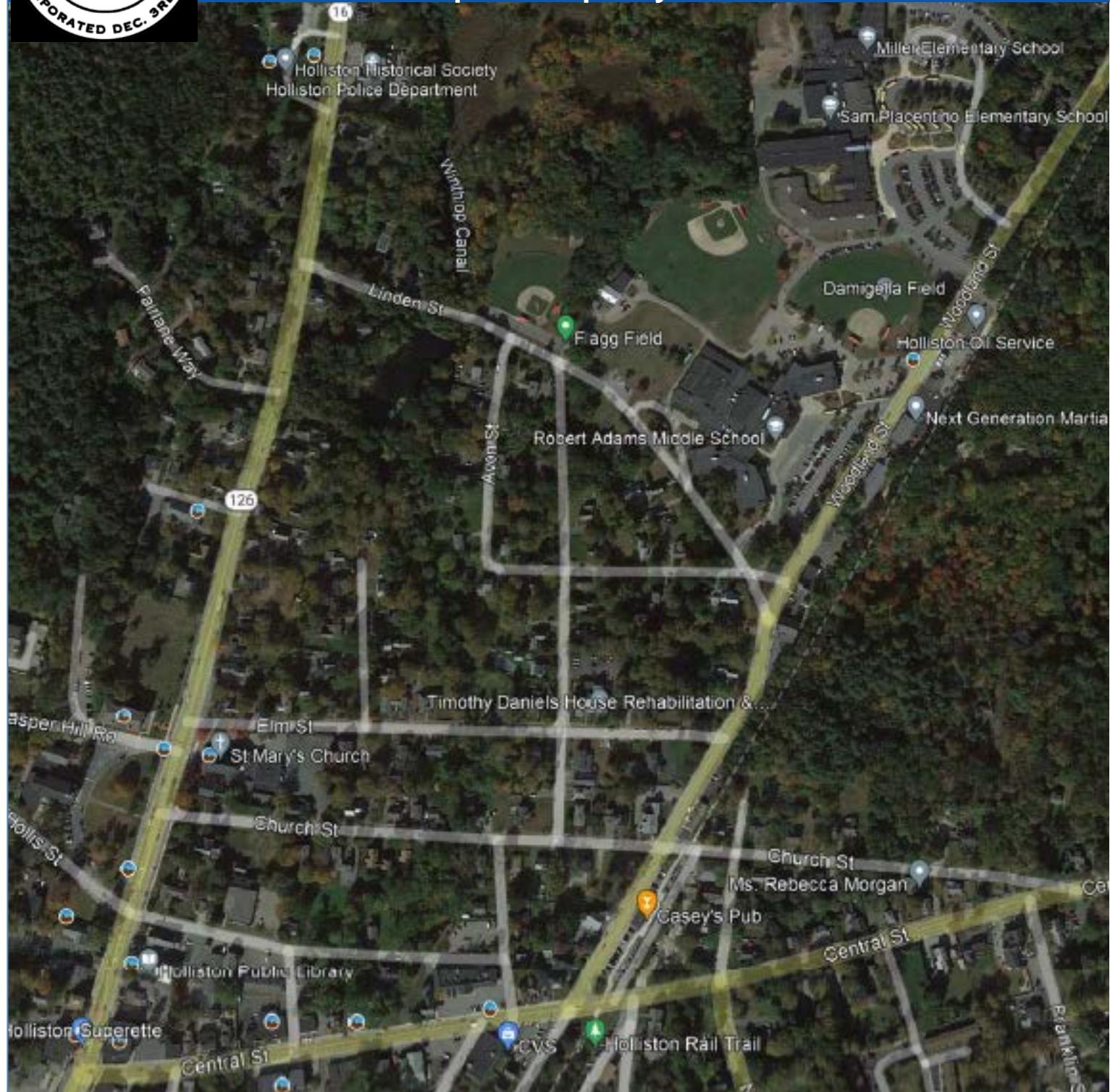


Holliston Woodland Street Schools Complex Existing Hydrogeologic Conditions Report & Disposal Capacity Estimates



Submitted to:
Town of Holliston
703 Washington Street
Holliston, MA 01746

Environmental Engineers/Consultants

LOMBARDO ASSOCIATES, INC.

188 Church Street, Newton, Massachusetts 02458

Updated as April 4, 2023 with Verdantas

TABLE OF CONTENTS

EXECUTIVE SUMMARY	4
1 INTRODUCTION.....	6
2 WATERSHED INFORMATION.....	9
3 PROJECT SITE BORINGS, SIEVE ANALYSIS, SLUG TESTS, HYDRAULIC CONDUCTIVITY ESTIMATES & GROUNDWATER MONITORING WELLS/ELEVATIONS.....	20
3.1 Site Borings.....	20
3.2 Hydraulic Conductivity Estimates Based upon Sieve and Slug Tests	21
4 GROUNDWATER ELEVATION MONITORING RESULTS.....	29
4.1 Groundwater Monitoring Wells, Elevations and Contours	29
4.2 Groundwater Contours.....	41
4.3 Groundwater Slope	42
4.4 Groundwater Velocity & Travel Time	42
5 SITE CAPACITY & MOUNDING ANALYSIS	43
5.1 Preliminary Site Capacity Analysis	43
5.2 Preliminary Mounding Analysis.....	45
APPENDIX A SOIL BORING LOGS – AUGUST & NOVEMBER 2022.....	46
APPENDIX B VERDANTAS SITE HYDROGEOLOGY REPORT WITH LOGS & SIEVE ANALYSIS.....	47

TABLE OF FIGURES

Figure ES-1 WWTP and Woodland School Complex – Aerial Photograph	5
Figure 1-1 WWTP and Woodland School Complex – Aerial Photograph.....	7
Figure 1-2 Available Areas for Subsurface Disposal.....	8
Figure 2-1 Project Site - Watershed Areas	9
Figure 2-2 USGS Observation wells & streamflow-measurement sites Near Project Area	10
Figure 2-2A USGS Observation well on Aerial Photo	11
Figure 2-3 USGS 2002 Estimated Thickness of the stratified glacial aquifers	12
Figure 2-4 Municipal Water Supply Wells in Project Area.....	12
Figure 2-5 Ground-water levels measured in selected observation wells 1999–2000	14
Figure 2-6 Schematic diagram of hydrogeologic units & flow components of water balance	14
Figure 2-7 Hydraulic conductivity zones for the USGS 2002 simulation models	15
Figure 2-8 Model-calculated steady-state water table	16
Figure 2-9 Zone II near School Site.....	17
Figure 2-10 Holliston Zone II Map on Aerial at Project Site	18
Figure 2-11 Wetlands Map	19
Figure 3-1 WWTP and Woodland School Complex – Photo with Recent Borings	20
Figure 3-2 1998 Survey.....	24
Figure 3-3 USGS Surficial Geology Map	26
Figure 3-4 Site Soils Map	27
Figure 4-1A Locations of Temporary & Permanent GW Monitoring Wells	29
Figure 4-1B Locations of Permanent GW Monitoring Wells.....	30
Figure 4-2 Soils Cross Sections - August 2022 Borings	31
Figure 4-3 Soil Boring Cross Section A – A'	35
Figure 4-4 Soil Boring Cross Section B – B'	36
Figure 4-5 Soil Boring Cross Section C – C'.....	37
Figure 4-6 Soil Boring Cross Section D – D'	38

Figure 4-7 Groundwater Elevations at Wells # 1, #2, #3 & #4	39
Figure 4-8 Depth to Groundwater at Wells # 1, #2, #3 & #4	40
Figure 4-9 Groundwater Contour Map.....	41
Figure 5-1 Estimated GW Elevations and GW Discharging Flux Lengths.....	44

TABLE OF TABLES

Table ES-1 Darcy's Law - Disposal Capacity Estimates.....	5
Table 2-1 Holliston Water Supply Wells	13
Table 3-1 Soil Boring Coordinates.....	21
Table 3-2 Hydraulic Conductivity Estimates Based Upon Sieve Analysis	21
Table 3-3 Hydraulic Conductivity Estimates Based on Sieves Utilizing Various Techniques – B-4 Site Only	22
Table 3-4 Slug Tests Hydraulic Conductivity Results	22
Table 3-6 Site Soils Units.....	28
Table 4-1 Groundwater Elevations – August 2022 Data.....	32
Table 4-2 Groundwater Elevations – Nov. 2022 – Feb. 2023 Data.....	33
Table 4-2 Groundwater Slope	42
Table 5-1 Disposal Capacity Initial Estimates.....	45

EXECUTIVE SUMMARY

This Report provide tables and figures of relevant project information with minimal text as the tables and figures are self-explanatory.

