, HSG B						
*	0.0	005	96	Ston	Stone Dust Path, HSG B						
*	0.0	007	98	Impe	ervious Sui	face, HSG	6 B				
	0.	143	62	Weig	ghted Aver	age					
	0.136 95.10% Pervious Area										
	0.0	0.007 4.90% Impervious Area									
	Tc	Leng	th	Slope	Velocity	Capacity	Description				
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0						Direct Entry, Tc Min.				

### **Summary for Reach DP-1: Central Street**

Inflow Area = 0.799 ac, 17.65% Impervious, Inflow Depth > 0.22" for Middlesex-010yr event

Inflow = 0.16 cfs @ 12.45 hrs, Volume= 0.015 af

Outflow = 0.16 cfs @ 12.45 hrs, Volume= 0.015 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Summary for Reach DP-2: Railroad Street

Inflow Area = 0.267 ac, 0.00% Impervious, Inflow Depth > 1.14" for Middlesex-010yr event

Inflow = 0.32 cfs @ 12.10 hrs, Volume= 0.025 af

Outflow = 0.32 cfs @ 12.10 hrs, Volume= 0.025 af, Atten= 0%, Lag= 0.0 min

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## **Summary for Reach DP-3: Front Street**

Inflow Area = 0.143 ac, 4.90% Impervious, Inflow Depth > 1.14" for Middlesex-010yr event

Inflow = 0.17 cfs @ 12.10 hrs, Volume= 0.014 af

Outflow = 0.17 cfs @ 12.10 hrs, Volume= 0.014 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

### **Summary for Pond Pavers: Pavers**

Inflow Area = 0.164 ac, 72.56% Impervious, Inflow Depth > 3.70" for Middlesex-010yr event

Inflow = 0.67 cfs @ 12.08 hrs, Volume= 0.051 af

Outflow = 0.25 cfs @ 12.33 hrs, Volume= 0.051 af, Atten= 63%, Lag= 14.9 min

Discarded = 0.12 cfs @ 11.73 hrs, Volume= 0.040 af Primary = 0.13 cfs @ 12.33 hrs, Volume= 0.011 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 199.62' @ 12.33 hrs Surf.Area= 5,190 sf Storage= 395 cf

Plug-Flow detention time= 10.0 min calculated for 0.051 af (100% of inflow)

Center-of-Mass det. time= 9.8 min ( 790.7 - 780.9 )

Volume	Invert Av	ail.Storage	Storage Descrip	tion	
#1	199.37'	1,448 cf	Custom Stage I	Data (Prismatic)	Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
199.37 199.38 200.30	5,190 5,190 5,190	30.0	0 16 1,432	0 16 1,448	

Device	Routing	Invert	Outlet Devices				
#1	Discarded	199.37'	1.020 in/hr Exfiltration over Surface area				
#2	Primary	199.30'	<b>4.0" Round Culvert</b> L= 17.0' Ke= 1.000				
	-		Inlet / Outlet Invert= 199.30' / 198.90' S= 0.0235 '/' Cc= 0.900				

n= 0.020 Corrugated PE, corrugated interior, Flow Area= 0.09 sf

**Discarded OutFlow** Max=0.12 cfs @ 11.73 hrs HW=199.38' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.13 cfs @ 12.33 hrs HW=199.62' (Free Discharge) 2=Culvert (Inlet Controls 0.13 cfs @ 1.45 fps)

### **Summary for Pond RG-1: Rain Garden-1**

Inflow Area =	0.276 ac, 43.12% Impervious, Inflow D	epth > 0.92" for Middlesex-010yr event
Inflow =	0.22 cfs @ 12.13 hrs, Volume=	0.021 af
Outflow =	0.16 cfs @ 12.48 hrs, Volume=	0.021 af, Atten= 25%, Lag= 20.8 min
Discarded =	0.03 cfs @ 12.48 hrs, Volume=	0.015 af
Primary =	0.13 cfs @ 12.48 hrs, Volume=	0.006 af

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Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 198.87' @ 12.48 hrs Surf.Area= 625 sf Storage= 247 cf

Plug-Flow detention time= 66.7 min calculated for 0.021 af (100% of inflow)

Center-of-Mass det. time= 66.6 min ( 886.2 - 819.7 )

Volume	Inv	ert Ava	il.Storage	age Storage Description					
#1	195.7	74'	339 с	f Custom Stage	Data (Prismatic)	Listed below (Recalc)			
			Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)				
195.7	74	202	0.0	0	0				
195.7	75	202	30.0	1	1				
196.7	75	202	30.0	61	61				
196.7	76	202	10.0	0	61				
198.5	50	202	10.0	35	97				
198.5	51	202	100.0	2	99				
199.0	00	780	100.0	241	339				
Device	Routing	In	vert Oı	utlet Devices					
#1	Discarde	ed 195	5.74' <b>2.</b> 4	410 in/hr Exfiltrati	ion over Surface	area			
#2 Primary 198.80' <b>3.</b>			•		ed Rectangular Weir				
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60									
2.50 3.00 3.50 4.00 4.50 5.00 5.50									
			Co	pef. (English) 2.38	3 2.54 2.69 2.68	2.67 2.67 2.65 2.66 2.66			
2.68 2.72 2.73 2.76 2.79 2.88 3.07					7 3.32				

**Discarded OutFlow** Max=0.03 cfs @ 12.48 hrs HW=198.87' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.13 cfs @ 12.48 hrs HW=198.87' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.13 cfs @ 0.62 fps)

### **Summary for Pond RG-2: Rain Garden-2**

Inflow Area = 0.334 ac, 2.69% Impervious, Inflow Depth > 1.39" for Middlesex-010yr event 
Inflow = 0.51 cfs @ 12.10 hrs, Volume= 0.039 af 
Outflow = 0.06 cfs @ 13.03 hrs, Volume= 0.039 af, Atten= 88%, Lag= 56.1 min 
Discarded = 0.06 cfs @ 13.03 hrs, Volume= 0.039 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 200.91' @ 13.03 hrs Surf.Area= 1,122 sf Storage= 611 cf

Plug-Flow detention time= 120.9 min calculated for 0.039 af (100% of inflow)

Center-of-Mass det. time= 120.8 min ( 984.6 - 863.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	197.74'	1,891 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

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Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
197.74	561	0.0	0	0
197.75	561	30.0	2	2
198.75	561	30.0	168	170
198.76	561	10.0	1	171
200.50	561	10.0	98	268
200.51	561	100.0	6	274
201.00	1,248	100.0	443	717
201.50	3,450	100.0	1,175	1,891

Device Routing Invert Outlet Devices

#1 Discarded 197.74' **2.410 in/hr Exfiltration over Surface area** 

**Discarded OutFlow** Max=0.06 cfs @ 13.03 hrs HW=200.91' (Free Discharge)

**1=Exfiltration** (Exfiltration Controls 0.06 cfs)

### **Summary for Pond RG-3: Rain Garden-3**

Inflow Area = 0.141 ac, 0.00% Impervious, Inflow Depth > 1.89" for Middlesex-010yr event

Inflow = 0.31 cfs @ 12.09 hrs, Volume= 0.022 af

Outflow = 0.06 cfs @ 12.55 hrs, Volume= 0.022 af, Atten= 79%, Lag= 27.5 min

Discarded = 0.06 cfs @ 12.55 hrs, Volume= 0.022 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 202.05' @ 12.55 hrs Surf.Area= 1,139 sf Storage= 319 cf

Plug-Flow detention time= 77.4 min calculated for 0.022 af (100% of inflow)

Center-of-Mass det. time= 77.3 min ( 921.9 - 844.6 )

Volume	Invert Ava	il.Storage	Storage Description						
#1	199.04'	944 cf	Custom Stage Data (Prismatic) Listed below (Recalc)						
Elevation	Surf.Area	Voids	Inc.Store	Cum.Store					
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)					
199.04	301	0.0	0	0					
199.05	301	30.0	1	1					
200.05	301	30.0	90	91					
200.06	301	10.0	0	92					
201.80	301	10.0	52	144					
201.81	301	100.0	3	147					
202.00	945	100.0	118	265					
202.40	2,450	100.0	679	944					
Dovice Pr	outing In	wort Out	lot Davisos						

Device Routing Invert Outlet Devices

#1 Discarded 199.04' 2.410 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.06 cfs @ 12.55 hrs HW=202.05' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-1A: PDA-1A Runoff Area=0.048 ac 27.08% Impervious Runoff Depth>2.96"

Tc=6.0 min CN=78 Runoff=0.17 cfs 0.012 af

Subcatchment PDA-1B: PDA-1B Runoff Area=0.112 ac 0.00% Impervious Runoff Depth>1.55"

Tc=6.0 min CN=61 Runoff=0.19 cfs 0.014 af

**Subcatchment PDA-1C: PDA-1C**Runoff Area=0.164 ac 72.56% Impervious Runoff Depth>4.49"

Tc=6.0 min CN=93 Runoff=0.81 cfs 0.061 af

Subcatchment PDA-1D: PDA-1D Runoff Area=0.334 ac 2.69% Impervious Runoff Depth>1.93"

Tc=6.0 min CN=66 Runoff=0.73 cfs 0.054 af

Subcatchment PDA-1E: PDA-1E Runoff Area=0.141 ac 0.00% Impervious Runoff Depth>2.51"

Tc=6.0 min CN=73 Runoff=0.41 cfs 0.030 af

Subcatchment PDA-2: PDA-2 Runoff Area=0.267 ac 0.00% Impervious Runoff Depth>1.62"

Tc=6.0 min CN=62 Runoff=0.48 cfs 0.036 af

Subcatchment PDA-3: PDA-3 Runoff Area=0.143 ac 4.90% Impervious Runoff Depth>1.62"

Tc=6.0 min CN=62 Runoff=0.26 cfs 0.019 af

Reach DP-1: Central Street Inflow=0.27 cfs 0.023 af

Outflow=0.27 cfs 0.023 af

Reach DP-2: Railroad Street Inflow=0.48 cfs 0.036 af

Outflow=0.48 cfs 0.036 af

Reach DP-3: Front Street Inflow=0.26 cfs 0.019 af

Outflow=0.26 cfs 0.019 af

Pond Pavers: Pavers Peak Elev=199.71' Storage=522 cf Inflow=0.81 cfs 0.061 af

Discarded=0.12 cfs 0.046 af Primary=0.15 cfs 0.015 af Outflow=0.28 cfs 0.061 af

Pond RG-1: Rain Garden-1 Peak Elev=198.89' Storage=262 cf Inflow=0.31 cfs 0.030 af

Discarded=0.04 cfs 0.018 af Primary=0.20 cfs 0.012 af Outflow=0.24 cfs 0.030 af

Pond RG-2: Rain Garden-2 Peak Elev=201.11' Storage=880 cf Inflow=0.73 cfs 0.054 af

Outflow=0.10 cfs 0.054 af

Pond RG-3: Rain Garden-3 Peak Elev=202.15' Storage=442 cf Inflow=0.41 cfs 0.030 af

Outflow=0.08 cfs 0.030 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.227 af Average Runoff Depth = 2.25" 87.76% Pervious = 1.061 ac 12.24% Impervious = 0.148 ac Prepared by Beals and Thomas, Inc.

