



October 20, 2022

Mr. Ryan Clapp, Conservation Agent
Holliston Conservation Commission
703 Washington Street
Holliston, MA 01746

Re: **Notice of Intent – AnyFence Co., 1485 Washington Street, Holliston MA,
DEP File#: CE 185-0931**

Dear Ryan:

CDW has reviewed the comments by McClure Engineering, Inc. (McClure), dated September 27, 2022, regarding the Notice of Intent Stormwater Management Design for AnyFence Co. The Site Plan Set have been revised to address the comments from the McClure letter and have a revision date of October 18, 2022, and offer the following responses to the comments. The McClure comments are in bold. Additionally, the Conservation Commission requested additional plan revisions listed at the end of the comments.

McClure Comments

Initial Review Comments:

1. **In review of the site, it appears there was a gravel expansion in and adjacent to the Washington Street right-of-way on the southern side of the southern driveway entrance within the 100' buffer zone and potentially 50' buffer zone after the existing conditions survey was performed. This should be added to the plans and should likely be restored.**
The plans have been revised to show the removal and restoration of the gravel expansion on the southern side of the southern driveway.
2. **It appears there are no soil testing logs included with the plans or stormwater report. Soil testing should be conducted to ensure proper design of infiltration systems.**
A soil test pit was conducted on 10/12/2022. The test pit was excavated approximately 8.7 feet, to elevation 253.8, with no sign of groundwater or mottling. The bottom of the stone for the infiltration system is at 257.29, approximately 3.4 feet above the bottom of the test pit. A soil log is now included with the Stormwater Narrative. The soil type has been confirmed as a type A loamy sand soil.
3. **A single catch basin and subsurface infiltration system is proposed for the site redevelopment which will capture a portion of the parking area. Although not required under the MA Stormwater Standards or Holliston Stormwater Management and Land Disturbance Regulations, it appears there may be opportunity for improvement of stormwater quality and possibly groundwater recharge for the rest of the site as well. See additional comments. McClure respectfully defers to the Commission.**
Comment noted.

MA Stormwater Standards Comments

1. **Standard 1 - Computations to Show That Discharge Does Not Cause Scour or Erosion. No new discharges are proposed therefore Standard 1 appears to be met.**



Comment noted.

2. **Standard 2 - Peak Rate Attenuation.** As a redevelopment project this standard needs to be met to the maximum extent practicable. Through review of the stormwater report and HydroCAD calculations, Standard 2 appears to be met.

Comment noted.

3. **Standard 3 - Ground Water Recharge Volume.** As a redevelopment project this standard needs to be met to the maximum extent practicable. No soil testing was conducted to determine estimated seasonal high groundwater, soil classification, or hydraulic conductivity. Soil mapping indicates "Udorthents" soil types, which are typically associated with fill. If fill is found to be present, the system shall be designed per the MA Stormwater Standards Manual Volume 3: "When fill materials are present or are added prior to construction of the system, a soil textural analysis must be conducted in both the fill material and the underlying parent materials, and the Hydrologic Soil Group of the more restrictive layer shall be used to size the infiltration BMP. If fill is present that is composed of asphalt, brick, concrete, construction debris, or if materials classified as solid or hazardous waste are identified at the specific location where recharge is proposed, recharge elsewhere on site must be considered. Alternatively, the debris or waste may be removed in accordance with all applicable Solid and Hazardous Waste Regulations (see 310 CMR 19.000 and 40.0000) and replaced with clean material suitable for infiltration. Any solid or hazardous wastes present on the site must be managed in strict accordance with MassDEP Solid Waste Regulations, 310 CMR 19.000, Hazardous Waste Regulations, 310 CMR 30.00, and the Massachusetts Contingency Plan Regulations, 310 CMR 40.000." Soil testing should be performed to ensure proper design of the infiltration system including depth to groundwater, infiltration rate, and drawdown. The proposed system is sized for the required recharge volume for only the impervious area directed to the system and not for the site. Although the proposal meets the standard for redevelopment and it is not required under the MA Stormwater Standards or Holliston Regulations, there is opportunity to bring the site into further compliance for required recharge of all impervious areas. The Holliston Land Disturbance regulations emphasize maximizing infiltration and groundwater recharge. A second story is proposed on a portion of the building and it is assumed a new roof drainage system will be constructed as part of this addition. This "clean" roof runoff could potentially be used to provide additional groundwater recharge without requiring pretreatment. McClure respectfully defers to the Commission.

A soil test was conducted on October 12, 2022 and a soil log is now included in the Stormwater Narrative. The test pit was excavated to a depth of 8.7 feet below ground surface. There was no presence of fill materials, and the soil type has been confirmed as a loamy sand. No groundwater or mottling was observed. Additionally, as an additional check of the underground system, the exfiltration rate for the chambers was modeled as 0.17 in/hr, equivalent to a Hydrological Group C, Sandy Clay Loam. Using the significantly lower infiltration rate, while there is a slight increase in the post development flow, it is still below the pre-existing runoff rates. The calculation using the lower infiltration rate for a 100-yr storm is included at the end of this letter, shows that the functioning of system is largely unaffected by the infiltration rate.