The determinants of site capacity for effluent disposal / reuse are described in Section 6 and summarized below.

1. Using Darcy's Law determine the volume of water that could be discharged from the site while maintaining subsurface flow to receiving surface water bodies. The Darcy's Law key parameters are:
 - a. Site hydraulic conductivity, K
 - b. Groundwater slope, i
 - c. Unsaturated depth through which the additional flow can discharge to a surface water body

Based upon data collected to date, preliminary estimate of the site wastewater design capacity is approximately 50,000 gpd.

2. Groundwater mounding analysis. The key site parameter is for mounding analysis is:
 - a. Site hydraulic conductivity, K
 - b. The sophisticated mounding analytical model MODFLOW needs to be used due to the limitations of the Hantush method. Site groundwater slopes, multiple groundwater directions and interaction amongst potential drainfields at the site requires the more sophisticated MODFLOW model versus Hantush method.

While slug tests results were lower than expected other techniques - and groundwater elevation monitoring is necessary to determine slope(s), hydraulic conductivity and unsaturated depth.



Figure ES-1 WWTP and Woodland School Complex – Aerial Photograph
Table ES-1 Darcy's Law - Disposal Capacity Estimates

Holliston Woodsand Street School Complex Site - Discharge Capacity Initial Estimates with Sensitivity Analysis of Values for K & Unsaturated Thickness Available for Discharge	
$Q = K * A * i$	Q = volumetric flow (cf/day), K = Hydraulic conductivity (ft/day) of unsaturated zone A = cross sectional area (sf) of discharge/flux area i = groundwater slope

K (ft/day)	5	10			
Area (sf)	17,500	17,500			
Length	3,500	3,500			
Width	5	5			
i	0.0217	0.0217			
Q (cf/day)	1,899	3,798			
Q (gpd)	14,203	28,405			
Add'l WW design Flow Capacity (gpd)	18,000	36,000			
Existing WW Design Capacity (gpd)	15,000	15,000			
Total Capacity (gpd)	33,000	51,000			

Existing School WW design Flow (gpd)	15,000
Existing Downtown WW design Flow (gpd)	30,000
Total Exist School & Downtown WW Design Flow (gpd)	45,000

Holliston Woodsand Street School Complex Site					
MW #	GW Elev	Distance	(ft)	GW Elev change	slope (i)
1	170.49	MW #1 to #2	350	7.6	0.0217
2	178.09				
3	177.15	MW #3 to #5	750	7.51	0.0100
5	169.64				

1 INTRODUCTION

This Report presents a summary of hydrogeologic information for watersheds that affect or are affected by reclaimed water reuse/disposal from the Holliston Woodland Street School Complex Wastewater Treatment Plant (WWTP). Historical and project collected data is presented, along with a preliminary groundwater collection plan.

The WWTP receives approximately 7,500 gallons per day (gpd) of wastewater from:

- ✓ Adams Middle School
- ✓ Placentino Elementary Middle School
- ✓ Miller Elementary School

with the treated effluent discharging to three (3) drainfields as shown on Figure 1-1.

MassDEP Groundwater Discharge Permit WP12 GW #652-3 states that "if the 5-day average recorded wastewater flow from the school complex exceeds 22,915 gpd, the mounding in the leach fields shall be re-evaluated."

Average daily wastewater flow is approximately 6,000 gpd for 7-day week and 8,500 gpd for 5-day week.

Sources of existing information are:

- ✓ Simulation of Ground-Water Flow and Evaluation of Water-Management Alternatives in the Upper Charles River Basin, Eastern Massachusetts, by Leslie A. Desimone, Donald A. Walter, John R. Eggleston, and Mark T. Nimiroski, Water-Resources Investigations Report 02-4234, 2002
- ✓ Town of Holliston GIS Web Site, <https://www.mapsonline.net/hollistonma/index.html#x=-7953060.534904,5190297.909296,-7948474.313208,5192399.927574>

✓

While LAI has reviewed the circa 1998 groundwater mounding reports, no data from those reports is being used.

Figure 1-2 illustrates the available area on the property for treated effluent reuse/dispersal. It is anticipated that drip irrigation will be used to maximize site capacity for wastewater disposal.

To place the analysis in context, the analysis objective is to determine the maximum amount of wastewater flow coming from the Holliston downtown area that can be discharged at the School site and be permitted by MassDEP.



Figure 1-1 WWTP and Woodland School Complex – Aerial Photograph

Priority	Location	Approx. Available Area	
		acres	sf
1	Unnamed Ballfield	1.0	43,560
2	West of W Adams DF	0.5	21,780
3	East of East Adams DF	0.5	21,780
4	Former Flagg School	1.0	43,560
5	Adjacent to Miller DF	0.33	14,375
	Total	3.3	145,055



Figure 1-2 Available Areas for Subsurface Disposal

2 WATERSHED INFORMATION

Figure 2-1 presents the watershed areas that affect and are affected by project site.