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## Summary for Subcatchment PDA-1A: PDA-1A

Runoff = 0.17 cfs @ 12.09 hrs, Volume= 0.012 af, Depth> 2.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

_	Area	(ac)	CN	Desc	ription		
	0.	026	61	>75%	√ Grass co	over, Good	I, HSG B
*	0.	013	98	Impe	rvious Su	face, HSG	BB
*	0.	009	96	Ston	e Dust Pa	th, HSG B	
	0.048 78 Weighted Average						
	0.035 72.92% Pervious Area						
	0.	013		27.0	8% Imperv	ious Area	
	_	_					
	Tc	Leng	jth	Slope	Velocity	Capacity	Description
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-1B: PDA-1B**

Runoff = 0.19 cfs @ 12.10 hrs, Volume= 0.014 af, Depth> 1.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

	Area	(ac)	CN	Desc	cription			
	0.	0.112 61 >75% Grass cover, Good, HSG B						
	0.112 100.00% Pervious Area							
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
_	6.0						Direct Entry, Tc Min.	

### **Summary for Subcatchment PDA-1C: PDA-1C**

Runoff = 0.81 cfs @ 12.08 hrs, Volume= 0.061 af, Depth> 4.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

	Area (ac)	CN	Description
	0.021	61	>75% Grass cover, Good, HSG B
*	0.024	96	Stone Dust Path, HSG B
*	0.119	98	Pavers, HSG B
	0.164	93	Weighted Average
	0.045		27.44% Pervious Area
	0.119		72.56% Impervious Area

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Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
 6.0		Direct Entry, Tc Min.				

### **Summary for Subcatchment PDA-1D: PDA-1D**

Runoff = 0.73 cfs @ 12.09 hrs, Volume= 0.054 af, Depth> 1.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

	Area	(ac)	CN	Desc	cription		
	0.	046	55	Woo	ds, Good,	HSG B	
	0.	233	61	>759	% Grass co	over, Good	, HSG B
*	0.	046	96	Ston	e Dust Pat	h, HSG B	
*	0.	009	98	Impe	ervious Sui	face, HSG	БВ
	0.	334 66 Weighted Average					
	0.	325		97.3	1% Pervio	us Area	
	0.	009		2.69	% Impervi	ous Area	
	Tc	Leng	th	Slope	Velocity	Capacity	Description
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

## **Summary for Subcatchment PDA-1E: PDA-1E**

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 0.030 af, Depth> 2.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

	Area	(ac)	CN	Desc	cription		
	0.	003	55	Woo	ds, Good,	HSG B	
	0.	880	61	>759	% Grass co	over, Good	, HSG B
*	0.	050	96	Ston	e Dust Pa	th, HSG B	
	0.141 73 Weighted Average				ghted Aver	age	
	0.141 100.00% Pervious Area				00% Pervi	ous Area	
	Tc (min)	Leng	•	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_		(166	<del>-</del> ()	(11/11)	(II/SEC)	(615)	
	6.0						Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-2: PDA-2**

Runoff = 0.48 cfs @ 12.10 hrs, Volume= 0.036 af, Depth> 1.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

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	Area	(ac)	CN	Desc	cription		
	0.	081	55	Woo	ds, Good,	HSG B	
	0.	164	61	>759	% Grass co	over, Good	, HSG B
*	0.	022	96	Ston	e Dust Pat	th, HSG B	
	0.267 62 Weighted Average					age	
	0.267 100.00% Pervious Area				00% Pervi	ous Area	
	_						
	Tc	Leng	th	Slope	Velocity	Capacity	Description
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

## **Summary for Subcatchment PDA-3: PDA-3**

Runoff = 0.26 cfs @ 12.10 hrs, Volume= 0.019 af, Depth> 1.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-025yr Rainfall=5.30"

_	Area (	(ac)	CN	Desc	cription				
	0.0	039	55	Woo	Woods, Good, HSG B				
	0.0	092	61	>75%	>75% Grass cover, Good, HSG B				
*	0.0	005	96	Ston	Stone Dust Path, HSG B				
*	0.0	007	98	Impe	rvious Su	face, HSG	В		
	0.	143	62	Weig	ghted Aver	age			
0.136 95.10% Pervious Area					0% Pervio	us Area			
	0.0	007		4.90	% Impervi	ous Area			
	Тс	Leng	th	Slope	Velocity	Capacity	Description		
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)			
	6.0						Direct Entry, Tc Min.		

### **Summary for Reach DP-1: Central Street**

Inflow Area = 0.799 ac, 17.65% Impervious, Inflow Depth > 0.35" for Middlesex-025yr event

Inflow = 0.27 cfs @ 12.33 hrs, Volume= 0.023 af

Outflow = 0.27 cfs @ 12.33 hrs, Volume= 0.023 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Summary for Reach DP-2: Railroad Street

Inflow Area = 0.267 ac, 0.00% Impervious, Inflow Depth > 1.62" for Middlesex-025yr event

Inflow = 0.48 cfs @ 12.10 hrs, Volume= 0.036 af

Outflow = 0.48 cfs @ 12.10 hrs, Volume= 0.036 af, Atten= 0%, Lag= 0.0 min

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## **Summary for Reach DP-3: Front Street**

Inflow Area = 0.143 ac, 4.90% Impervious, Inflow Depth > 1.62" for Middlesex-025yr event

Inflow = 0.26 cfs @ 12.10 hrs, Volume= 0.019 af

Outflow = 0.26 cfs @ 12.10 hrs, Volume= 0.019 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

### **Summary for Pond Pavers: Pavers**

Inflow Area = 0.164 ac, 72.56% Impervious, Inflow Depth > 4.49" for Middlesex-025yr event

Inflow = 0.81 cfs @ 12.08 hrs, Volume= 0.061 af

Outflow = 0.28 cfs @ 12.36 hrs, Volume= 0.061 af, Atten= 66%, Lag= 16.4 min

Discarded = 0.12 cfs @ 11.68 hrs, Volume= 0.046 af Primary = 0.15 cfs @ 12.36 hrs, Volume= 0.015 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 199.71' @ 12.36 hrs Surf.Area= 5,190 sf Storage= 522 cf

Plug-Flow detention time= 12.1 min calculated for 0.061 af (100% of inflow)

Center-of-Mass det. time= 11.9 min (787.8 - 776.0)

Volume	Invert Av	ail.Storage	Storage Descrip	tion		_
#1	199.37'	1,448 cf	Custom Stage I	Data (Prismatic) Lis	sted below (Recalc)	_
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
199.37 199.38 200.30	5,190 5,190 5,190	30.0	0 16 1,432	0 16 1,448		

Device	Routing	Invert	Outlet Devices
#1	Discarded	199.37'	1.020 in/hr Exfiltration over Surface area
#2	Primary	199.30'	<b>4.0" Round Culvert</b> L= 17.0' Ke= 1.000
	-		Inlet / Outlet Invert= 199.30' / 198.90' S= 0.0235 '/' Cc= 0.900
			n= 0.020 Corrugated PE, corrugated interior, Flow Area= 0.09 sf

**Discarded OutFlow** Max=0.12 cfs @ 11.68 hrs HW=199.38' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.15 cfs @ 12.36 hrs HW=199.71' (Free Discharge) 2=Culvert (Inlet Controls 0.15 cfs @ 1.76 fps)

### **Summary for Pond RG-1: Rain Garden-1**

Inflow Area =	0.276 ac, 43.12% Impervious, Inflow D	epth > 1.29" for Middlesex-025yr event
Inflow =	0.31 cfs @ 12.12 hrs, Volume=	0.030 af
Outflow =	0.24 cfs @ 12.37 hrs, Volume=	0.030 af, Atten= 22%, Lag= 15.0 min
Discarded =	0.04 cfs @ 12.37 hrs, Volume=	0.018 af
Primary =	0.20 cfs @ 12.37 hrs, Volume=	0.012 af

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Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 198.89' @ 12.37 hrs Surf.Area= 653 sf Storage= 262 cf

Plug-Flow detention time= 57.5 min calculated for 0.030 af (100% of inflow)

Center-of-Mass det. time= 57.4 min (872.1 - 814.8)

Volume	Inve	rt Ava	il.Storage	Storage Description						
#1	195.7	4'	339 с	f Custom Stage	Data (Prismatic)	Listed below (Recalc)				
Elevation	on S	Surf.Area	Voids	Inc.Store	Cum.Store					
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)					
195.7	74	202	0.0	0	0					
195.7	75	202	30.0	1	1					
196.7	<b>7</b> 5	202	30.0	61	61					
196.7	76	202	10.0	0	61					
198.5	50	202	10.0	35	97					
198.5	51	202	100.0	2	99					
199.0	00	780	100.0	241	339					
Device	Routing	In	vert Ou	utlet Devices						
#1	Discarded	d 195	5.74' <b>2.</b> 4	410 in/hr Exfiltrati	on over Surface	area				
#2	Primary	198	3.80' <b>3.</b> 0	0' long x 4.0' brea	dth Broad-Crest	ed Rectangular Weir				
	-		He	ead (feet) 0.20 0.4	40 0.60 0.80 1.0	00 1.20 1.40 1.60 1.80 2.00				
				2.50 3.00 3.50 4.00 4.50 5.00 5.50						
			Co	oef. (English) 2.38	2.54 2.69 2.68	2.67 2.67 2.65 2.66 2.66				
	7 3.32									

**Discarded OutFlow** Max=0.04 cfs @ 12.37 hrs HW=198.89' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.20 cfs @ 12.37 hrs HW=198.89' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.20 cfs @ 0.72 fps)

# **Summary for Pond RG-2: Rain Garden-2**

Inflow Area = 0.334 ac, 2.69% Impervious, Inflow Depth > 1.93" for Middlesex-025yr event Inflow = 0.73 cfs @ 12.09 hrs, Volume= 0.054 af Outflow = 0.10 cfs @ 12.88 hrs, Volume= 0.054 af, Atten= 87%, Lag= 47.3 min Discarded = 0.10 cfs @ 12.88 hrs, Volume= 0.054 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 201.11' @ 12.88 hrs Surf.Area= 1,731 sf Storage= 880 cf

Plug-Flow detention time= 133.6 min calculated for 0.054 af (100% of inflow)

Center-of-Mass det. time= 132.9 min ( 986.8 - 853.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	197.74'	1,891 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
	<u> </u>			
197.74	561	0.0	0	0
197.75	561	30.0	2	2
198.75	561	30.0	168	170
198.76	561	10.0	1	171
200.50	561	10.0	98	268
200.51	561	100.0	6	274
201.00	1,248	100.0	443	717
201.50	3,450	100.0	1,175	1,891

Device Routing Invert Outlet Devices

#1 Discarded 197.74' **2.410 in/hr Exfiltration over Surface area** 

**Discarded OutFlow** Max=0.10 cfs @ 12.88 hrs HW=201.11' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.10 cfs)

### Summary for Pond RG-3: Rain Garden-3

Inflow Area = 0.141 ac, 0.00% Impervious, Inflow Depth > 2.51" for Middlesex-025yr event

Inflow = 0.41 cfs @ 12.09 hrs, Volume= 0.030 af

Outflow = 0.08 cfs @ 12.55 hrs, Volume= 0.030 af, Atten= 80%, Lag= 27.4 min

Discarded = 0.08 cfs @ 12.55 hrs, Volume= 0.030 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 202.15' @ 12.55 hrs Surf.Area= 1,491 sf Storage= 442 cf