4. **Standard 4 - Water Quality Volume.** As a redevelopment project this standard needs to be met to the maximum extent practicable. The proposed treatment system involved a deep sump and hooded catch basin, along with the subsurface infiltration system with an "isolator" row which will treat the water quality volume. The report indicates the system will discharge (infiltrate) within soils with a rapid infiltration rate and therefore requires a 1" water quality volume depth as well as 44% TSS removal prior to infiltration. Once again soil testing should be conducted to ensure proper design of the infiltration system. Per the report, the required water quality volume for the impervious area which is

directed to the infiltration system is 774 cubic feet (629 cubic feet per the Holliston regulations for redevelopment). The entire system as modeled provides for 776 cubic feet of volume, which is greater than the required water quality volume, however as the system is required to have 44% TSS removal pretreatment, the isolator row itself should be sized to handle the water quality volume of 774 cubic feet before bypass to the rest of the system, and therefore it does not appear that Standard 4 is met.

McClure recommends separating the catch basin, manhole, and piping from the infiltration system in the HydroCAD model, as well as modeling the isolator row separately from the remainder of the infiltration system to ensure proper water quality volume treatment within the isolator row.

Alternatively the model could be modified to separate the isolator row to show that the water quality volume is directed to and passed through the isolator row (infiltrated) during the water quality storm event (24-hr rainfall which produces 1" of runoff over the impervious area directed to the system). Separately a water quality unit could be proposed prior to the system which is sized for the water quality volume or equivalent water quality flow rate. As with Standard 3, as the site is a redevelopment, providing water quality treatment for the site as a whole is not required under the MA Stormwater Standards. In review of the Holliston Stormwater Regulations, it is not clear if the calculations must take into account the entire impervious area on site, and if so, the system would need to be revised and expanded, and additional impervious area would likely need to be directed to water quality treatment device to ensure adequate treatment volume is directed to it. There is also likely additional opportunity to provide improvement of water quality to the maximum extent practicable from other impervious areas on site (gravel and pavement) which are currently directed to wetland resources without treatment by utilizing BMPs such as a pea stone diaphragm or filter berm at the edge of impervious areas, or other LID measures, especially in areas which are proposed to be repaved within the Town's 50' No Disturbance Buffer Zone. McClure respectfully defers to the Commission.

To simplify the water quality design, the isolator row has been removed from the underground infiltration system and a water quality structure (Stormceptor STC 450i) with a grate has been provided, replacing deep sump catchbasin. The 450i will capture and treat the surface runoff prior to entering the infiltration system. The water quality structure was sized based on an equivalent water quality flow rate (calculations are shown on Stormwater Management Report; Management Standards; Standard #4: Required Water Quality Volume). A Cape Cod Berm has also been added to the plans along the left side of the proposed pavement in the front. The surface runoff of the proposed repaved area will then flow to the Stormceptor STC 450i, decreasing the amount of impervious area flowing directly to the wetlands.

5. **Standard 5 - Land Uses with Higher Potential Pollutant Loads.** As a redevelopment project this standard needs to be met to the maximum extent practicable. Site is not a LUHPPL and Standard 5 appears to be met. Comment noted.
6. **Standard 6 - Critical Areas.** As a redevelopment project this standard needs to be met to the maximum extent practicable. The stormwater report indicates that the site is not within a Zone II wellhead protection area which is correct. The wetland system to the west is identified as a potential vernal pool by the NHESP. Although only certified vernal pools at this time are considered critical areas, this area should be identified in the report as being a potential critical area. As a redevelopment project, and a project with a water quality volume depth of 1" and 44% required pretreatment prior to infiltration due to soils with a rapid infiltration rate, Standard 6 will be met. See Standard 4. Standard 6 on the Stormwater Management Report has been revised to identify the area as a potential critical area. The water quality structure has been sized based on an equivalent water quality flow and offers a pretreatment of at least 44% prior to infiltrating to soils with rapid infiltration rate.



7. **Standard 7 — Redevelopment.** As a redevelopment project certain standard only need to be met to the maximum extent practicable other than Standards 1, 8, 9, 10 which are required to be and appear to be fully met.
Comment noted.
8. **Standard 8 - Construction Period Controls.** The stormwater report includes a Long Term Pollution Prevention Plan. Site plans include construction period erosion controls, notes, and details. Standard 8 appears to be met.
Comment noted.
9. **Standard 9 - Operation and Maintenance Plan.** The stormwater report includes an Operation and Maintenance Plan. Standard 9 appears to be met. Some changes to the O&M are required per the Holliston stormwater regulations.
Comment noted, see comments under Holliston regulations for the changes to the O&M Plan.
10. **Standard 10 — Illicit Discharges to Drainage System.** The stormwater report includes a signed illicit discharge compliance statement. Standard 10 appears to be met.
Comment noted.

Holliston Regulations for Stormwater Management and Land Disturbance Comments

§11.4.3.a — Addition of a locus map

A locus map is present on the existing conditions plan by Alpha Survey Group, LLC entitled Existing Conditions & Boundary Survey 1485 Washington Street, Holliston, MA 01746. We have also added a locus map to sheets C-1.0 (Demolition and Erosion Control Plan), C-2.0 (Layout and Materials Plan), and C-3.0 (Grading and Drainage Plan).