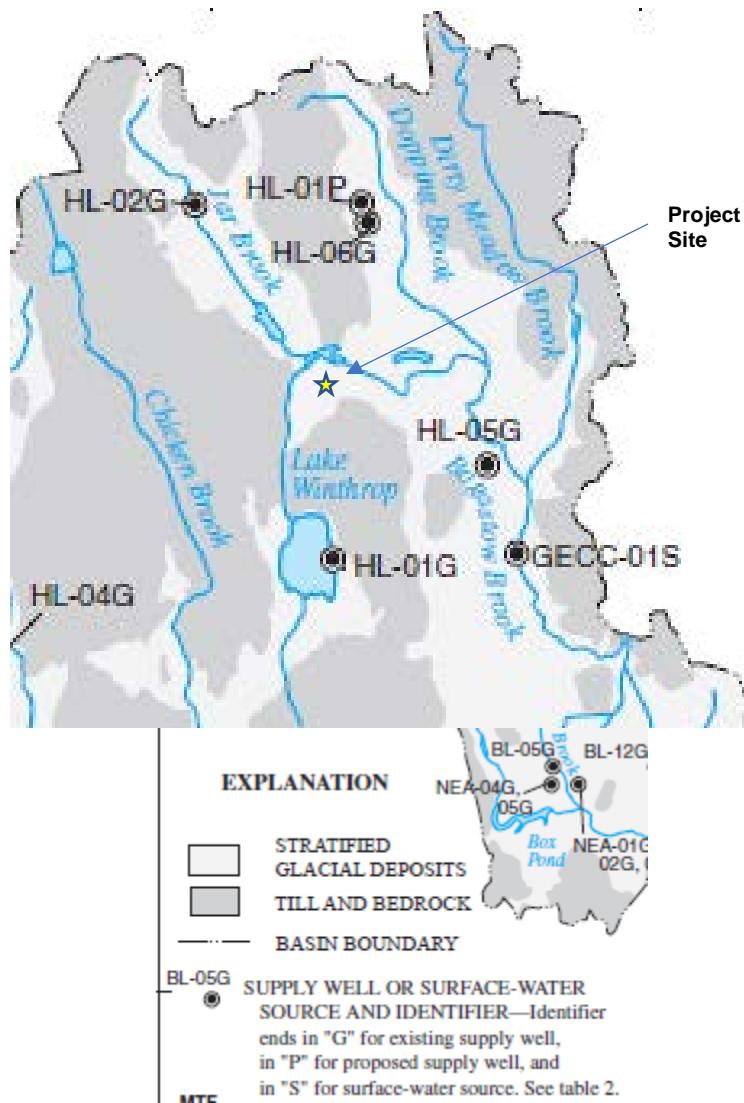


Figure 2-1 Project Site - Watershed Areas

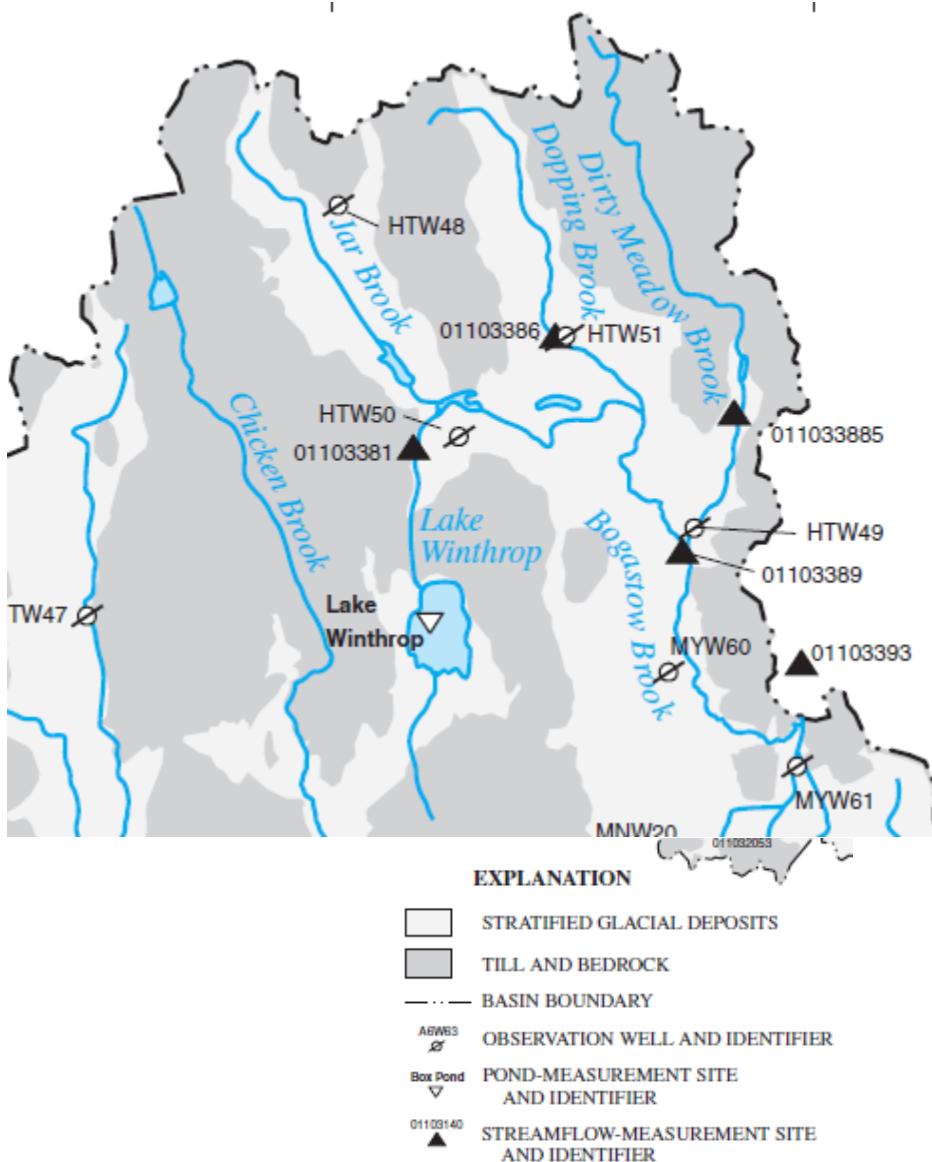


Figure 2-2 USGS Observation wells & streamflow-measurement sites Near Project Area



Figure 2-2A USGS Observation well on Aerial Photo

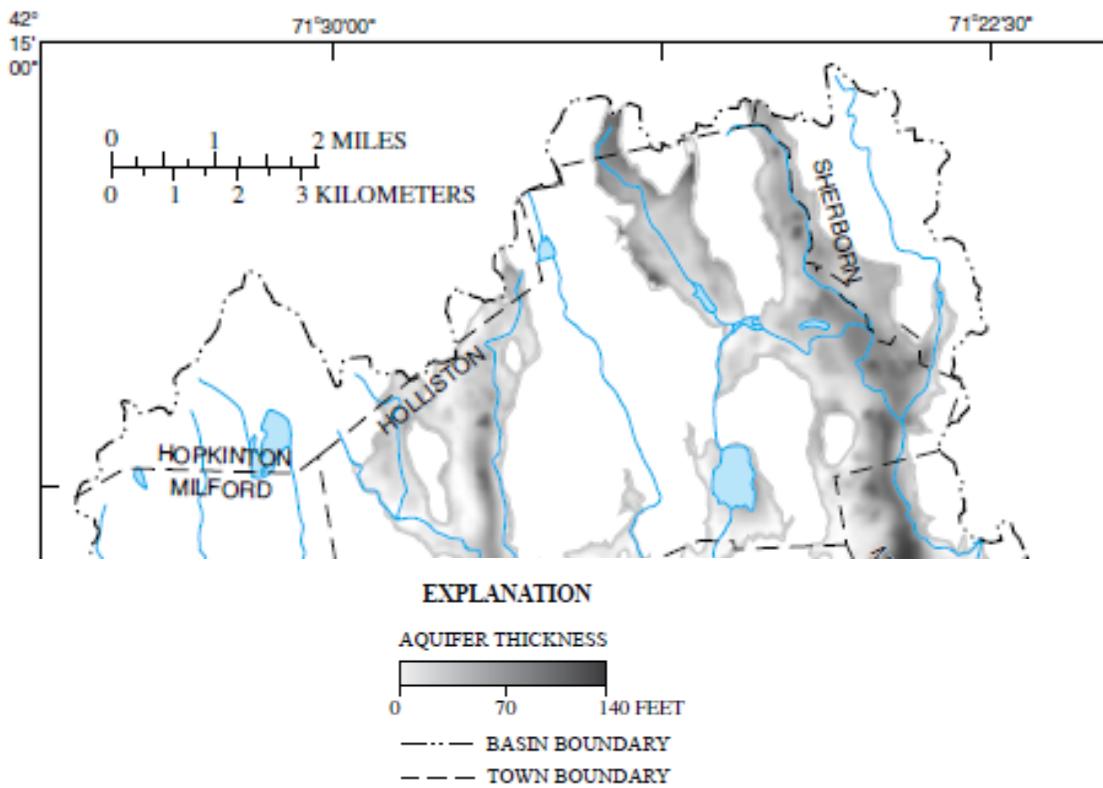


Figure 2-3 USGS 2002 Estimated Thickness of the stratified glacial aquifers

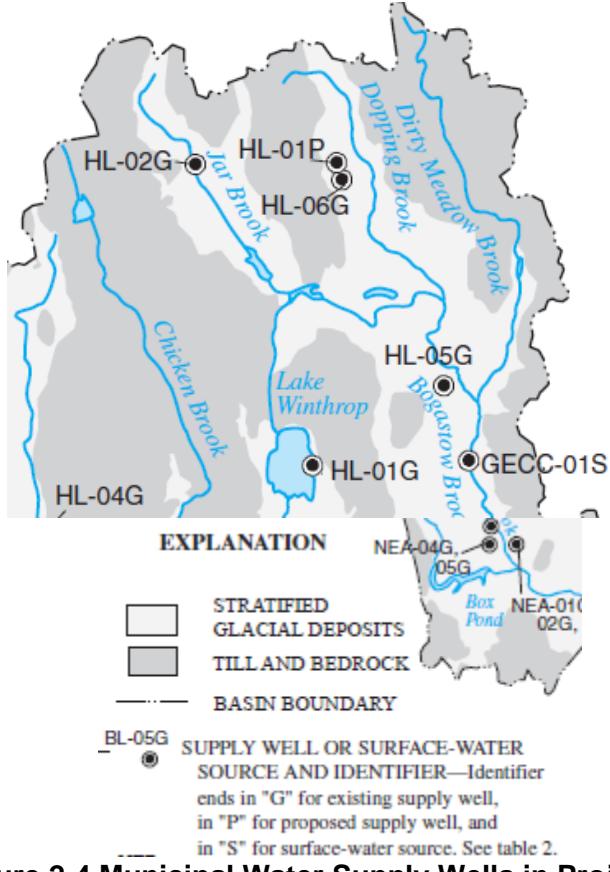


Figure 2-4 Municipal Water Supply Wells in Project Area

Table 2-1 Holliston Water Supply Wells

Well identifier	Town	Source name	Source type	Well depth (feet)	Mean annual withdrawal or discharge rate, 1989–98 (Mgal/d)	Maximum permitted withdrawal rate (Mgal/d)
HL-01G.....	Holliston	Well No. 1 Lake Winthrop	GW	50	.05	.32
HL-02G.....	Holliston	Well No. 2 Maple St.	GW	40	.06	.31
HL-04G.....	Holliston	Well No. 4 Washington St.	GW	50	.14	.48
HL-05G.....	Holliston	Well No. 5 Central St.	GW	60	.45	.71
HL-06G.....	Holliston	Well No. 6 Brook St.	GW	60	.45	.86
HL-01P.....	Holliston	Proposed well No. 7	GW	--	--	.86