Plug-Flow detention time= 79.3 min calculated for 0.030 af (100% of inflow)

Center-of-Mass det. time= 79.1 min ( 915.4 - 836.3 )

Volume	Invert Ava	il.Storage	Storage Description						
#1	199.04'	944 cf	Custom Stage [	<b>Data (Prismatic)</b> Lis	ted below (Recalc)				
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)					
199.04	301	0.0	Ó	0					
199.05	301	30.0	1	1					
200.05	301	30.0	90	91					
200.06	301	10.0	0	92					
201.80	301	10.0	52	144					
201.81	301	100.0	3	147					
202.00	945	100.0	118	265					
202.40	2,450	100.0	679	944					

Device Routing Invert Outlet Devices

#1 Discarded 199.04' 2.410 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.08 cfs @ 12.55 hrs HW=202.15' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.08 cfs)

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-1A: PDA-1A Runoff Area=0.048 ac 27.08% Impervious Runoff Depth>3.49"

Tc=6.0 min CN=78 Runoff=0.20 cfs 0.014 af

Subcatchment PDA-1B: PDA-1B Runoff Area=0.112 ac 0.00% Impervious Runoff Depth>1.94"

Tc=6.0 min CN=61 Runoff=0.24 cfs 0.018 af

Subcatchment PDA-1C: PDA-1C Runoff Area=0.164 ac 72.56% Impervious Runoff Depth>5.08"

Tc=6.0 min CN=93 Runoff=0.91 cfs 0.069 af

Subcatchment PDA-1D: PDA-1D Runoff Area=0.334 ac 2.69% Impervious Runoff Depth>2.36"

Tc=6.0 min CN=66 Runoff=0.91 cfs 0.066 af

Subcatchment PDA-1E: PDA-1E Runoff Area=0.141 ac 0.00% Impervious Runoff Depth>3.00"

Tc=6.0 min CN=73 Runoff=0.50 cfs 0.035 af

Subcatchment PDA-2: PDA-2 Runoff Area=0.267 ac 0.00% Impervious Runoff Depth>2.02"

Tc=6.0 min CN=62 Runoff=0.61 cfs 0.045 af

Subcatchment PDA-3: PDA-3 Runoff Area=0.143 ac 4.90% Impervious Runoff Depth>2.02"

Tc=6.0 min CN=62 Runoff=0.32 cfs 0.024 af

Reach DP-1: Central Street Inflow=0.35 cfs 0.031 af

Outflow=0.35 cfs 0.031 af

Reach DP-2: Railroad Street Inflow=0.61 cfs 0.045 af

Outflow=0.61 cfs 0.045 af

Reach DP-3: Front Street Inflow=0.32 cfs 0.024 af

Outflow=0.32 cfs 0.024 af

Pond Pavers: Pavers Peak Elev=199.77' Storage=621 cf Inflow=0.91 cfs 0.069 af

Discarded=0.12 cfs 0.051 af Primary=0.17 cfs 0.019 af Outflow=0.30 cfs 0.069 af

Pond RG-1: Rain Garden-1 Peak Elev=198.91' Storage=273 cf Inflow=0.37 cfs 0.037 af

Discarded=0.04 cfs 0.020 af Primary=0.26 cfs 0.017 af Outflow=0.29 cfs 0.037 af

Pond RG-2: Rain Garden-2 Peak Elev=201.22' Storage=1,096 cf Inflow=0.91 cfs 0.066 af

Outflow=0.12 cfs 0.064 af

Pond RG-3: Rain Garden-3 Peak Elev=202.21' Storage=544 cf Inflow=0.50 cfs 0.035 af

Outflow=0.10 cfs 0.035 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.271 af Average Runoff Depth = 2.69" 87.76% Pervious = 1.061 ac 12.24% Impervious = 0.148 ac

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## **Summary for Subcatchment PDA-1A: PDA-1A**

Runoff = 0.20 cfs @ 12.09 hrs, Volume= 0.014 af, Depth> 3.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

_	Area	(ac)	CN	Desc	Description					
	0.	026	61	>75%	√ Grass co	over, Good	, HSG B			
*	0.	013	98	Impe	rvious Su	face, HSG	БВ			
*	0.	.009	96	Ston	Stone Dust Path, HSG B					
	0.	048	78	Weig						
	0.035 72.92% Pervious Area					us Area				
	0.013 27.08% Impervious Area			8% Imperv	ious Area					
	Tc	Leng	th	Slope	Velocity	Capacity	Description			
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)				
	6.0						Direct Entry, Tc Min.			

# **Summary for Subcatchment PDA-1B: PDA-1B**

Runoff = 0.24 cfs @ 12.10 hrs, Volume= 0.018 af, Depth> 1.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

_	Area	(ac)	CN	Desc	cription				
	0.	0.112 61 >75% Grass cover, Good, HSG B							
	0.112 100.00% Pervious Area								
	Tc	Leng	gth	Slope	Velocity	Capacity	Description		
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)			
	6.0						Direct Entry, Tc Min.		

# **Summary for Subcatchment PDA-1C: PDA-1C**

Runoff = 0.91 cfs @ 12.08 hrs, Volume= 0.069 af, Depth> 5.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

	Area (ac)	CN	Description
	0.021	61	>75% Grass cover, Good, HSG B
*	0.024	96	Stone Dust Path, HSG B
*	0.119	98	Pavers, HSG B
	0.164	93	Weighted Average
	0.045		27.44% Pervious Area
	0.119		72.56% Impervious Area

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
 6.0					Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-1D: PDA-1D**

Runoff = 0.91 cfs @ 12.09 hrs, Volume= 0.066 af, Depth> 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

	Area	(ac)	CN	Desc	Description						
	0.	046	55 Woods, Good, HSG B								
	0.	233	61	>759	>75% Grass cover, Good, HSG B						
*	0.	046	96	Ston	Stone Dust Path, HSG B						
*	0.	009	, ,								
-	0.	0.334 66 Weighted Average									
	0.325 97.31% Pervious Area					us Area					
	0.009 2.			2.69	% Impervi	ous Area					
	Тс	Leng	jth	Slope	Velocity	Capacity	Description				
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0			•			Direct Entry, Tc Min.				

# **Summary for Subcatchment PDA-1E: PDA-1E**

Runoff = 0.50 cfs @ 12.09 hrs, Volume= 0.035 af, Depth> 3.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

	Area	(ac)	CN	Desc	cription		
0.003 55 Woods, Good, HSG B							
0.088 61 >75% Grass cover, Good, HSG B					, HSG B		
*	0.	0.050 96 Stone Dust Path, HSG B					
	0.	0.141 73 Weighted Average					
	0.	0.141 100.00% Pervious Area				ous Area	
	Tc	Leng	•	Slope	Velocity	Capacity	Description
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

# **Summary for Subcatchment PDA-2: PDA-2**

Runoff = 0.61 cfs @ 12.09 hrs, Volume= 0.045 af, Depth> 2.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

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	Area	(ac)	CN	Desc	Description					
	0.	0.081 55 Woods, Good, HSG B								
	0.164 61 >75% Grass cover, Good, HSG B					, HSG B				
*	0.	0.022 96 Stone Dust Path, HSG B								
	0.	0.267 62 Weighted Average								
	0.	0.267 100.00% Pervious Area								
	Тс	Tc Length		Slope	Velocity	Capacity	Description			
	(min)	(fee	,	(ft/ft)	(ft/sec)	(cfs)				
	6.0						Direct Entry, Tc Min.			

# **Summary for Subcatchment PDA-3: PDA-3**

Runoff = 0.32 cfs @ 12.09 hrs, Volume= 0.024 af, Depth> 2.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-050yr Rainfall=5.90"

_	Area (	(ac)	CN	Desc	cription						
	0.0	039	55	Woo	Woods, Good, HSG B						
	0.0	092	61	>75%	>75% Grass cover, Good, HSG B						
*	0.0	005	96	Ston	Stone Dust Path, HSG B						
*	0.0	007	98	Impe	mpervious Surface, HSG B						
	0.	143	62	Weig	ghted Aver	age					
	0.136 95.10% Pervious Area										
	0.0	0.007 4.90% Impervious Area									
	Tc	Leng	th	Slope	Velocity	Capacity	Description				
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0						Direct Entry, Tc Min.				

# **Summary for Reach DP-1: Central Street**

Inflow Area = 0.799 ac, 17.65% Impervious, Inflow Depth > 0.46" for Middlesex-050yr event

Inflow = 0.35 cfs @ 12.25 hrs. Volume = 0.031 af

Outflow = 0.35 cfs @ 12.25 hrs, Volume= 0.031 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

# Summary for Reach DP-2: Railroad Street

Inflow Area = 0.267 ac, 0.00% Impervious, Inflow Depth > 2.02" for Middlesex-050yr event

Inflow = 0.61 cfs @ 12.09 hrs, Volume= 0.045 af

Outflow = 0.61 cfs @ 12.09 hrs, Volume= 0.045 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

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# **Summary for Reach DP-3: Front Street**

Inflow Area = 0.143 ac, 4.90% Impervious, Inflow Depth > 2.02" for Middlesex-050yr event

Inflow = 0.32 cfs @ 12.09 hrs, Volume= 0.024 af

Outflow = 0.32 cfs @ 12.09 hrs, Volume= 0.024 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

### **Summary for Pond Pavers: Pavers**

Inflow Area = 0.164 ac, 72.56% Impervious, Inflow Depth > 5.08" for Middlesex-050yr event

Inflow = 0.91 cfs @ 12.08 hrs, Volume= 0.069 af

Outflow = 0.30 cfs @ 12.37 hrs, Volume= 0.069 af, Atten= 67%, Lag= 17.4 min

Discarded = 0.12 cfs @ 11.66 hrs, Volume= 0.051 af Primary = 0.17 cfs @ 12.37 hrs, Volume= 0.019 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 199.77' @ 12.37 hrs Surf.Area= 5,190 sf Storage= 621 cf

Plug-Flow detention time= 13.5 min calculated for 0.069 af (100% of inflow)

Center-of-Mass det. time= 13.3 min (786.2 - 772.9)

Volume	Invert Av	ail.Storage	Storage Descrip	tion		_
#1	199.37'	1,448 cf	Custom Stage I	Data (Prismatic) Lis	sted below (Recalc)	_
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
199.37 199.38 200.30	5,190 5,190 5,190	30.0	0 16 1,432	0 16 1,448		

Device	Routing	Invert	Outlet Devices
#1	Discarded		1.020 in/hr Exfiltration over Surface area
#2	Primary	199.30'	<b>4.0" Round Culvert</b> L= 17.0' Ke= 1.000 Inlet / Outlet Invert= 199.30' / 198.90' S= 0.0235 '/' Culvert

Inlet / Outlet Invert= 199.30' / 198.90' S= 0.0235 '/' Cc= 0.900 n= 0.020 Corrugated PE, corrugated interior, Flow Area= 0.09 sf

**Discarded OutFlow** Max=0.12 cfs @ 11.66 hrs HW=199.38' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.17 cfs @ 12.37 hrs HW=199.77' (Free Discharge) 2=Culvert (Inlet Controls 0.17 cfs @ 1.99 fps)