§11.4.3.h — Identification of who delineated on-site wetlands.

Identification of who delineated on-site wetlands is shown on Note # 4 of the Existing Conditions & Boundary Survey Plan. Note #3 was added under Existing Conditions/Survey Notes to sheets C-1.0 (Demolition and Erosion Control Plan), C-2.0 (Layout and Materials Plan), and C-3.0 (Grading and Drainage Plan) to identify that the wetland delineation was performed by Applied Ecological Sciences on April 26, 2022

§11.4.3.j — Estimated seasonal high groundwater in area of proposed infiltration which should be determined by on-site soil testing.

On-site soil testing was performed on 10/12/2022. The pit was excavated to a depth of 8.7 feet and no groundwater or mottling was observed. Soils logs are now included in the Stormwater Report.

§11.4.3.p — Description on fueling of vehicles.

Vehicles will not be fueled on site.

§11.10.1.7 — Indicate minimum 6" loam depth on areas to be seeded.

A 6" min. depth has been specified on all callouts for loam in sheet C-2.0 Layout and Materials Plan.

§11.10.2.2.f. — Note 5 under Stabilization Practices on sheet C-4.0 should be revised or removed.

Note 5 under Stabilization Practices has been removed.

§11.10.2.3. — The plan should be revised to indicate re-vegetation is to take place no more than 7 days after final grading.



Indication that re-vegetation is to take place no more than 7 days after final grading has been added as Note #18 to C-2.0 Layout and Materials plan.

§11.5.2.2.e — The signature of the owner.

The Appendix B - O&M have been revised to include the owners' signature.

§11.5.2.2.f — The estimated operation and maintenance budget.

An estimated operation and maintenance budget has been included on the Appendix B - O&M Plan.

§11.5.2.2.g.1 — Indicate logs must be kept for 3 years

A statement indicating that logs must be kept for 3 years has been added under section Documentation on the Appendix B - O&M Plan.

§11.5.2.2.g.2 — Indicate the logs are to be made available to the town.

A statement indicating that logs are to be made available to the Town of Holliston upon request had been added under section Documentation on the Appendix B - O&M Plan.

§11.5.2.2.g.3 — Indicate the town is allowed to inspect BMPs.

A statement indicating that the Town of Holliston is allowed to inspect the BMPs has been added to the section Documentation on the Appendix B - O&M Plan.

§11.5.3 — Future changes to the O&M.

A section called Changes to O&M has been added to Appendix B – O&M Plan with instructions on how the owner will proceed in case of changes to the ownership or assignment of financial responsibility, as well as amendments to the O&M.

§11.5.4 — Annual certification to the Conservation Commission and Planning Board.

A section called Annual Certification has been added to Appendix B – O&M Plan, showing that the owner will submit a signed annual certification to both the Conservation Commission and Planning Board documenting the work that has been done over the past 12 months.

The applicant should comment on whether LID site planning and design was considered per §11.10.1 and 2. McClure respectfully defers to the Commission.

As an existing developed site, along with its configuration, there is little to no room to provide a low-impact-design, at least not without encroaching even closer to the wetlands than the existing site is already. Portions of the existing pavement at the front of the site are roughly 20-ft off the wetland edge. An underground infiltration system beneath the paved areas, will reduce the impacts to the adjacent wetland and buffer zone.

Plan Review and General Engineering Comments

1. Information on who conducted the wetland delineation and when the wetland delineation was performed should be added to the existing conditions plan.

Note # 4 of the Existing Conditions & Boundary Survey Plan identifies that the wetlands were delineated by Applied Ecological Sciences on April 26, 2022, and field located by Alpha Survey Group on April 28, 2022. Note #3 was added under Existing Conditions/Survey Notes to sheets C-1.0 (Demolition and Erosion Control Plan), C-2.0 (Layout and Materials Plan), and C-3.0 (Grading and Drainage Plan).

2. Existing and proposed clearing limits should be added to the plans.

There is no tree clearing being proposed, all work will be within the existing limits of clearing. If the new outlet structure by the left driveway appears as if it might impact an existing tree, the outlet can be moved towards the existing pavement as necessary.

3. The pipes in the HydroCAD model do not match those on the plan set. McClure recommends separating the catch basin, manhole, and pipes from the infiltration basin storage in the model. The exfiltration discharge is applied to the wetted area so these items are technically being modeled as providing infiltration as well as the chambers.

HydroCAD model has been revised to match the pipes on the plan set. HydroCAD offers an option, via a check box, to allow exfiltration for each volume modeled as storage. The exfiltration box is unchecked for the catch basin, manhole, and pipes modeled as storage. Exfiltration only occurs through the underground infiltration chambers.

4. The model indicates that even during a 2-year storm event, the system will overtop the proposed catch basin inlet grate. This would allow for trapped oil/grease/hydrocarbons which are afloat and trapped within the catch basin to leave the catch basin and be directed into the wetlands. McClure recommends either an overflow discharge be proposed from the infiltration system to prevent overtopping of the catch basin grate or resizing of the system so as to eliminate overtopping of the catch basin grate.