Aquifer test well site	Predominant grain size of tested interval	Length of test (days)	Well discharge (gal/min)	Transmissivity (ft ² /d)			Saturated thickness (ft)	Mean hydraulic conductivity (ft/d)	Specific yield	Reference
				Mean	Minimum	Maximum				
HL-01G	Medium to coarse sand and gravel, silt	2	190	7,550	7,032	7,990	60	126	0.07–0.14	Whitman and Howard, Inc., 1996
HL-02G	Medium to coarse sand and gravel	2	240	4,850	4,520	5,070	30	162	0.13–0.15	Whitman and Howard, Inc., 1996
HL-05G	Medium to coarse sand and gravel	2	440	12,800	11,900	13,800	52	247	0.19	Whitman and Howard, Inc., 1996
HL-06G	--	--	--	11,230	--	--	47	239	--	Whitman and Howard, Inc., 1996

Station No.	Station name	Years of historical data	Total number of measurements	Drainage-area characteristics		
				Area (mi ²)	Area of sand and gravel (percent)	Mean slope (percent)
Bogastow Brook and Tributaries						
01103381	Winthrop Canal at Lindon Street, Holliston	none	11	2.7	11	1.9

Well identifier or pond name	Latitude ° ′ ″	Longitude ° ′ ″	Well depth (feet below and surface)	Mean depth to water (feet below land surface)	Mean water-level elevation (feet above sea level)	
					Water year 2000	Estimated, 1989–98
Holliston						
HTW50	42 12 22	071 25 13	12.0	6.71	167.16	167.07

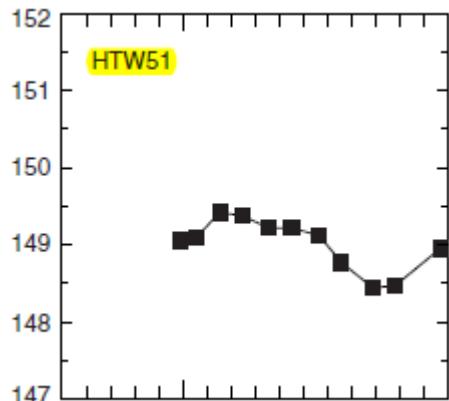


Figure 2-5 Ground-water levels measured in selected observation wells 1999–2000

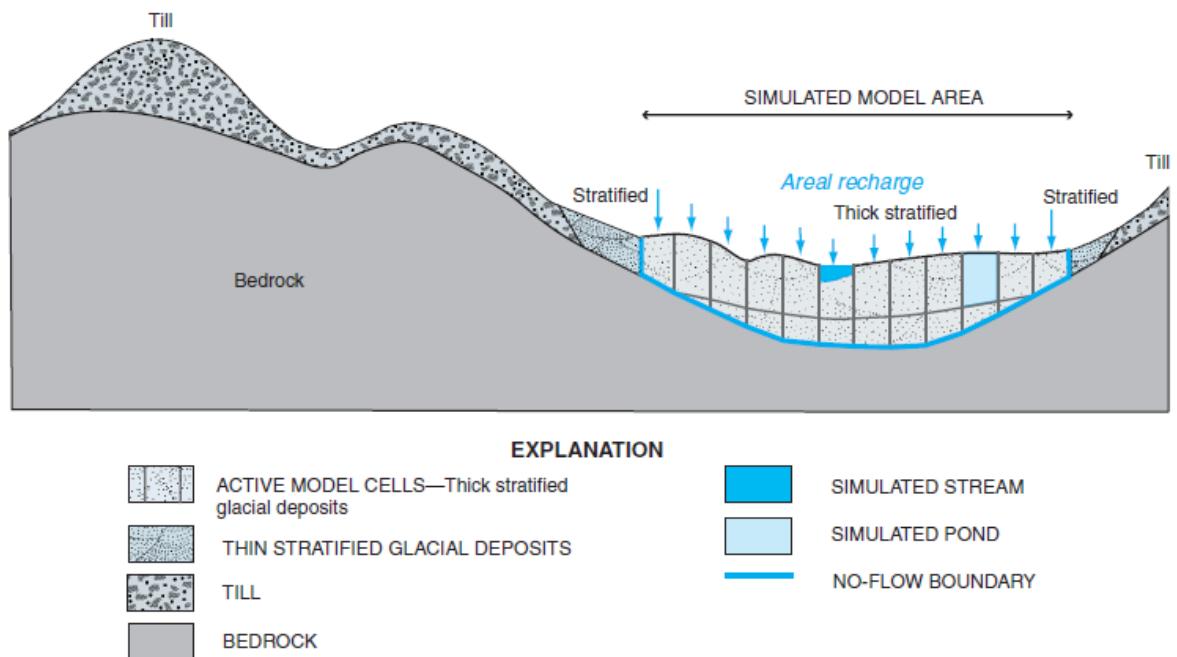
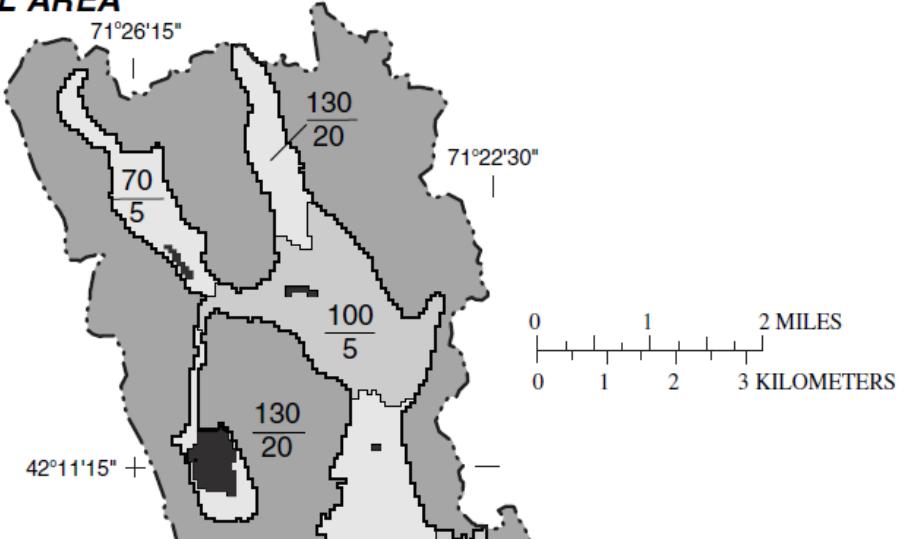


Figure 2-6 Schematic diagram of hydrogeologic units & flow components of water balance

B. EAST MODEL AREA



EXPLANATION



INACTIVE MODEL AREA



HYDRAULIC CONDUCTIVITY ZONES—Upper number is horizontal hydraulic conductivity and lower number is vertical hydraulic conductivity, in feet per day