# **Summary for Pond RG-1: Rain Garden-1**

Inflow Area =	0.276 ac, 43.12% Impervious, Inflow De	epth > 1.60" for Middlesex-050yr event
Inflow =	0.37 cfs @ 12.11 hrs, Volume=	0.037 af
Outflow =	0.29 cfs @ 12.29 hrs, Volume=	0.037 af, Atten= 21%, Lag= 11.0 min
Discarded =	0.04 cfs @ 12.29 hrs, Volume=	0.020 af
Primary =	0.26 cfs @ 12.29 hrs, Volume=	0.017 af

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Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 198.91' @ 12.29 hrs Surf.Area= 672 sf Storage= 273 cf

Plug-Flow detention time= 52.9 min calculated for 0.037 af (100% of inflow)

Center-of-Mass det. time= 52.8 min ( 865.0 - 812.2 )

Volume	Inve	rt Avai	I.Storage	Storage Descrip				
#1	195.74	1'	339 cf	Custom Stage	Custom Stage Data (Prismatic) Listed below (Recalc)			
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
195.7	74	202	0.0	0	0			
195.7	75	202	30.0	1	1			
196.7	75	202	30.0	61	61			
196.7	76	202	10.0	0	61			
198.5	50	202	10.0	35	97			
198.5	51	202	100.0	2	99			
199.0	00	780	100.0	241	339			
Device Routing Invert Outlet Devices								
#1	Discarded	195	5.74' <b>2.4</b> 1	410 in/hr Exfiltration over Surface area				
#2 Primary 198.80'			3.0' long x 4.0' breadth Broad-Crested Rectangular Weir					
Head (feet) 0.20 0.40 0.60 0.80 1.0								
2.50 3.00 3.50 4.00 4.50 5.00 5.50								
			Coe	ef. (English) 2.38	2.54 2.69 2.68	2.67 2.67 2.65 2.66 2.66		
2.68 2.72 2.73 2.76 2.79			3 2.79 2.88 3.07	3.32				

**Discarded OutFlow** Max=0.04 cfs @ 12.29 hrs HW=198.91' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.25 cfs @ 12.29 hrs HW=198.91' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.25 cfs @ 0.78 fps)

# Summary for Pond RG-2: Rain Garden-2

Inflow Area = 0.334 ac, 2.69% Impervious, Inflow Depth > 2.36" for Middlesex-050yr event 
Inflow = 0.91 cfs @ 12.09 hrs, Volume= 0.066 af 
Outflow = 0.12 cfs @ 12.80 hrs, Volume= 0.064 af, Atten= 86%, Lag= 42.7 min 
Discarded = 0.12 cfs @ 12.80 hrs, Volume= 0.064 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 201.22' @ 12.80 hrs Surf.Area= 2,212 sf Storage= 1,096 cf

Plug-Flow detention time= 137.3 min calculated for 0.064 af (97% of inflow)

Center-of-Mass det. time= 121.3 min ( 969.2 - 847.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	197.74'	1,891 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
197.74	561	0.0	0	0
197.75	561	30.0	2	2
198.75	561	30.0	168	170
198.76	561	10.0	1	171
200.50	561	10.0	98	268
200.51	561	100.0	6	274
201.00	1,248	100.0	443	717
201.50	3,450	100.0	1,175	1,891

Device Routing Invert Outlet Devices

#1 Discarded 197.74' **2.410 in/hr Exfiltration over Surface area** 

**Discarded OutFlow** Max=0.12 cfs @ 12.80 hrs HW=201.22' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

### **Summary for Pond RG-3: Rain Garden-3**

Inflow Area = 0.141 ac, 0.00% Impervious, Inflow Depth > 3.00" for Middlesex-050yr event

Inflow = 0.50 cfs @ 12.09 hrs, Volume= 0.035 af

Outflow = 0.10 cfs @ 12.55 hrs, Volume= 0.035 af, Atten= 81%, Lag= 27.6 min

Discarded = 0.10 cfs @ 12.55 hrs, Volume= 0.035 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 202.21' @ 12.55 hrs Surf.Area= 1,730 sf Storage= 544 cf

Plug-Flow detention time= 81.8 min calculated for 0.035 af (100% of inflow)

Center-of-Mass det. time= 81.6 min ( 912.8 - 831.2 )

Volume	Invert Ava	il.Storage	Storage Description					
#1	199.04'	944 cf	Custom Stage D	Data (Prismatic) Listed be	low (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)				
199.04	301	0.0	0	0				
199.05	301	30.0	1	1				
200.05	301	30.0	90	91				
200.06	301	10.0	0	92				
201.80	301	10.0	52	144				
201.81	301	100.0	3	147				
202.00	945	100.0	118	265				
202.40	2,450	100.0	679	944				

Device Routing Invert Outlet Devices

#1 Discarded 199.04' 2.410 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.10 cfs @ 12.55 hrs HW=202.21' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.10 cfs)

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-1A: PDA-1A Runoff Area=0.048 ac 27.08% Impervious Runoff Depth>4.47"

Tc=6.0 min CN=78 Runoff=0.25 cfs 0.018 af

Subcatchment PDA-1B: PDA-1B Runoff Area=0.112 ac 0.00% Impervious Runoff Depth>2.70"

Tc=6.0 min CN=61 Runoff=0.35 cfs 0.025 af

**Subcatchment PDA-1C: PDA-1C**Runoff Area=0.164 ac 72.56% Impervious Runoff Depth>6.17"

Tc=6.0 min CN=93 Runoff=1.09 cfs 0.084 af

Subcatchment PDA-1D: PDA-1D Runoff Area=0.334 ac 2.69% Impervious Runoff Depth>3.20"

Tc=6.0 min CN=66 Runoff=1.25 cfs 0.089 af

Subcatchment PDA-1E: PDA-1E Runoff Area=0.141 ac 0.00% Impervious Runoff Depth>3.93"

Tc=6.0 min CN=73 Runoff=0.65 cfs 0.046 af

Subcatchment PDA-2: PDA-2 Runoff Area=0.267 ac 0.00% Impervious Runoff Depth>2.80"

Tc=6.0 min CN=62 Runoff=0.86 cfs 0.062 af

Subcatchment PDA-3: PDA-3 Runoff Area=0.143 ac 4.90% Impervious Runoff Depth>2.80"

Tc=6.0 min CN=62 Runoff=0.46 cfs 0.033 af

Reach DP-1: Central Street Inflow=0.57 cfs 0.045 af

Outflow=0.57 cfs 0.045 af

Reach DP-2: Railroad Street Inflow=0.86 cfs 0.062 af

Outflow=0.86 cfs 0.062 af

Reach DP-3: Front Street Inflow=0.46 cfs 0.033 af

Outflow=0.46 cfs 0.033 af

Pond Pavers: Pavers Peak Elev=199.89' Storage=810 cf Inflow=1.09 cfs 0.084 af

Discarded=0.12 cfs 0.059 af Primary=0.21 cfs 0.025 af Outflow=0.33 cfs 0.084 af

Pond RG-1: Rain Garden-1 Peak Elev=198.94' Storage=296 cf Inflow=0.50 cfs 0.050 af

Discarded=0.04 cfs 0.022 af Primary=0.39 cfs 0.027 af Outflow=0.43 cfs 0.050 af

Pond RG-2: Rain Garden-2 Peak Elev=201.39' Storage=1,547 cf Inflow=1.25 cfs 0.089 af

Outflow=0.17 cfs 0.085 af

Pond RG-3: Rain Garden-3 Peak Elev=202.31' Storage=749 cf Inflow=0.65 cfs 0.046 af

Outflow=0.12 cfs 0.046 af

Total Runoff Area = 1.209 ac Runoff Volume = 0.358 af Average Runoff Depth = 3.55" 87.76% Pervious = 1.061 ac 12.24% Impervious = 0.148 ac

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# Summary for Subcatchment PDA-1A: PDA-1A

Runoff = 0.25 cfs @ 12.09 hrs, Volume= 0.018 af, Depth> 4.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

			٠	_							
_	Area (	ac)	CN	Desc	Description						
	0.0	026	61	>75%	√ Grass co	over, Good	I, HSG B				
*	0.0	013	98	Impe	rvious Sui	face, HSG	BB				
*	0.0	009	96	Ston	e Dust Pat	th, HSG B					
	0.0	0.048 78 Weighted Average									
0.035 72.92% Pervious Area						us Area					
	0.013 27.08% Impervious Area					rious Area					
	Tc	Leng	th	Slope	Velocity	Capacity	Description				
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0						Direct Entry, Tc Min.				

### **Summary for Subcatchment PDA-1B: PDA-1B**

Runoff = 0.35 cfs @ 12.09 hrs, Volume= 0.025 af, Depth> 2.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

_	Area	(ac)	CN	Desc	cription					
	0.	0.112 61 >75% Grass cover, Good, HSG B								
	0.112 100.00% Pervious Area									
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	6.0						Direct Entry, Tc Min.			

## **Summary for Subcatchment PDA-1C: PDA-1C**

Runoff = 1.09 cfs @ 12.08 hrs, Volume= 0.084 af, Depth> 6.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

	Area (ac)	CN	Description
	0.021	61	>75% Grass cover, Good, HSG B
*	0.024	96	Stone Dust Path, HSG B
*	0.119	98	Pavers, HSG B
	0.164	93	Weighted Average
	0.045		27.44% Pervious Area
	0.119		72.56% Impervious Area

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-1D: PDA-1D**

Runoff = 1.25 cfs @ 12.09 hrs, Volume= 0.089 af, Depth> 3.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

	Area	(ac)	CN	Desc	cription			
	0.	046	55	Woo	ds, Good,	HSG B		
	0.	233	61	>759	% Grass co	over, Good	, HSG B	
*	0.	046	96	Ston	e Dust Pat	th, HSG B		
*	0.	009	98	Impe	ervious Sui	face, HSG	В	
-	0.	0.334 66 Weighted Average						
	0.325 97.31% Pervious Area					us Area		
	0.009 2.69% Impervious Area			% Impervi	ous Area			
	Тс	Leng	jth	Slope	Velocity	Capacity	Description	
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)		
	6.0			•			Direct Entry, Tc Min.	

# **Summary for Subcatchment PDA-1E: PDA-1E**

Runoff = 0.65 cfs @ 12.09 hrs, Volume= 0.046 af, Depth> 3.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

	Area	(ac)	CN	Desc	cription		
	0.	003	55	Woo	ds, Good,	HSG B	
	0.088 61 >75% Grass cover, Good, HSG B						
*	0.	050	96	Ston	e Dust Pa	th, HSG B	
	0.	0.141 73 Weighted Average					
	0.	0.141 100.00% Pervious Area					
	Tc	Leng	•	Slope	Velocity	Capacity	Description
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

# **Summary for Subcatchment PDA-2: PDA-2**

Runoff = 0.86 cfs @ 12.09 hrs, Volume= 0.062 af, Depth> 2.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

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	Area	(ac)	CN	Desc	cription		
	0.	081	55	Woo	ds, Good,	HSG B	
	0.	164	, HSG B				
*	0.	022	96	Ston	e Dust Pat	th, HSG B	
	0.	267	62	Weig	ghted Aver	age	
	0.	0.267 100.00% Pervious Area					
	_					_	
	Tc	Leng	ıth	Slope	Velocity	Capacity	Description
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Tc Min.