The proposed stormwater system has been revised to include a separate outlet structure within the grassed area next to CB-1 (STC-450i), avoiding overtopping of CB-1 itself.

5. The infiltration system should include a monitoring well per the MassDEP Stormwater Handbook.

A monitoring well has been added within the footprint of the underground infiltration system, shown on Grading and Drainage Plan sheet C-3.0, and Construction Details C-4.2

6. CULTEC recommends a 24" minimum sump within the manhole directly upstream of the system for maintenance purposes (sediment accumulation during jetting of isolator row). No sump is currently proposed.

A 24" minimum sump has been added to the manhole detail, as well as a note to the manhole itself on the Grading and Drainage Plan sheet C-3.0.

7. CULTEC recommends the maximum diameter inlet pipe to the isolator row possible or the unit type. R-280HD chambers can accept an 18" pipe. A 12" is proposed.

The isolator row has been removed, therefore the 12" diameter pipe will remain.

8. CULTEC recommends at least (1) inspection port in the separator row and (1) inspection port in overflow chamber section for inspection and maintenance. A detail should be provided.

Detail #8 Cultec R- 280HD Inspection Port has been added to sheet C-4.1 at the center of the system. Only one inspection port has been provided as there is no longer an isolator row.

9. The CULTEC cross-section detail on sheet C-4.1 should be revised to eliminate the feed connector from the isolator row to the overflow chambers. The isolator row should be identified as well.

The isolator row has been removed from the system. The detail containing the isolator row has been removed from the details sheet C-4.1.

10. The O&M Plan should be revised to include CULTEC specific inspection and maintenance recommendations for the subsurface infiltration system.



The Operation and Maintenance Guidelines for CULTEC Stormwater Systems has been included at the end of the O&M Plan.

11. Areas in which unpermitted gravel expansion has taken place and where such gravel will be removed and restored to vegetated areas should be tilled and the soil should be amended as necessary as to restore the infiltrative capacity of the area which has likely been reduced or eliminated through compaction.

Note #19 under Layout and Materials Notes has been added to C-2.0 Layout and Materials Plan. The callouts of the two areas of gravel expansion also have been modified to show that the area should be tilled to restore infiltrative capacity.

Conservation Commission Comments

Specify a native seed mix to the area of loam and seed.

The callout on the plan was revised to include "New England Native Grass Seed Mix".

Removal of 3 parking spaces within the 30' Wetland Buffer on the front left side of the site.

Spaces have been moved to be outside the 30' Wetland Buffer on the plans.

A construction entrance is not shown on the plans.

A construction entrance has been added to the right side driveway.

Add curbing along left side of the pavement, behind the dumpsters.

Cape Cod Berm has been added along the left side of the pavement from the building all the way into the site entrance.

Please contact us with any questions or concerns.

Very truly yours,

CDW CONSULTANTS, INC.

A handwritten signature in blue ink, appearing to read "Eric Wilhelmsen", is written over a light blue horizontal line.

Eric Wilhelmsen, P.E.
Associate Principal

Pre&Post flow R20.17infil rate check

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Type III 24-hr 100-YR Rainfall=8.27"

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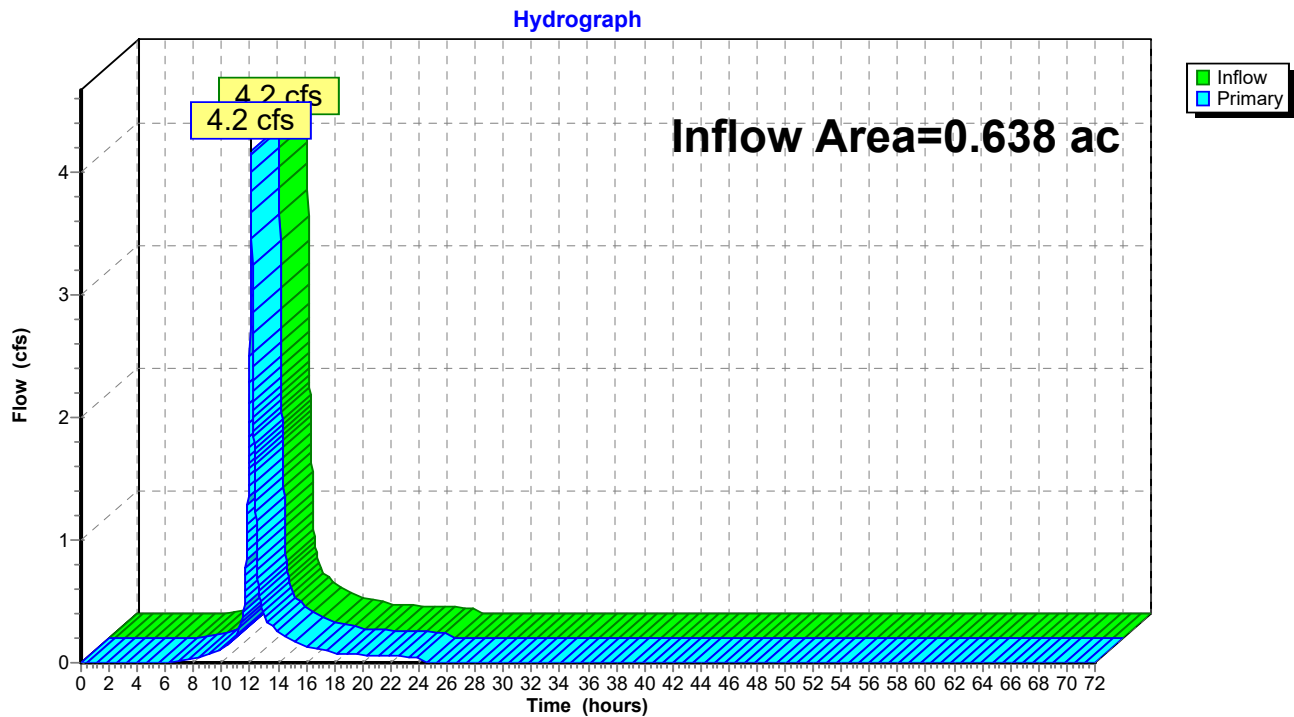
Page 3