**290
70**



Figure 2-7 Hydraulic conductivity zones for the USGS 2002 simulation models

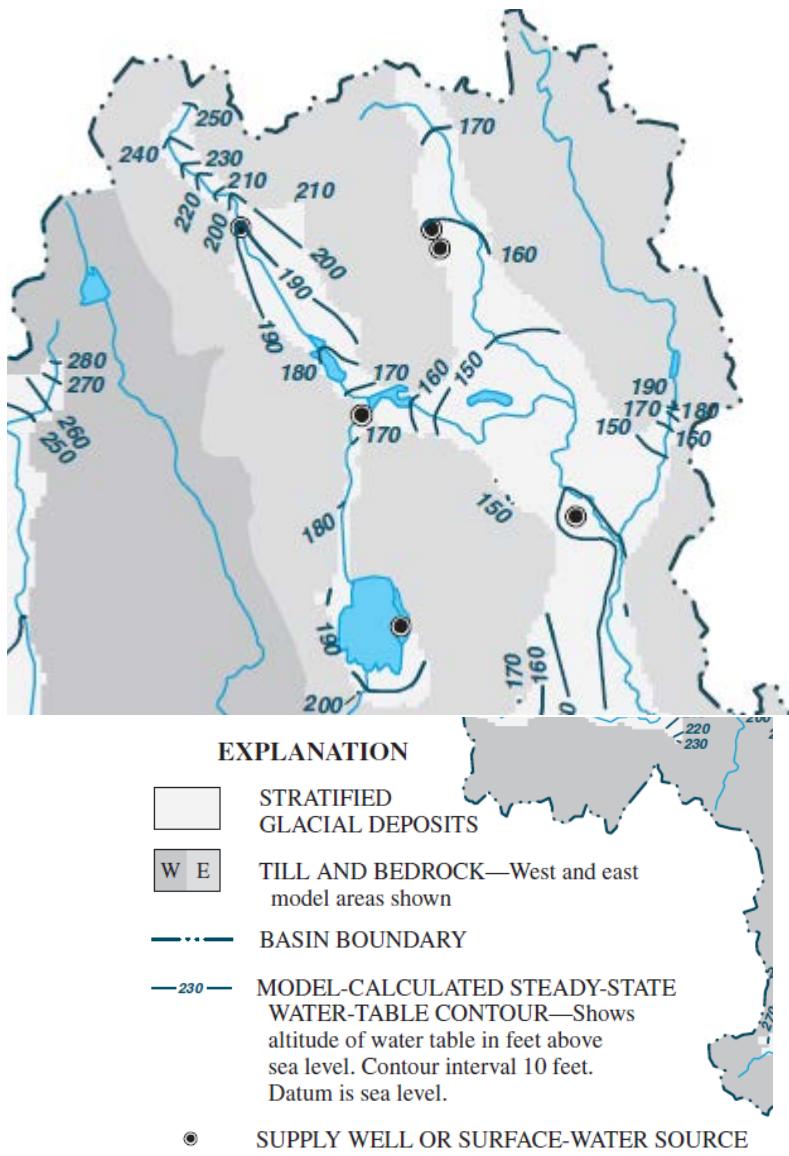


Figure 2-8 Model-calculated steady-state water table

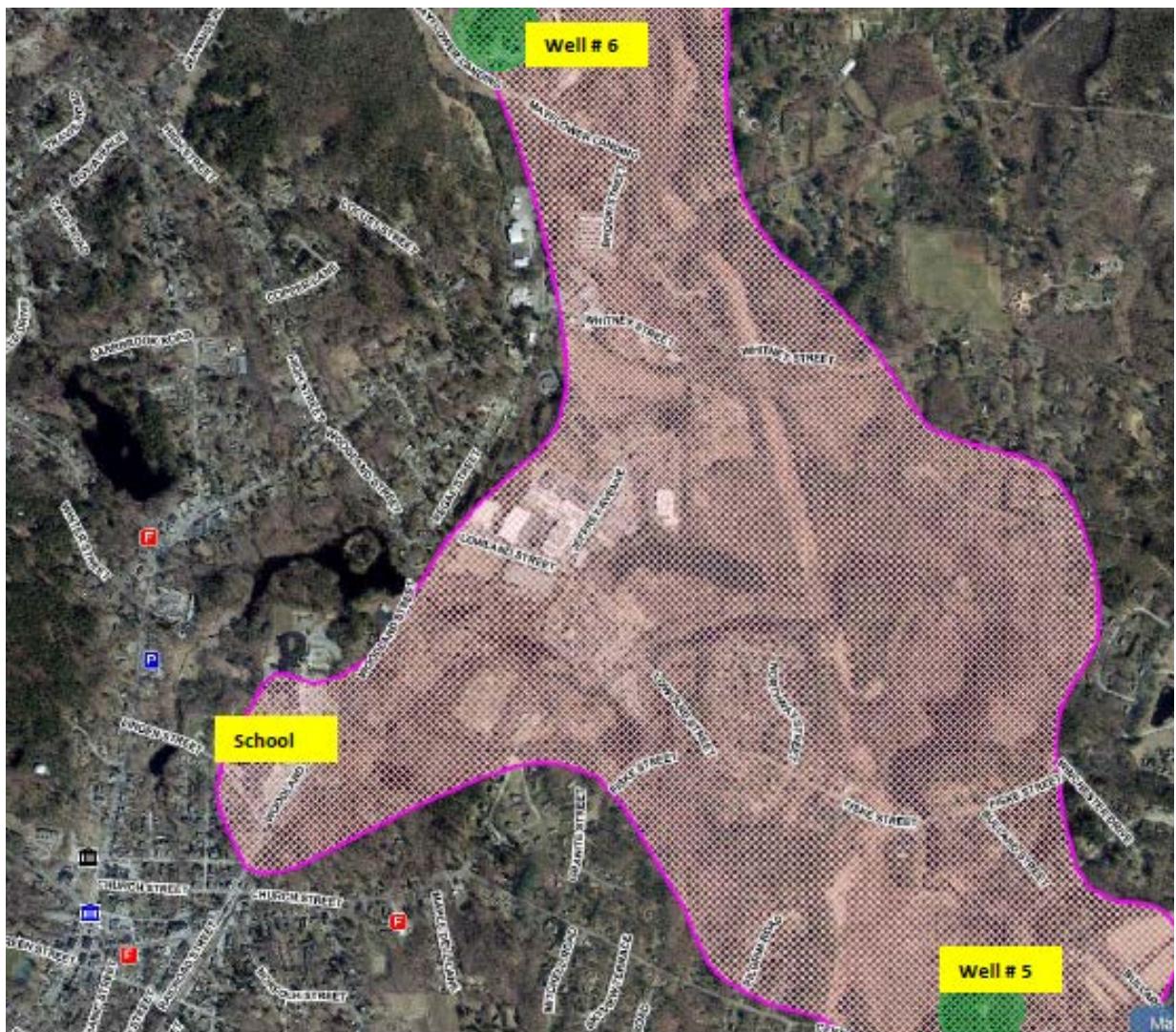


Figure 2-9 Zone II near School Site

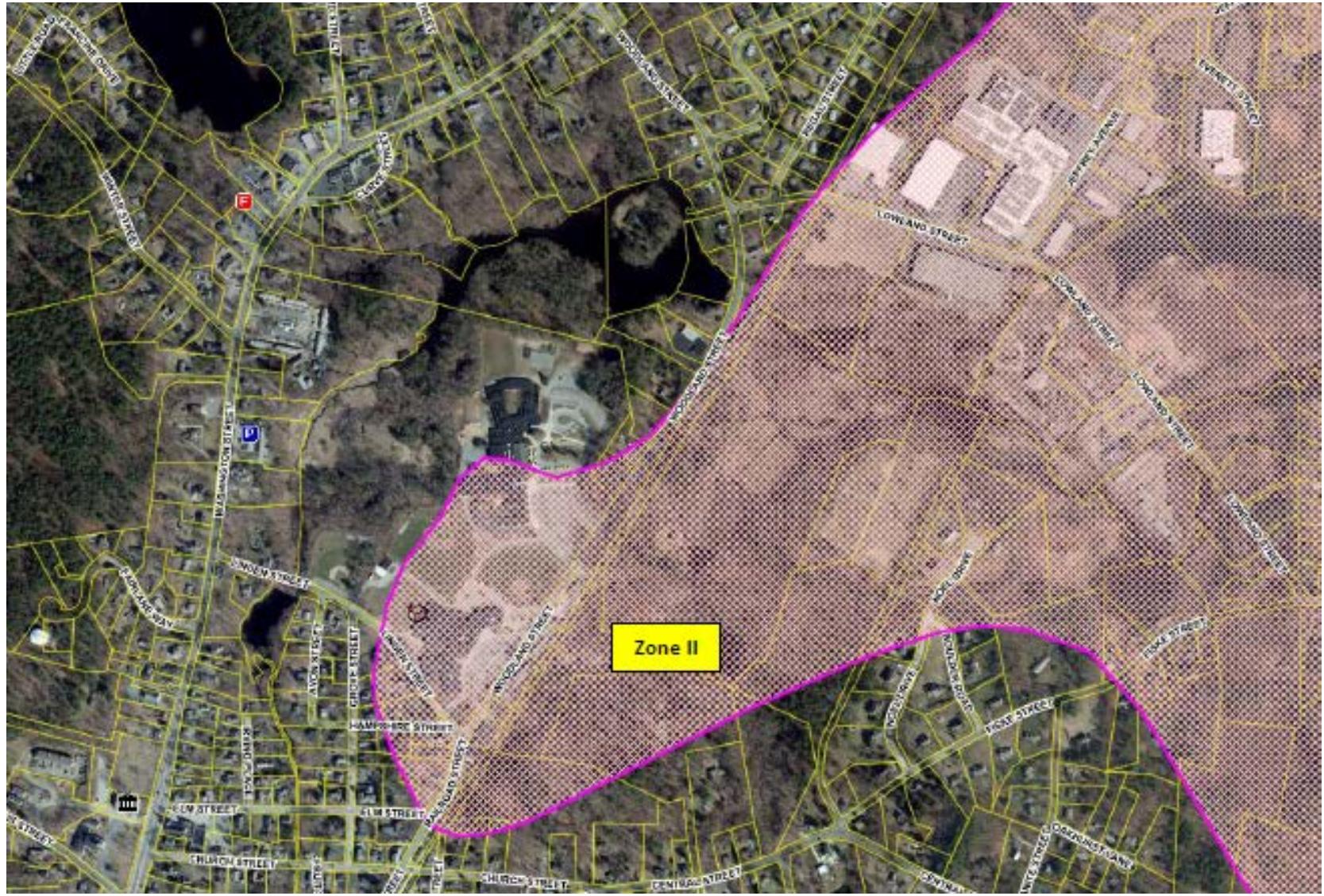


Figure 2-10 Holliston Zone II Map on Aerial at Project Site

Figure 2-11 Wetlands Map

3 PROJECT SITE BORINGS, SIEVE ANALYSIS, SLUG TESTS, HYDRAULIC CONDUCTIVITY ESTIMATES & GROUNDWATER MONITORING WELLS/ELEVATIONS

The Verdantas hydrogeology report is presented in Appendix C

3.1 SITE BORINGS

The location of August 2022 project performed borings are illustrated on Figure 3-1 and presented on Table 3-1.



Figure 3-1 WWTP and Woodland School Complex – Photo with Recent Borings

Table 3-1 Soil Boring Coordinates

Woodland Street School Complex		
Soil Boring Locations		
Label	Latitude	Longitude
B-1	42.205079	-71.424718
B-2	42.204784	-71.424322
B-3	42.205141	-71.424420
B-4	42.205277	-71.424072
B-5	42.205084	-71.423981
B-6	42.205493	-71.422611
B-7	42.205504	-71.422119
B-8	42.205664	-71.421899
B-9	42.205554	-71.421451
B-10	42.205657	-71.423118
B-11	42.206182	-71.424024
B-12	42.208810	-71.421520
B-13	42.208723	-71.422010
B-14	42.208802	-71.422457
B-15	42.208262	-71.422396

3.2 HYDRAULIC CONDUCTIVITY ESTIMATES BASED UPON SIEVE AND SLUG TESTS**Table 3-2 Hydraulic Conductivity Estimates Based Upon Sieve Analysis**

Hydraulic Conductivity Estimates Based Upon Sieve Analysis				