### **Summary for Subcatchment PDA-3: PDA-3**

Runoff = 0.46 cfs @ 12.09 hrs, Volume= 0.033 af, Depth> 2.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr Middlesex-100yr Rainfall=7.00"

	Area	(ac)	CN	Desc	cription				
	0.	039	55	Woo	ds, Good,	HSG B			
	0.	092	61	>759	% Grass co	over, Good	I, HSG B		
*	0.	005	96	Ston	e Dust Pat	th, HSG B			
*	0.	007	07 98 Impervious Surface, HSG B						
	0.	143	62	Weig	ghted Aver	age			
	0.136 95.10% Pervious Area					us Area			
	0.007 4.90% Impervious Area					ous Area			
	Тс	Leng	th	Slope	Velocity	Capacity	Description		
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)			
	6.0						Direct Entry, Tc Min.		

### **Summary for Reach DP-1: Central Street**

Inflow Area = 0.799 ac, 17.65% Impervious, Inflow Depth > 0.68" for Middlesex-100yr event

Inflow = 0.57 cfs @ 12.15 hrs, Volume= 0.045 af

Outflow = 0.57 cfs @ 12.15 hrs, Volume= 0.045 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

# **Summary for Reach DP-2: Railroad Street**

Inflow Area = 0.267 ac, 0.00% Impervious, Inflow Depth > 2.80" for Middlesex-100yr event

Inflow = 0.86 cfs @ 12.09 hrs, Volume= 0.062 af

Outflow = 0.86 cfs @ 12.09 hrs, Volume= 0.062 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

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# **Summary for Reach DP-3: Front Street**

Inflow Area = 0.143 ac, 4.90% Impervious, Inflow Depth > 2.80" for Middlesex-100yr event

Inflow = 0.46 cfs @ 12.09 hrs, Volume= 0.033 af

Outflow = 0.46 cfs @ 12.09 hrs, Volume= 0.033 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

### **Summary for Pond Pavers: Pavers**

Inflow Area = 0.164 ac, 72.56% Impervious, Inflow Depth > 6.17" for Middlesex-100yr event

Inflow = 1.09 cfs @ 12.08 hrs, Volume= 0.084 af

Outflow = 0.33 cfs @ 12.40 hrs, Volume= 0.084 af, Atten= 70%, Lag= 18.9 min

Discarded = 0.12 cfs @ 11.61 hrs, Volume= 0.059 af Primary = 0.21 cfs @ 12.40 hrs, Volume= 0.025 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 199.89' @ 12.40 hrs Surf.Area= 5,190 sf Storage= 810 cf

Plug-Flow detention time= 16.1 min calculated for 0.084 af (100% of inflow)

Center-of-Mass det. time= 15.9 min (784.2 - 768.2)

Volume	Invert A	vail.Storage	Storage Descrip	tion			
#1	199.37' 1,448		Custom Stage Data (Prismatic) Listed below (Recalc)				
Elevation (feet)	Surf.Are (sq-f		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
199.37	5,19	0.0	0	0			
199.38	5,19	0 30.0	16	16			
200.30	5 19	0 30 0	1 432	1 448			

Device	Routing	Invert	Outlet Devices
#1	Discarded	199.37'	1.020 in/hr Exfiltration over Surface area
#2	Primary	199.30'	<b>4.0" Round Culvert</b> L= 17.0' Ke= 1.000

Inlet / Outlet Invert= 199.30' / 198.90' S= 0.0235 '/' Cc= 0.900 n= 0.020 Corrugated PE, corrugated interior, Flow Area= 0.09 sf

**Discarded OutFlow** Max=0.12 cfs @ 11.61 hrs HW=199.38' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.21 cfs @ 12.40 hrs HW=199.89' (Free Discharge) —2=Culvert (Inlet Controls 0.21 cfs @ 2.35 fps)

# Summary for Pond RG-1: Rain Garden-1

Inflow Area =	0.276 ac, 43.12% Impervious, Inflow De	epth > 2.19" for Middlesex-100yr event
Inflow =	0.50 cfs @ 12.10 hrs, Volume=	0.050 af
Outflow =	0.43 cfs @ 12.18 hrs, Volume=	0.050 af, Atten= 15%, Lag= 4.7 min
Discarded =	0.04 cfs @ 12.18 hrs, Volume=	0.022 af
Primary =	0.39 cfs @ 12.18 hrs, Volume=	0.027 af

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Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 198.94' @ 12.18 hrs Surf.Area= 712 sf Storage= 296 cf

Plug-Flow detention time= 46.4 min calculated for 0.050 af (98% of inflow)

Center-of-Mass det. time= 36.9 min ( 846.0 - 809.1 )

Volume	Inve	rt Ava	il.Storage	ge Storage Description				
#1	195.7	4'	339 с	f Custom Stage	Data (Prismatic)	Listed below (Recalc)		
Elevation	on S	Surf.Area	Voids	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)			
195.7	74	202	0.0	0	0			
195.7	75	202	30.0	1	1			
196.7	<b>7</b> 5	202	30.0	61	61			
196.7	76	202	10.0	0	61			
198.5	50	202	10.0	35	97			
198.5	51	202	100.0	2	99			
199.0	00	780	100.0	241	339			
Device	Routing	In	vert Ou	utlet Devices				
#1	Discarded	d 195	5.74' <b>2.</b> 4	410 in/hr Exfiltrati	on over Surface	area		
#2 Primary 198.80'			3.80' <b>3.</b> 0	3.0' long x 4.0' breadth Broad-Crested Rectangular Weir				
He				ead (feet) 0.20 0.4	40 0.60 0.80 1.0	00 1.20 1.40 1.60 1.80 2.00		
				50 3.00´3.50 4.00				
			Co	oef. (English) 2.38	2.54 2.69 2.68	2.67 2.67 2.65 2.66 2.66		
				68 2.72 2.73 2.70				

**Discarded OutFlow** Max=0.04 cfs @ 12.18 hrs HW=198.94' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.38 cfs @ 12.18 hrs HW=198.94' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.38 cfs @ 0.90 fps)

### **Summary for Pond RG-2: Rain Garden-2**

Inflow Area = 0.334 ac, 2.69% Impervious, Inflow Depth > 3.20" for Middlesex-100yr event 
Inflow = 1.25 cfs @ 12.09 hrs, Volume= 0.089 af 
Outflow = 0.17 cfs @ 12.77 hrs, Volume= 0.085 af, Atten= 87%, Lag= 40.6 min 
Discarded = 0.17 cfs @ 12.77 hrs, Volume= 0.085 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 201.39' @ 12.77 hrs Surf.Area= 2,978 sf Storage= 1,547 cf

Plug-Flow detention time= 139.7 min calculated for 0.085 af (95% of inflow)

Center-of-Mass det. time= 113.1 min ( 952.1 - 838.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	197.74'	1,891 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
197.74	561	0.0	0	0
197.75	561	30.0	2	2
198.75	561	30.0	168	170
198.76	561	10.0	1	171
200.50	561	10.0	98	268
200.51	561	100.0	6	274
201.00	1,248	100.0	443	717
201.50	3,450	100.0	1,175	1,891

Device Routing Invert Outlet Devices

#1 Discarded 197.74' **2.410 in/hr Exfiltration over Surface area** 

**Discarded OutFlow** Max=0.17 cfs @ 12.77 hrs HW=201.39' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.17 cfs)

### **Summary for Pond RG-3: Rain Garden-3**

Inflow Area = 0.141 ac, 0.00% Impervious, Inflow Depth > 3.93" for Middlesex-100yr event

Inflow = 0.65 cfs @ 12.09 hrs, Volume= 0.046 af

Outflow = 0.12 cfs @ 12.56 hrs, Volume= 0.046 af, Atten= 82%, Lag= 28.0 min

Discarded = 0.12 cfs @ 12.56 hrs, Volume= 0.046 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 202.31' @ 12.56 hrs Surf.Area= 2,129 sf Storage= 749 cf

Plug-Flow detention time= 88.3 min calculated for 0.046 af (100% of inflow)

Center-of-Mass det. time= 85.7 min ( 909.2 - 823.5 )

Volume	Invert Ava	il.Storage	Storage Descrip		
#1	199.04'	944 cf	Custom Stage I	Data (Prismatic) Lis	sted below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
199.04	301	0.0	0	0	
199.05	301	30.0	1	1	
200.05	301	30.0	90	91	
200.06	301	10.0	0	92	
201.80	301	10.0	52	144	
201.81	301	100.0	3	147	
202.00	945	100.0	118	265	
202.40	2,450	100.0	679	944	

Device Routing Invert Outlet Devices

#1 Discarded 199.04' 2.410 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.12 cfs @ 12.56 hrs HW=202.31' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Attachment 3 Recharge and Drawdown Calculations



### **Groundwater Recharge Volume Required:**

#### Rv = F x Impervious Area, where:

Rv = Required Recharge Volume [Ac-ft]

**F** = Target Depth Factor associated with each Hydrologic Soil Group (HSG) [in]

Impervious Area = Total Impervious Area Under Proposed Conditions [Ac]

Notes: 1) Goal to provide maximum recharge based on an HSG A soil, given in-situ testing

2) Permeable pavers broken out separately and not fully impervious

			Impervious Area	Required Recharge	
_			[Acres]	Volume [Ac-ft]	_
HSG "A", use F =	0.6	in	0.029	0.001	
			0.119	0.006	* For Perm. Paver Footprint
Total R	equire	d Recl	harge Volume (Rv) =	0.007	Ac-ft

### <u>Capture Area Adjustment:</u> (Ref: DEP Handbook V.3 Ch.1 P.27-28)

Adjusted Required Recharge Volume = Ca x Rv	0.009 Ac-ft
Capture Area Adjustment Factor = (Total)/(Infil) = Ca =	1.16
Percent Imp. Area Draining to Infiltrative BMPs =	86.5%
Impervious Area Draining to Infiltrative BMPs (infil) =	0.128 Acres
Total Site Impervious Area (Total)=	0.148 Acres

### **Groundwater Recharge Volume Provided:**

ВМР	Provided Recharge Volume [Ac-ft]	
Permeable Pavers	0.033	_
Rain Garden - 1	0.005	
Rain Garden - 2	0.043	
Rain Garden - 3	0.021	
Total Provided Recharge Volume =	0.102	Ac-ft

PROVIDED GROUNDWATER RECHARGE VOLUME IS GREATER THAN OR EQUAL TO THE REQUIRED RECHARGE VOLUME, THEREFORE PROPOSED STORMWATER MANAGEMENT DESIGN IS IN COMPLIANCE WITH STANDARD 3.

 JOB NO. 1439.03
 COMPUTED BY:
 NBB
 CHECKED BY:
 JRM

 JOB: Blair Square
 DATE:
 09/02/21
 DATE:
 09/03/21

Rv = Storage Volume Below Outlet [Ac-ft] Rv Drawdown Time = · where: (K) (Bottom Area)

K= Infiltration Rate [in/hr]

Bottom Area = Bottom Area of Recharge System [Ac]

**Permeable Pavers** 

Rv = 0.033 Ac-ft

1.020 in/hr K =

Bottom Area = 0.119 Acres

Drawdown Time = 3.262 Hours < 72 Hours, Design is in compliance with the standard.

Rain Garden - 1

0.005 Ac-ft Rv =

2.410 in/hr K =

Bottom Area = 0.005 Acres

Drawdown Time = 4.979 Hours < 72 Hours, Design is in compliance with the standard.