Summary for Pond DP-2e: (new Pond)

Inflow Area = 0.638 ac, 43.96% Impervious, Inflow Depth = 5.64" for 100-YR event
Inflow = 4.2 cfs @ 12.09 hrs, Volume= 0.300 af
Primary = 4.2 cfs @ 12.09 hrs, Volume= 0.300 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Pond DP-2e: (new Pond)



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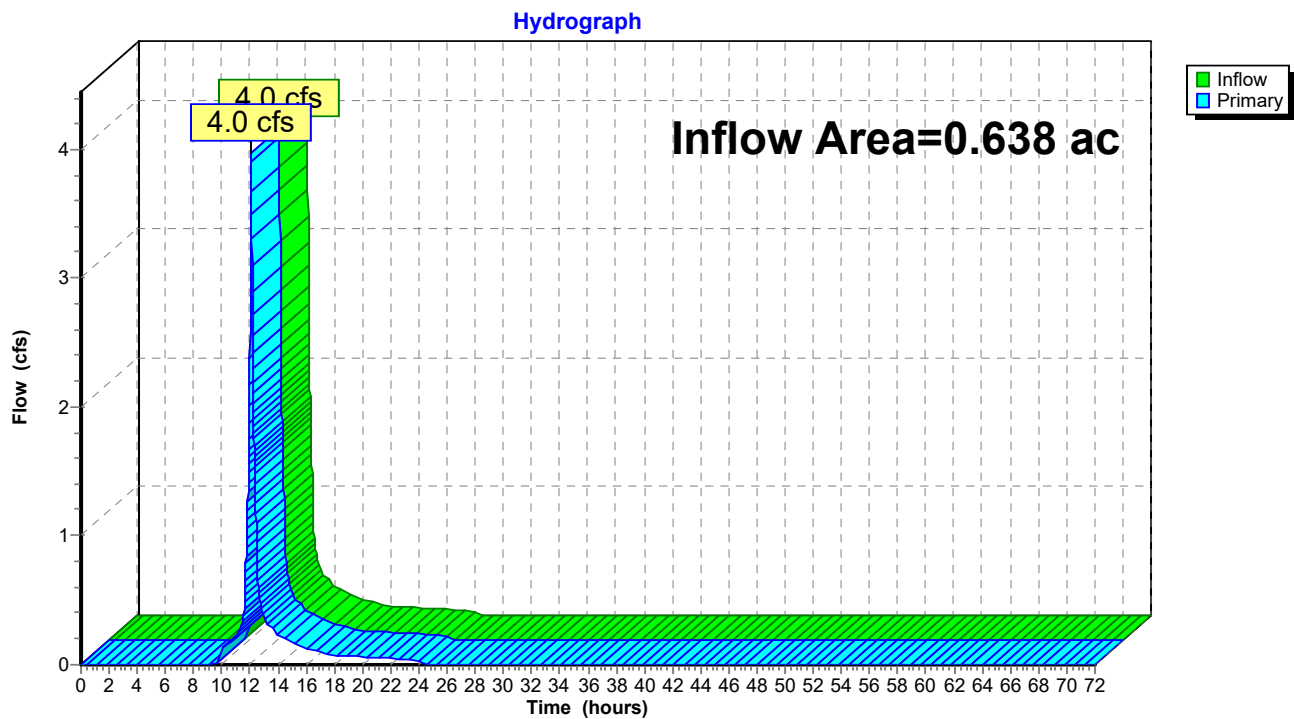
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Summary for Pond DP-2p: (new Pond)

Inflow Area = 0.638 ac, 50.29% Impervious, Inflow Depth = 5.18" for 100-YR event
Inflow = 4.0 cfs @ 12.09 hrs, Volume= 0.276 af
Primary = 4.0 cfs @ 12.09 hrs, Volume= 0.276 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Pond DP-2p: (new Pond)



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Summary for Pond UGI: Underground Infiltration System