Boring Location	Depth Below Surface Interval	Geo Mean (ft/day)	Arith Mean (ft/day)	Soils Description
B4	0-10	33	133	poorly sorted gravelly sand low in fines
	10-16	98	267	poorly sorted sandy gravel low in fines
	16+	55	858	poorly sorted sandy gravel low in fines
B6	5-12	18	82	poorly sorted gravelly sand low in fines
	10-15	33	138	poorly sorted gravelly sand low in fines
	15+	32	150	poorly sorted sandy gravel low in fines
B9	15-17.5	15	57	poorly sorted gravelly sand low in fines
	0-9	27	94	poorly sorted gravelly sand low in fines
	9-14.5	87	184	poorly sorted gravelly sand low in fines
B11	5-10	20	91	poorly sorted gravelly sand low in fines
	10-14	32	142	poorly sorted gravelly sand low in fines
B12	0-10	2	14	poorly sorted gravelly sand low in fines
B-13	5-15	0.4	33	poorly sorted sandy silt low in fines
	15-20	1	29	poorly sorted sand low in fines

Table 3-3 Hydraulic Conductivity Estimates Based on Sieves Utilizing Various Techniques – B-4 Site Only

B-4 0 - 10 feet below grade	
Estimation of Hydraulic Conductivity	ft/day
Hazen	
Hazen K (cm/s) = d10 (mm)	
Slichter	
Terzaghi	1.26
Beyer	
Sauerbrei	3.74
Kruger	
Kozeny-Carmen	254
Zunker	197
Zamarin	236
USBR	
Barr	0.94
Alyamani and Sen	277
Chapuis	
Krumbein and Monk	94.72
Geometric mean	33
Arithmetic mean	133

Table 3-4 Slug Tests Hydraulic Conductivity Results

**Summary of Slug Test Results in Feet per Day
Holliston School December 2022**

Well	Trial 1	Trial 2	Notes
MW-1	5	n/a	
MW-2	20	15	note 1
MW-3	7	n/a	note 1
MW-4	n/a	n/a	no well installed
MW-5	2	2	
MW-6	n/a	n/a	dry
MW-7	5	4	
MW-8	6	5	
USGS	1	n/a	assumed 10 foot screen