Rain Garden - 2

Rv =0.043 Ac-ft

2.410 in/hr

0.013 Acres Bottom Area =

Drawdown Time = 16.470 Hours < 72 Hours, Design is in compliance with the standard.

Rain Garden - 3

Rv = 0.021 Ac-ft

K = 2.410 in/hr

Bottom Area = 0.007 Acres

Drawdown Time = 14.938 Hours < 72 Hours, Design is in compliance with the standard.

#### Note:

- 1. The infiltration BMPs have been designed to fully drain within 72 hours, therefore the proposed stormwater management design is in compliance with Standard 3.
- 2. Infiltration Rate based on Volume 3, Chapter 1, Table 2.3.3 Rawls Rates from the 2008 MA DEP Stormwater Management Handbook.

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199.85

199.86

199.87

199.88

0.119

0.119

0.119

0.119

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# Stage-Area-Storage for Pond Pavers: Pavers

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(acres)	(acre-feet)	(feet)	(acres)	(acre-feet)
199.37	0.119	0.000	199.89	0.119	0.019
199.38	0.119	0.000	199.90	0.119	0.019
199.39	0.119	0.001	199.91	0.119	0.019
199.40	0.119	0.001	199.92	0.119	0.020
199.41	0.119	0.001	199.93	0.119	0.020
199.42	0.119	0.002	199.94	0.119	0.020
199.43	0.119	0.002	199.95	0.119	0.021
199.44	0.119	0.003	199.96	0.119	0.021
199.45	0.119	0.003	199.97	0.119	0.021
199.46	0.119	0.003	199.98	0.119	0.022
199.47	0.119	0.004	199.99	0.119	0.022
199.48	0.119	0.004	200.00	0.119	0.023
199.49	0.119	0.004	200.01	0.119	0.023
199.50	0.119	0.005	200.02	0.119	0.023
199.51	0.119	0.005 0.005	200.03	0.119	0.024
199.52 199.53	0.119 0.119	0.005	200.04 200.05	0.119 0.119	0.024 0.024
199.54	0.119	0.006	200.03	0.119	0.024
199.55	0.119	0.006	200.07	0.119	0.025
199.56	0.119	0.007	200.08	0.119	0.025
199.57	0.119	0.007	200.09	0.119	0.026
199.58	0.119	0.008	200.10	0.119	0.026
199.59	0.119	0.008	200.11	0.119	0.026
199.60	0.119	0.008	200.12	0.119	0.027
199.61	0.119	0.009	200.13	0.119	0.027
199.62	0.119	0.009	200.14	0.119	0.028
199.63	0.119	0.009	200.15	0.119	0.028
199.64	0.119	0.010	200.16	0.119	0.028
199.65	0.119	0.010	200.17	0.119	0.029
199.66	0.119	0.010	200.18	0.119	0.029
199.67	0.119	0.011	200.19	0.119	0.029
199.68	0.119	0.011	200.20	0.119	0.030
199.69	0.119	0.011	200.21	0.119	0.030
199.70	0.119	0.012	200.22	0.119	0.030
199.71 199.72	0.119 0.119	0.012 0.013	200.23 200.24	0.119 0.119	0.031 0.031
199.72	0.119	0.013	200.24	0.119	0.031
199.74	0.119	0.013	200.25	0.119	0.031
199.75	0.119	0.013	200.27	0.119	0.032
199.76	0.119	0.014	200.28	0.119	0.033
199.77	0.119	0.014	200.29	0.119	0.033
199.78	0.119	0.015	200.30	0.119	0.033
199.79	0.119	0.015			
199.80	0.119	0.015			
199.81	0.119	0.016			
199.82	0.119	0.016			
199.83	0.119	0.016			
199.84	0.119	0.017			
100.95	0 110	0.017	İ		

0.017

0.018

0.018

0.018

Storage (acre-feet)

0.002

0.002

0.002

0.002

0.002

0.003

0.003

0.004

0.004

0.005

0.005

 $0.006 \\ 0.007$ 

0.008

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# Stage-Area-Storage for Pond RG-1: Rain Garden-1

Surface

(acres) 0.005

0.005

0.005

0.005

0.005

0.007

0.008

0.010

0.011

0.012

0.014

0.015

0.016

0.018

Elevation	Surface	Storage	Elevation
(feet)	(acres)	(acre-feet)	(feet)
195.74	0.005	0.000	198.34
195.79	0.005	0.000	198.39
195.84	0.005	0.000	198.44
195.89	0.005	0.000	198.49
195.94	0.005	0.000	198.54
195.99	0.005	0.000	198.59
196.04	0.005	0.000	198.64
196.09	0.005	0.000	198.69
196.14	0.005	0.001	198.74
196.19	0.005	0.001	198.79
196.24	0.005	0.001	198.84
196.29	0.005	0.001	198.89
196.34	0.005	0.001	198.94
196.39	0.005	0.001	198.99
196.44	0.005	0.001	100.00
196.49	0.005	0.001	
196.54	0.005	0.001	
196.59	0.005	0.001	
196.64	0.005	0.001	
196.69	0.005	0.001	
196.74	0.005	0.001	
196.74	0.005	0.001	
196.79	0.005	0.001	
196.89	0.005	0.001	
196.94	0.005	0.001	
196.99	0.005	0.001	
190.99	0.005	0.002	
197.09	0.005	0.002	
197.14	0.005	0.002	
197.19	0.005	0.002	
197.19	0.005	0.002	
197.29	0.005	0.002	
197.34	0.005	0.002	
197.39	0.005	0.002	
197.44	0.005	0.002	
197.49	0.005	0.002	
197.54	0.005	0.002	
197.59	0.005	0.002	
197.64	0.005	0.002	
197.69	0.005	0.002	
197.74	0.005	0.002	
197.79	0.005	0.002	
197.79	0.005	0.002	
197.89	0.005	0.002	
197.94	0.005	0.002	
197.99	0.005	0.002	
198.04	0.005	0.002	
198.09	0.005	0.002	
198.14	0.005	0.002	
198.19	0.005	0.002	
198.24	0.005	0.002	
198.29	0.005	0.002	
100.20	0.000	0.002	

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# Stage-Area-Storage for Pond RG-2: Rain Garden-2

			_		
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(acres)	(acre-feet)	(feet)	(acres)	(acre-feet)
197.74	0.013	0.000	200.34	0.013	0.006
197.79	0.013	0.000	200.39	0.013	0.006
197.84	0.013	0.000	200.44	0.013	0.006
197.89	0.013	0.001	200.49	0.013	0.006
197.94	0.013	0.001	200.54	0.014	0.007
197.99	0.013	0.001	200.59	0.015	0.007
198.04	0.013	0.001	200.64	0.017	0.008
198.09	0.013	0.001	200.69	0.019	0.009
198.14	0.013	0.002	200.74	0.020	0.010
198.19	0.013	0.002	200.79	0.022	0.011
198.24	0.013	0.002	200.84	0.024	0.012
198.29 198.34	0.013	0.002 0.002	200.89 200.94	0.025	0.014 0.015
198.34	0.013 0.013	0.002	200.94	0.027 0.028	0.015
196.39		0.003	200.99		0.018
198.49	0.013 0.013	0.003	201.04	0.033 0.038	0.018
198.54	0.013	0.003	201.09	0.036	0.019
198.59	0.013	0.003	201.14	0.043	0.021
198.64	0.013	0.003	201.19	0.053	0.024
198.69	0.013	0.003	201.24	0.058	0.020
198.74	0.013	0.004	201.23	0.063	0.029
198.79	0.013	0.004	201.39	0.068	0.032
198.84	0.013	0.004	201.44	0.003	0.039
198.89	0.013	0.004	201.49	0.078	0.039 0.043
198.94	0.013	0.004	201.40	0.070	0.040
198.99	0.013	0.004			
199.04	0.013	0.004			
199.09	0.013	0.004			
199.14	0.013	0.004			
199.19	0.013	0.004			
199.24	0.013	0.005			
199.29	0.013	0.005			
199.34	0.013	0.005			
199.39	0.013	0.005			
199.44	0.013	0.005			
199.49	0.013	0.005			
199.54	0.013	0.005			
199.59	0.013	0.005			
199.64	0.013	0.005			
199.69	0.013	0.005			
199.74	0.013	0.005			
199.79	0.013	0.005			
199.84	0.013	0.005			
199.89	0.013	0.005			
199.94	0.013	0.005			
199.99	0.013	0.005			
200.04	0.013	0.006			
200.09	0.013	0.006			
200.14	0.013	0.006			
200.19	0.013	0.006			
200.24	0.013	0.006			
200.29	0.013	0.006			

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# Stage-Area-Storage for Pond RG-3: Rain Garden-3

		J	J		
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(acres)	(acre-feet)	(feet)	(acres)	(acre-feet)
199.04	0.007	0.000	201.64	0.007	0.003
199.09	0.007	0.000	201.69	0.007	0.003
199.14	0.007	0.000	201.74	0.007	0.003
199.19	0.007	0.000	201.79	0.007	0.003
199.24	0.007	0.000	201.84	0.009	0.004
199.29	0.007	0.001	201.89	0.013	0.004
199.34	0.007	0.001	201.94	0.017	0.005
199.39	0.007	0.001	201.99	0.021	0.006
199.44	0.007	0.001	202.04	0.025	0.007
199.49	0.007	0.001	202.09	0.029	0.008
199.54	0.007	0.001	202.14	0.034	0.010
199.59	0.007	0.001	202.19	0.038	0.012
199.64	0.007	0.001	202.13	0.042	0.012
199.69	0.007	0.001	202.29	0.042	0.014
199.74	0.007	0.001	202.29	0.047	0.018
199.79	0.007	0.001	202.34 202.39		0.018 0.021
	0.007	0.002	202.39	0.055	U.UZ I
199.84					
199.89	0.007	0.002			
199.94	0.007	0.002			
199.99	0.007	0.002			
200.04	0.007	0.002			
200.09	0.007	0.002			
200.14	0.007	0.002			
200.19	0.007	0.002			
200.24	0.007	0.002			
200.29	0.007	0.002			
200.34	0.007	0.002			
200.39	0.007	0.002			
200.44	0.007	0.002			
200.49	0.007	0.002			
200.54	0.007	0.002			
200.59	0.007	0.002			
200.64	0.007	0.003			
200.69	0.007	0.003			
200.74	0.007	0.003			
200.79	0.007	0.003			
200.84	0.007	0.003			
200.89	0.007	0.003			
200.94	0.007	0.003			
200.99	0.007	0.003			
201.04	0.007	0.003			
201.09	0.007	0.003			
201.14	0.007	0.003			
201.19	0.007	0.003			
201.24	0.007	0.003			
201.29	0.007	0.003			
201.34	0.007	0.003			
201.39	0.007	0.003			
201.44	0.007	0.003			
201.49	0.007	0.003			
201.54	0.007	0.003			
201.59	0.007	0.003			
201.00	0.001	0.000			

Attachment 4
Site Owner's Manual

# Site Owner's Manual

# BLAIR SQUARE SITE IMPROVEMENTS

**0** Railroad Bed Holliston, Massachusetts

Prepared for: Holliston Planning Board 703 Washington Street Holliston, MA 01746

Prepared by:



September 3, 2021

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FIGURE 1: SITE PLANS

### **APPENDICES**

APPENDIX A: OPERATION AND MAINTENANCE LOG APPENDIX B: LIST OF EMERGENCY CONTACTS



### 1.0 INTRODUCTION

The Site Owner's Manual complies with the Long-Term Pollution Prevention Plan (Standard 4) and the Long-Term Operation and Maintenance Plan (Standard 9) requirements of the 2008 Massachusetts Department of Environmental Protection (DEP) Stormwater Handbook. The Manual outlines source control and pollution prevention measures and maintenance requirements of stormwater best management practices (BMPs) associated with the proposed development.