Inflow Area = 0.305 ac, 73.71% Impervious, Inflow Depth = 7.43" for 100-YR event
 Inflow = 2.4 cfs @ 12.08 hrs, Volume= 0.189 af
 Outflow = 2.4 cfs @ 12.08 hrs, Volume= 0.178 af, Atten= 0%, Lag= 0.0 min
 Discarded = 0.0 cfs @ 12.08 hrs, Volume= 0.013 af
 Primary = 2.4 cfs @ 12.08 hrs, Volume= 0.165 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 9

Peak Elev= 261.30' @ 12.08 hrs Surf.Area= 363 sf Storage= 886 cf

Plug-Flow detention time= 170.9 min calculated for 0.178 af (94% of inflow)

Center-of-Mass det. time= 138.3 min (902.8 - 764.5)

Volume	Invert	Avail.Storage	Storage Description
#1	259.60'	25 cf	4.00'D x 2.00'H CB1 -Impervious
#2	259.60'	8 cf	12.0" Round Pipe Storage -Impervious L= 10.0' S= 0.0100 ' /'
#3	257.79'	54 cf	4.00'D x 4.30'H DMH1 -Impervious
#4	259.40'	3 cf	12.0" Round Pipe Storage -Impervious L= 4.0'
#5A	257.29'	325 cf	11.33'W x 32.00'L x 3.21'H Field A 1,164 cf Overall - 352 cf Embedded = 811 cf x 40.0% Voids
#6A	257.79'	352 cf	Cultec R-280HD x 8 Inside #5 Effective Size= 46.9"W x 26.0"H => 6.07 sf x 7.00'L = 42.5 cf Overall Size= 47.0"W x 26.5"H x 8.00'L with 1.00' Overlap Row Length Adjustment= +1.00' x 6.07 sf x 2 rows
#7	257.79'	4 cf	12.0" Round Pipe Storage -Impervious L= 5.0'
#8	257.79'	58 cf	4.00'D x 4.60'H DMH2 -Impervious
#9	257.22'	37 cf	12.0" Round Pipe Storage -Impervious L= 47.0' S= 0.0100 ' /'
#10	257.22'	48 cf	4.00'D x 3.78'H CB2 -Impervious
		913 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	257.29'	0.170 in/hr Exfiltration over Wetted area Conductivity to Groundwater Elevation = 1.00'
#2	Primary	261.60'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	261.10'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0 cfs @ 12.08 hrs HW=261.30' (Free Discharge)↑ **1=Exfiltration** (Controls 0.0 cfs)**Primary OutFlow** Max=2.4 cfs @ 12.08 hrs HW=261.30' (Free Discharge)↑ **2=Orifice/Grate** (Controls 0.0 cfs)↑ **3=Orifice/Grate** (Weir Controls 2.4 cfs @ 1.48 fps)

Pre&Post flow R20.17infil rate check

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Pond UGI: Underground Infiltration System - Chamber Wizard Field A

Chamber Model = Cultec R-280HD (Cultec Recharger® 280HD)

Effective Size= 46.9"W x 26.0"H => 6.07 sf x 7.00'L = 42.5 cf

Overall Size= 47.0"W x 26.5"H x 8.00'L with 1.00' Overlap

Row Length Adjustment= +1.00' x 6.07 sf x 2 rows

47.0" Wide + 6.0" Spacing = 53.0" C-C Row Spacing

4 Chambers/Row x 7.00' Long +1.00' Row Adjustment = 29.00' Row Length +18.0" End Stone x 2 = 32.00' Base Length

2 Rows x 47.0" Wide + 6.0" Spacing x 1 + 18.0" Side Stone x 2 = 11.33' Base Width

6.0" Base + 26.5" Chamber Height + 6.0" Cover = 3.21' Field Height

8 Chambers x 42.5 cf +1.00' Row Adjustment x 6.07 sf x 2 Rows = 352.2 cf Chamber Storage

1,163.6 cf Field - 352.2 cf Chambers = 811.4 cf Stone x 40.0% Voids = 324.6 cf Stone Storage

Chamber Storage + Stone Storage = 676.7 cf = 0.016 af

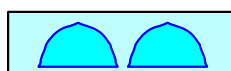
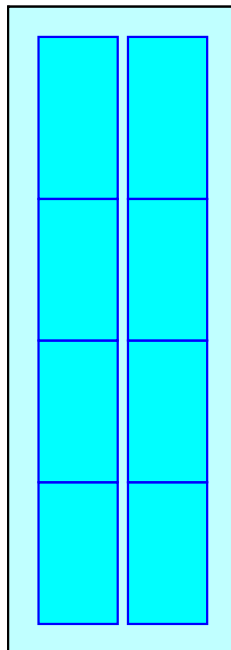
Overall Storage Efficiency = 58.2%

Overall System Size = 32.00' x 11.33' x 3.21'