Notes

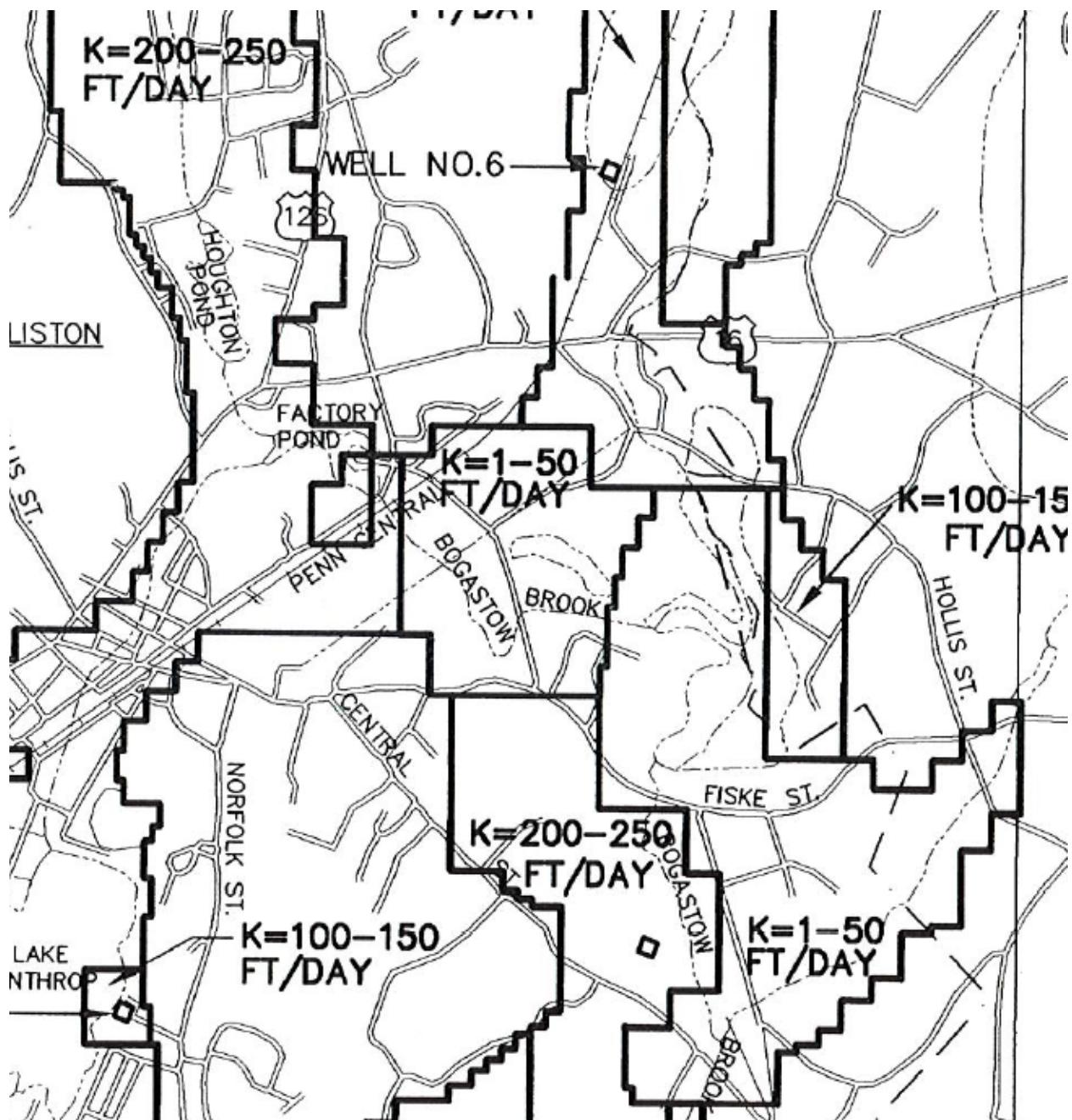
1. Analysis from slug in data which is not valid for wells without a fully saturated screen.

Results are presented because slug out data was unusable.

Despite this, MW-3 results are comparable to others.

However, higher MW-2 results should be viewed as questionable.

Hydraulic Conductivities of Zone II Model – 1999 Bogastow Brook Aquifer Layer 1



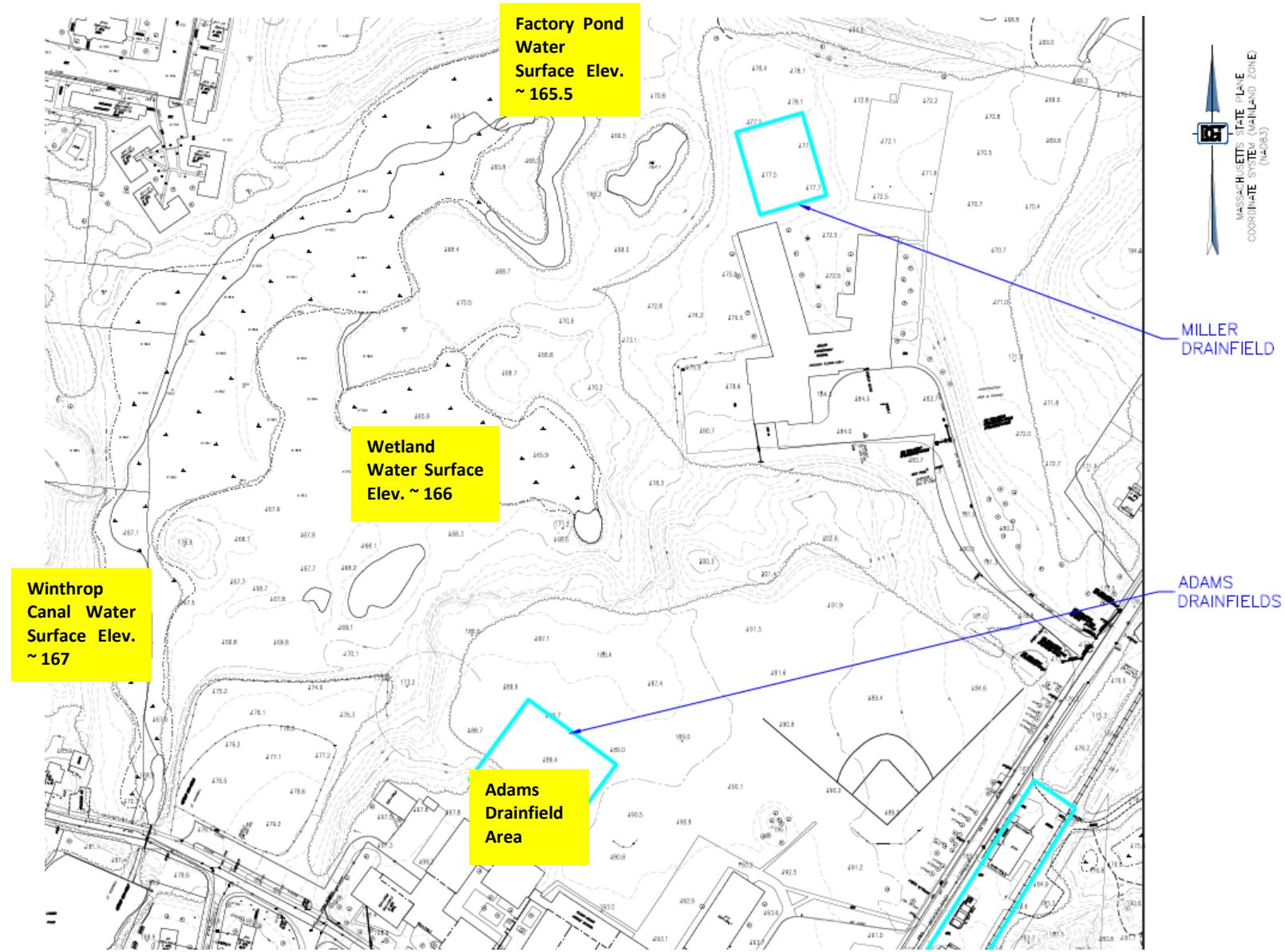
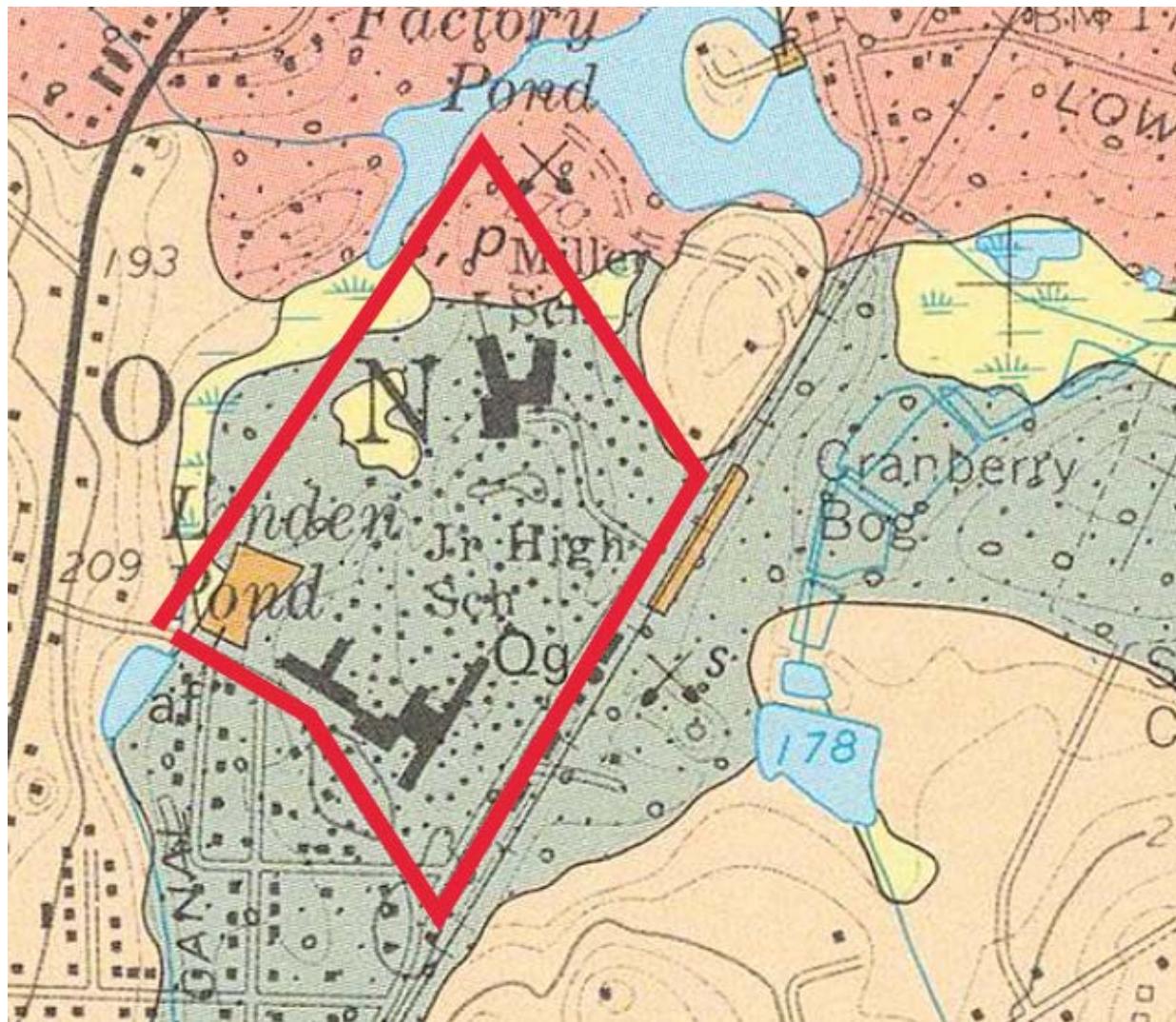