### 2.0 SITE OWNER'S AGREEMENT

### 2.1 Operation and Maintenance Compliance Statement

Site Owner: Town of Holliston

703 Washington Street Holliston, MA 01746

Responsible Party: Town of Holliston

The Town of Holliston shall maintain ownership of the on-site stormwater management system as well as the responsibility for operation and maintenance during the post-development stages of the project. The site has been inspected for erosion and appropriate measures have been taken to permanently stabilize any eroded areas. All aspects of stormwater best management practices (BMPs) have been inspected for damage, wear and malfunction, and appropriate steps have been taken to repair or replace the system or portions of the system so that the stormwater at the site may be managed in accordance with the Stormwater Management Standards. Future responsible parties shall be notified of their continuing legal responsibility to operate and maintain the BMPs. The operation and maintenance plan for the stormwater BMPs is being implemented.

Responsible Party Signature	Date

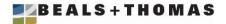
### 2.2 Stormwater Maintenance Easements

There are no off-site areas utilized for stormwater control, therefore no stormwater management easements are required. The Site Owner will have access to all stormwater practices for inspection and maintenance, including direct maintenance access by heavy equipment to structures requiring regular maintenance.

### 2.3 Record Keeping

The Site Owner shall maintain a rolling log in which all inspections and maintenance activities for the past three years shall be recorded. The Operation and Maintenance Log includes information pertaining to inspections, repairs, and disposal relevant to the project's stormwater management system. The Log is located in Appendix A.

The Operation and Maintenance Log shall be made available to the Conservation Commission and the DEP upon request. The Conservation Commission and the DEP shall be allowed to enter and inspect the premises to evaluate and ensure that the responsible party complies with the maintenance requirements for each BMP.



# 2.4 Training

Employees involved in grounds maintenance and emergency response will be educated on the general concepts of stormwater management and groundwater protection. The Site Owner's Manual will be reviewed with the maintenance staff. The staff will be trained on the proper course of action for specific events expected to be incurred during routine maintenance or emergency situations.



### 3.0 LONG-TERM POLLUTION PREVENTION PLAN

In compliance with Standard 4 of the 2008 DEP Stormwater Management Handbook, this section outlines source control and pollution prevention measures to be employed on-site after construction.

### 3.1 Storage of Materials and Waste

The site shall be kept clear of trash and debris at all times. Certain materials and waste products shall be stored inside or outside upon an impervious surface and covered, as required by local and state regulations.

### 3.2 Vehicle Washing

No commercial vehicle washing shall take place on site.

### 3.3 Routine Inspections and Maintenance of Stormwater BMPs

See Section 4.0 Long-Term Operation and Maintenance Plan, for routine inspection and maintenance requirements for all proposed stormwater BMPs.

### 3.4 Spill Prevention and Response

A contingency plan shall be implemented to address the spill or release of petroleum products and hazardous materials and will include the following measures:

- Equipment necessary to quickly attend to inadvertent spills or leaks shall be stored on-site in a secure but accessible location. Such equipment shall include but not be limited to the following: safety goggles, chemically resistant gloves and overshoe boots, water and chemical fire extinguishers, sand and shovels, suitable absorbent materials, storage containers and first aid equipment (i.e. Indian Valley Industries, Inc. 55-gallon Spill Containment kit or approved equivalent).
- Spills or leaks shall be treated properly according to material type, volume of spillage and location of spill. Mitigation shall include preventing further spillage, containing the spilled material in the smallest practical area, removing spilled material in a safe and environmentally-friendly manner, and remediation of any damage to the environment.
- 3. For large spills, Massachusetts DEP Hazardous Waste Incident Response Group shall be notified immediately at 888-304-1133 and an emergency response contractor shall be consulted.



# 3.5 Maintenance of Lawns, Gardens, and other Landscaped Areas

Lawns, gardens, and other landscaped areas shall be maintained regularly by the site owner. Vegetated and landscaped BMPs will be maintained as outlined in Section 4.0.

### 3.6 Storage and Use of Fertilizers, Herbicides, and Pesticides

All fertilizers, herbicides, and pesticides shall be stored in accordance with local, state, and federal regulations. The application rate and use of fertilizers, herbicides, and pesticides on the site shall at no time exceed local, state, or federal specifications.

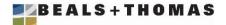
### 3.7 Pet Waste Management

Pet owners shall be required to pick up after their animals and dispose of waste in the trash.

### 3.8 Snow and Deicing Chemical Management

Snow removal at the proposed development shall comply with the following requirements:

- Plowed snow shall be placed in the areas outside of stormwater best management practices. The following maintenance measures shall be undertaken at all snow disposal sites:
  - Debris shall be cleared from an area prior to using it for snow disposal.
  - Debris and accumulated sediments shall be cleared from the site and properly disposed of at the end of the snow season and no later than May 15.
  - Sanding shall not be performed on permeable paver surface as this will clog the pore space within the stone aggregate.



### 4.0 LONG-TERM OPERATION AND MAINTENANCE PLAN

This section outlines the stormwater best management practices (BMPs) associated with the proposed stormwater management system and identifies the long-term inspection and maintenance requirements for each BMP.

### 4.1 Stormwater Management System Components

The following table outlines the type and quantity of the BMPs and their general location. Please reference the site plan(s) provided in the Figures section for exact location.

BMP Type	Quantity	Location
Permeable Pavers	1	Northeastern portion of the site.
Rain Garden	3	Throughout the site.

### 4.2 Inspection and Maintenance Schedules

### 4.2.1 Rain Gardens

Annual maintenance of all rain garden components, including plants, soil, and mulch, shall be performed to ensure the overall success. Specific maintenance activities and their required frequency are outlined below:

- Vegetation shall be watered at the end of the day for 14 consecutive days after planting.
- Trash shall be removed from the surface monthly.
- The soil surface shall be inspected on a monthly basis and any observed erosion shall be repaired.
- All void areas within the bioretention area shall be remulched on an annual basis. If the existing mulch layer has deteriorated, it shall be removed prior to applying the new layer.
- All dead and diseased vegetation shall be removed and replaced on an annual basis. Diseased trees and shrubs shall be treated as necessary.
- Inlet and outlet pipes shall be inspected every 6 months and after major storm events (rainfall totals greater than 2.5 inches in 24 hours) for evidence of clogging.
- During and after major storm, the length of time standing water remains in the rain garden area shall be recorded:
  - If the time is greater than 72 hours, thoroughly inspect the basin for signs of clogging.



 A corrective action plan shall be developed by a qualified professional to restore infiltrative function. The Site Owner shall take immediate action to implement these corrective measures.

#### 4.2.2 Permeable Pavers

Frequent cleaning and maintenance of permeable pavers is critical to prevent clogging. Required operation and maintenance measures include the following:

- Informational signs identifying permeable paver areas shall be posted.
- No winter sanding shall be conducted on the permeable pavers.
- Salt use during winter months shall be minimized.
- Adjacent landscaped areas shall be well maintained to prevent soil from being transported onto pavement.
- Surface of permeable pavers shall be cleaned using vacuum sweeping machines monthly.
- Permeable paver areas shall be monitored regularly to ensure proper drainage after storm events.
- Permeable pavers shall never be resealed or repaved with impermeable materials.
- Surface shall be inspected annually for deterioration.

### 4.3 Estimated Operation and Maintenance Budget

An operations and maintenance budget was prepared to approximate the annual cost of the inspections required in compliance with the DEP Stormwater Management Policy. The table below estimates the annual cost to inspect and maintain each proposed BMP, based on the requirements in Section 4.2.

ВМР Туре	# of BMPS	Annual O&M Cost (per BMP) <sup>1</sup>	Total Cost
Rain Gardens	3	\$200-\$400	\$600-\$1200
Permeable Pavers	1	\$300-\$400	\$300-\$400
		Total	\$900-\$1600

#### 4.4 Public Safety Features

Appropriate signage will be provided within the premises for public safety.

<sup>&</sup>lt;sup>1</sup> Annual maintenance cost is based on estimate of the cost to complete all inspection and maintenance measures outlined in Section 4.2. For BMPs that require sediment removal at regular intervals (i.e. every 5 or 10 years), the annual cost includes the annual percentage of that cost.



# Figures

Figure 1: Site Plans (attached separately)



Appendices



# Appendix A

Operation and Maintenance Log



#### **OPERATION AND MAINTENANCE LOG**

This template is intended to comply with the operation and maintenance log requirements of the 2008 DEP Stormwater Management Handbook. Copies of this log should be made for all inspections and kept on file for three years from the inspection date.

Name/Company of Inspector:
Date/Time of Inspection:
Weather Conditions: (Note current weather and any recent precipitation events)

Stormwater BMP	Inspection Observations	Actions Required



# Appendix B

List of Emergency Contacts



#### **List of Emergency Contacts**

Massachusetts DEP Hazardous Waste Incident Response Group (617) 792-7653

Town of Holliston Fire Department

Emergencies: Dial 911 59 Central Street Holliston, MA 01746 Tel: (508) 429-4631

Fire Chief: Michael R. Cassidy

Town of Holliston Police Department

Emergencies: Dial 911

550 Washington Street (Rt. 16)

Holliston, MA 01746 Tel: (508) 429-1212

Police Chief: Matthew Stone



Attachment 5
Construction Period Erosion Pollution Prevention Plan



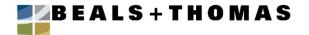
# Construction Sediment, Erosion & Pollution Prevention Plan

# **BLAIR SQUARE SITE IMPROVEMENTS**

0 Railroad Bed Holliston, Massachusetts

Prepared for:
Holliston Planning Board
703 Washington Street
Holliston, MA 01746

Presented by:



September 3, 2021

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APPENDIX A: SITE PLANS SHOWING EROSION & SEDIMENT CONTROL MEASURES

APPENDIX B: INSPECTION AND MAINTENANCE LOG



#### 1.0 SITE EVALUATION, ASSESSMENT, AND PLANNING

The Construction Erosion, Sediment & Pollution Prevention Plan complies with the requirements of Standard 8 of the 2008 Massachusetts Department of Environmental Protection (DEP) Stormwater Handbook. The Plan outlines source control and pollution prevention measures, and maintenance requirements for the erosion and sediment control measures associated with the construction of the proposed project.

The proposed project will disturb just under 1 acre.

#### 1.1 Responsible Party

Name of persons or entity responsible for Plan compliance, to be determined prior to construction.