8 Chambers

43.1 cy Field

30.1 cy Stone



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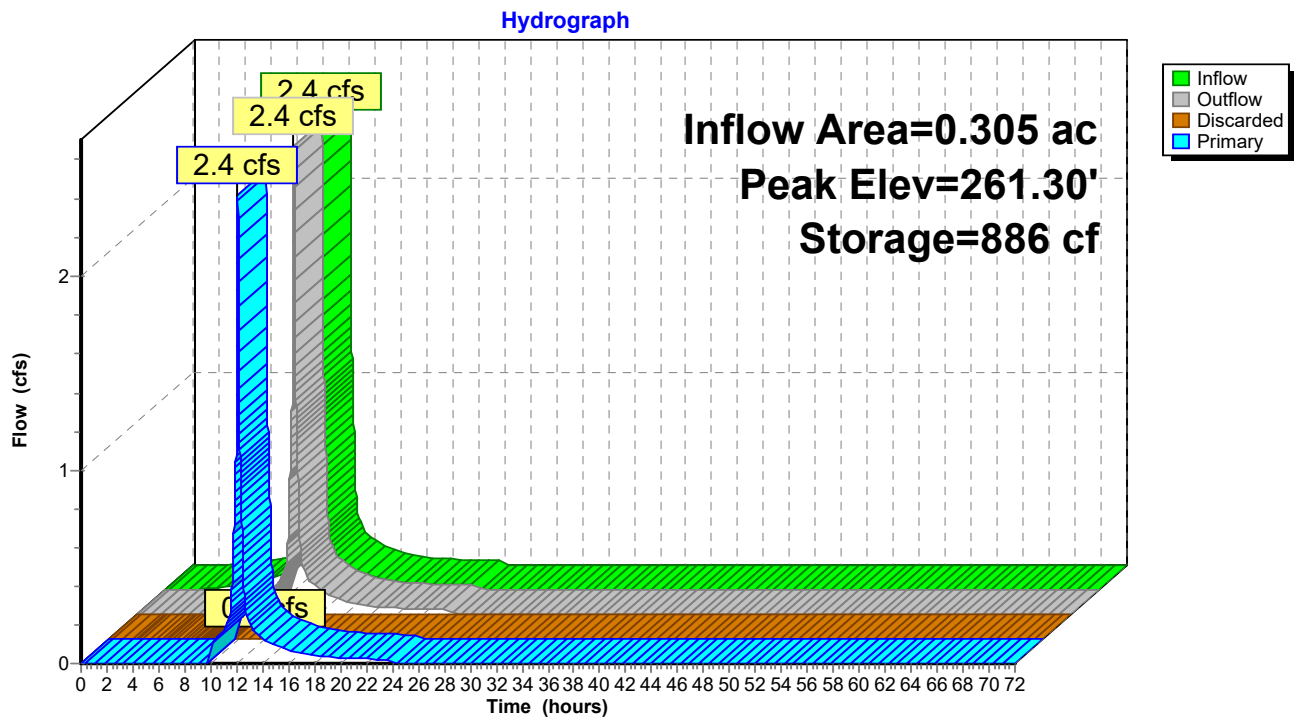
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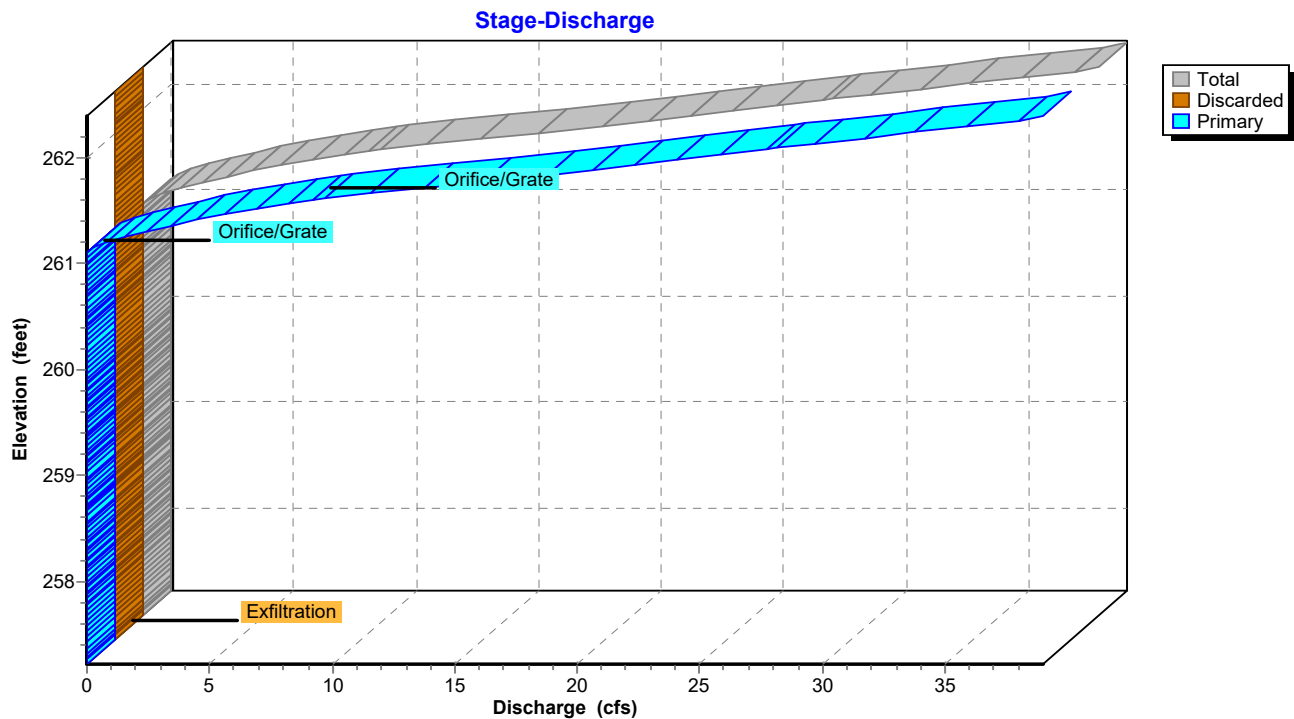
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Pond UGI: Underground Infiltration System



Pond UGI: Underground Infiltration System



Pre&Post flow R20.17infil rate check

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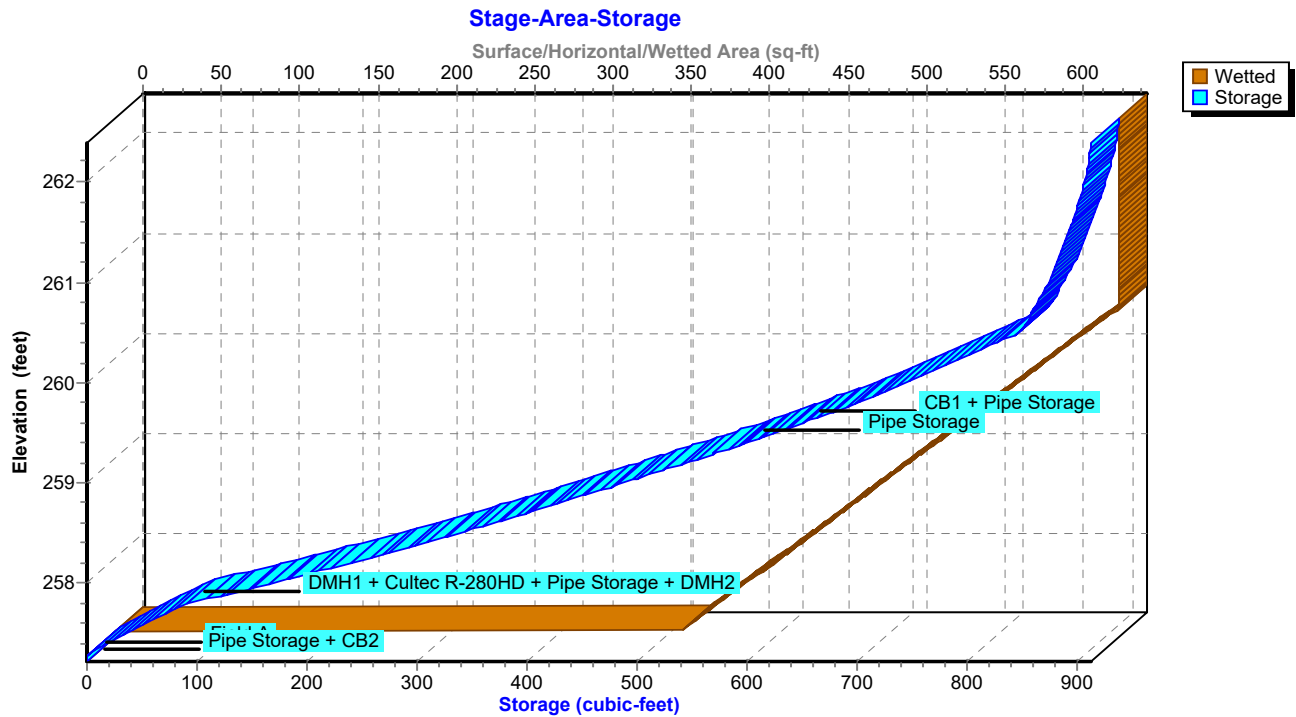
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Pond UGI: Underground Infiltration System



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Stage-Area-Storage for Pond UGI: Underground Infiltration System

Elevation (feet)	Wetted (sq-ft)	Storage (cubic-feet)
257.22	0	0
257.32	365	6
257.42	374	22
257.52	383	40
257.62	391	58
257.72	400	77
257.82	409	102
257.92	417	138
258.02	426	174
258.12	435	210
258.22	443	246
258.32	452	280
258.42	461	313
258.52	469	345
258.62	478	376
258.72	487	407
258.82	495	436
258.92	504	465
259.02	513	494
259.12	521	523
259.22	530	551
259.32	539	578
259.42	547	605
259.52	556	631
259.62	565	656
259.72	573	682
259.82	582	706
259.92	591	728
260.02	599	749
260.12	608	770
260.22	617	791
260.32	625	812
260.42	634	832
260.52	641	850
260.62	641	855
260.72	641	860
260.82	641	865
260.92	641	870
261.02	641	875
261.12	641	879
261.22	641	883
261.32	641	886
261.42	641	890
261.52	641	894
261.62	641	897
261.72	641	900
261.82	641	902
261.92	641	905
262.02	641	908
262.12	641	910
262.22	641	911
262.32	641	912