Company:			
Name:			
Address:			
City:	State:	ZIP Code:	
Telephone:	Fax:		

#### 1.2 Construction Sequencing Plan

#### Before any site grading activities begin:

- 1. Stake Limit of Construction. Workers shall be informed that no construction activity is to occur beyond this limit at any time.
- 2. Clear trees and vegetation as necessary as shown on the site preparation plan.
- 3. Grub the areas where trees are being removed, including stumps and roots as necessary. The existing ground surface shall be disturbed as little as possible prior to the start of construction.
- 4. Install erosion control barriers as shown on the plans. An adequate stockpile of erosion control materials shall be on site at all times for emergency or routine replacement and shall include materials to repair erosion controls, or any other devices planned for use during construction.
- 5. Construct staging and materials storage area.
- 6. Install temporary sanitary facilities and dumpsters.
- 7. Install temporary inlet protection in catch basins adjacent to site.



#### Site grading:

- 1. Establish soil stockpiles as necessary.
- Install silt fences around stockpile and cover stockpiles. Disturbed areas where construction will cease for more than 14 days shall be stabilized with erosion controls.
- 3. Prepare subgrades for various improvements and install materials per specifications.
- 4. Monitor perimeter controls for sediment accumulation.
- 5. Sweep site entrance as needed to prevent sediment tracking onto street.

#### Improvements:

1. Install proposed park improvement features.

#### Final stabilization and landscaping:

- 1. Remove all temporary control BMPs and stabilize any areas disturbed by their removal with erosion controls.
- 2. Prepare final seeding and landscaping.
- 3. Monitor stabilized areas until final stabilization is reached.

#### 1.3 Erosion and Sediment Control BMPs

This plan contains a listing of the erosion and sediment control best management practices (BMPs) that will be implemented to control pollutants in stormwater discharges. The BMPs are categorized under one of the areas of BMP activity as described below:

- **1.3.1** Minimize disturbed area and protect natural features and soil.
- **1.3.2** Control stormwater flowing onto and through the project.
- **1.3.3** Stabilize soils.
- **1.3.4** Establish perimeter controls and sediment barriers.



#### 1.4 Minimize Disturbed Area and Protect Natural Features and Soil

#### 1.4.1 Preserve Existing Vegetation

Description:	Preserved areas of existing vegetation as identified on the Site
	Plans and Sitework Specifications.
Installation	Install temporary tree protection as identified on the Site Plans.
Schedule:	
Maintenance	The area shall be inspected weekly to ensure the protective
and	measures are intact and functioning as intended. During
Inspection:	construction, preserved areas of existing vegetation shall be
	clearly marked at all times.

#### 1.4.2 Stockpiling Soil Materials

Description:	Soil stockpiles shall be contained within the proposed limit of		
	work.		
Installation	Soil stockpiles shall be established during grading activities. A silt		
Schedule:	fence and temporary erosion controls shall be installed		
	immediately after the stockpile has been established.		
Maintenance	Stockpile areas shall be inspected weekly for erosion and		
and	immediately after storm events. Areas on or around the stockpile		
Inspection:	that have eroded shall be stabilized immediately with erosion		
	controls.		

#### 1.5 Control Stormwater Flowing Onto and Through the Project

The Contractor shall be responsible for controlling the flow of stormwater through the project site throughout the construction period. This may be accomplished through the implementation of temporary sediment basins, berms, swales, etc. These stormwater control measures shall be monitored after significant rainfall events and corrective actions taken if needed to ensure that sediment is being appropriately contained within the limits of work without detrimental impacts to adjacent resource areas or properties.



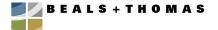
#### 1.6 Stabilize Soil

#### 1.6.1 Temporary Stabilization

Description:	Temporary vegetative cover shall be established using hydroseeding for areas of exposed soil (including stockpiles) where construction will cease for more that 14 days and seeded surfaces.
Installation Schedule:	Temporary stabilization measures shall be applied to portions of the site where construction activities will temporarily cease for more than 14 days.
Maintenance and Inspection:	Stabilized areas shall be inspected weekly and after storm events until a dense cover of vegetation has become established. If failure is noticed at the seeded area, the area shall be reseeded, fertilized, and mulched immediately.

#### 1.6.2 Permanent Stabilization

Description:	Permanent stabilization shall be done immediately after the final design grades are achieved but no later than 14 days after construction ceases. Native species of plants shall be used to establish vegetative cover on exposed soils.
Installation Schedule:	Portions of the site where construction activities have permanently ceased shall be stabilized, as soon as possible but no later than 14 days after construction ceases.
Maintenance and Inspection:	All seeded areas shall be inspected weekly during construction activities and after storm events until a dense cover of vegetation has been established. If failure is noticed at the seeded area, the area shall be reseeded, fertilized, and mulched immediately. After construction is completed at the site, permanently stabilized areas shall be monitored until final stabilization is reached.



#### 1.7 Establish Perimeter Controls and Sediment Barriers

#### 1.7.1 Erosion Control Barrier

Permanent	
Description:	An erosion control barrier, consisting of a silt fence and
	straw bales shall be installed along the down gradient
	side of the proposed project to decrease the velocity of
	sheet flows and intercept and detain small amounts of
	sediment from disturbed areas.
Installation Schedule:	Erosion Control Barrier shall be installed prior to clearing
	and grubbing.
Maintenance and	Erosion Control Barrier shall be inspected weekly,
Inspection:	following storms, and daily during rainy periods.
	Damaged ECB shall be replaced. Concentrated flows
	shall be intercepted and rerouted. Sediment
	accumulations shall be removed when reaching a depth
	of 6-inches. Deteriorated fencing material shall be
	replaced. Used fencing shall be properly disposed of.

#### 2.0 INSPECTION SCHEDULE AND PROCEDURES

Inspections of the site will be performed once every 7 days and within 24 hours of the end of a storm event of one-half inch or greater unless otherwise specified. The inspections will verify that all BMPs are implemented, maintained and effectively minimizing erosion and preventing stormwater contamination from construction materials.

Inspections shall include all areas of the site disturbed by construction activity and areas used for storage of materials that are exposed to precipitation. Inspectors shall look for evidence of, or the potential for, pollutants entering the storm water conveyance system. Sedimentation and erosion control measures identified in the plan shall be observed to ensure proper operation. Discharge locations shall be inspected to ascertain whether erosion control measures are effective in preventing significant impacts to resource areas, where accessible. Where discharge locations are inaccessible, nearby downstream locations shall be inspected to the extent that such inspections are practicable. Locations where vehicles enter or exit the site shall be inspected for evidence of off-site sediment tracking.



If corrective actions are identified during the inspection, he/she shall notify and submit a copy of the inspection report to the project managers. For corrective actions identified, the project managers shall be responsible for initiating the corrective action within 24 hours of the report and completing maintenance as soon as possible or before the next storm event.

#### 3.0 FINAL STABILIZATION

Stabilization measures shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than 14 days after the construction activity in that portion of the site has temporarily or permanently ceased. The following sections detail the management practices proposed to achieve final stabilization of the site.

#### 3.1 Permanent Seeding

Description:	Permanent seeding shall be applied immediately after the final design grades are achieved on portions of the site but no later than 14 days after construction activities have permanently ceased. After the entire site is stabilized, any sediment that has accumulated shall be removed and hauled off-site for disposal at an approved landfill. Construction debris, trash and temporary BMPs (including silt fences, material storage areas, sanitary toilets, and inlet protection) shall also be removed and any areas disturbed during removal shall be seeded immediately.
Installation	Seeding shall occur at portions of the site where construction activities
Schedule:	have permanently ceased shall be stabilized, as soon as possible but no later than 14 days after construction ceases.
Maintenance and Inspection:	All seeded areas shall be inspected weekly during construction activities for failure and after storm events until a dense cover of vegetation has been established. If failure is noticed at the seeded area, the area shall be reseeded, fertilized, and mulched immediately. After construction is completed at the site, permanently stabilized areas shall be monitored until final stabilization is reached.



### Appendix A

Site Plans Showing Erosion and Sediment Control Measures (Site Plans Attached Separately)



#### **Appendix B**

Inspection and Maintenance Log

Inspections under this Plan shall be conducted in accordance with each installed BMP's recommended maintenance requirements. This inspection frequency may be reduced to at least once every month if: a) the entire site is temporarily stabilized, b) runoff is unlikely due to winter conditions (e.g. site is covered with snow, ice, or the ground is frozen), or c) construction is occurring during seasonal arid periods in arid areas and semi-arid areas. If an inspection report is filed according to this modified schedule it shall be noted at the end of the report under the "NOTES" section.

The following four pages should be copied and completed for each inspection. All inspection forms should be compiled in a binder to prove compliance with this Plan.



# Construction Erosion, Sediment and Pollution Prevention Plan: Inspection Checklist

General Information				
Project Name				
Location				
Document				
<b>Date of Inspection</b>	Start/End			
Inspector's Name(s)	Time			
•				
Inspector's Title(s)				
Inspector's Contact				
Information				
Inspector's Qualifications				
Describe present phase				
of construction				
Type of Inspection: ☐ Regular ☐ Pre-store	rm event			
Weather Information				
Has there been a storm event since the last inspection? □Yes □No				
If yes, provide: Storm Start Date & Time:	Storm Duration (hrs):			
Storm State Date & Time.				
Approx. Amount of Precipitation (in):				
Weather at time of this inspection?				
☐ Clear ☐ Cloudy ☐ ☐ Other:	Rain  Sleet  Soo  Snowing  High Winds Temperature:			
a other.	Temperature.			
Have any discharges occurred since the last inspection? □Yes □No				
If yes, describe:				
Are there any discharges at the time of inspection? □Yes □No				
If yes, describe:				

#### **Site-specific BMPs**

• Number the structural and non-structural BMPs identified on your site plan and list them below (add as many BMPs as necessary). Carry a copy of the numbered site map with you during your inspections. This list will ensure that you are inspecting all required BMPs at your site.

ВМР	BMP Installed?	BMP Maintenance Required?	Corrective Action Needed and Notes
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	

#### **Overall Site Issues**

Below are some general site issues that should be assessed during inspections. Customize this list as needed for conditions at your site.

BMP/activity	Implemented?	Maintenance Required?	Corrective Action Needed and Notes
Are all slopes and disturbed areas not actively being worked properly stabilized?	□Yes □No	□Yes □No	
Are natural resource areas (e.g., streams, wetlands, mature trees, etc.) protected with barriers or similar BMPs?	□Yes □No	□Yes □No	
Are perimeter controls and sediment barriers adequately installed (keyed into substrate) and maintained?	□Yes □No	□Yes □No	
Are discharge points and receiving waters free of any sediment deposits?	□Yes □No	□Yes □No	
Are storm drain inlets properly protected?	□Yes □No	□Yes □No	
Is the construction exit preventing sediment from being tracked into the street?	□Yes □No	□Yes □No	
Is trash/litter from work areas collected and placed in covered dumpsters?	□Yes □No	□Yes □No	
Are washout facilities (e.g., paint, stucco, concrete) available, clearly marked, and maintained?	□Yes □No	□Yes □No	
Are vehicle and equipment fueling, cleaning, and maintenance areas free of spills, leaks, or any other deleterious material?	□Yes □No	□Yes □No	

BMP/activity	Implemented?	Maintenance Required?	Corrective Action Needed and Notes		
Are materials that are potential stormwater contaminants stored inside or under cover?	□Yes □No	□Yes □No			
Are non-stormwater discharges (e.g., wash water, dewatering) properly controlled?	□Yes □No	□Yes □No			
(Other)	□Yes □No	□Yes □No			
Non-Compliance					
	CEDTIEICAT	PION STATEM	ENT		
"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."					
Print name and title:					
Signature:					
Date:	_				