Figure 3-2 1998 Survey



https://ngmdb.usgs.gov/ProdDesc/proddesc_10755.htm

Qs	SWAMP DEPOSITS (HOLOCENE AND PLEISTOCENE(?)) – Sand, silt, clay, and organic material deposited in poorly drained areas
Qg	UNCORRELATED SAND AND GRAVEL DEPOSITS (WISCONSIN) – Of probable glacial origin
	GLACIAL-STREAM AND GLACIAL-LAKE DEPOSITS (WISCONSIN)
Qcs	CEDAR STREET DEPOSITS – Glacial-stream sand and pebble gravel along Cedar Street
Qd	DOPPING BROOK DEPOSITS – Glacial-stream sand and pebble to cobble gravel along Dopping Brook
Qb	BOGASTOW BROOK DEPOSITS – Glacial-lake and glacial- stream sand and pebble to boulder gravel in the vicinity of Bogastow Brook
Qm4	MEDWAY DEPOSITS – Glacial-lake sand and silt and glacial- stream sand and pebble to boulder gravel deposited in or

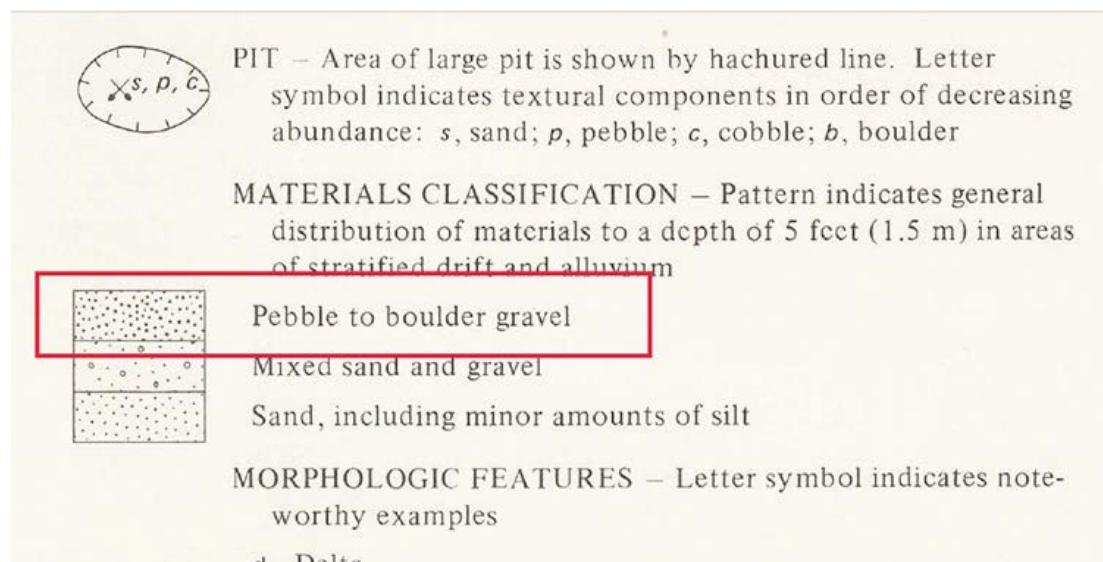


Figure 3-3 USGS Surficial Geology Map

Figure 3-4 Site Soils Map

Table 3-6 Site Soils Units

424C	Canton fine sandy loam, 8 to 15 percent slopes, extremely bouldery	2.6	5.8%	
655	Udorthents, wet substratum	6.3	13.9%	
656	Udorthents-Urban land complex	30.6	67.6%	
Totals for Area of Interest		45.2	100.0%	

Site soils are primarily are in the map unit symbol 656.

4 GROUNDWATER ELEVATION MONITORING RESULTS

4.1 GROUNDWATER MONITORING WELLS, ELEVATIONS AND CONTOURS

Fifteen (15) wells (B-1 through B-15) were installed in August 2022 and seven (7) of the proposed eight (8) (GW MW #4 not installed) groundwater monitoring wells were installed in Nov. 28 -30, 2022 – see Figures 4-1A and Figure 4-1B.



Figure 4-1A Locations of Temporary & Permanent GW Monitoring Wells



Figure 4-1B Locations of Permanent GW Monitoring Wells



Figure 4-2 Soils Cross Sections - August 2022 Borings

Table 4-1 Groundwater Elevations – August 2022 Data

Holliston Schools Site – 100 Linden Street - Soil Boring Locations										Est. DGW	Est GW Elev	Est Avg GW Elev
Field	LAI Location ID	Verd Location ID	Description	Original Survey Elev.	Approx. Surface elev.	Boring Date	Boring Depth (ft)	Depth to GW (ft)	Depth to Refusal (ft)			
Damigella Field	D-1	B-6	NW corner / Left field corner	191.07		15-Aug-22	19.5	>19.5	19.5	20	171.07	173.77
		B-7		189.40		15-Aug-22	17	>17	17	20	169.40	
	D-2	B-8	Center field	191.20		16-Aug-22	13	12	13		179.20	
	D-3	B-9	right field	188.40		16-Aug-22	17.5	13	17.5		175.40	
Former Flagg School	F-1	B-2	South	197.54		15-Aug-22	10.5	>10.5	>10.5	11	186.54	~ 177
	F-2	B-1	NW	197.99		15-Aug-22	11.5	>11.5	>11.5	12	185.99	
	F-3	B-3	North	197.98		15-Aug-22	5	>5	>5	6	191.98	
		B-5	East	198.00		15-Aug-22	17	>17	17	20	178.00	
	F-4	B-4	NE	197.27		15-Aug-22	19	~19+	19	20	177.27	
Adams Ballfield with Drainfields	W-1	B-10	East	189.46		16-Aug-22	24	15	24		174.46	174.08
	W-3	B-11	West	188.70		16-Aug-22	13	>13	13	15	173.70	
Miller School Field	M-1	B-15	South	176.51		16-Aug-22	15	14	>15		162.51	162.89
	M-2	B-14	NW	176.16		16-Aug-22	20	15	>20		161.16	
	M-3			172.00		16-Aug-22	25	7	>25		165.00	
Esker Area	E-1	B-12	Top of Esker	184.00		16-Aug-22	25	17	>25		167.00	

Table 4-2 Groundwater Elevations – Nov. 2022 – April 2023 Data

Brief Description	GW MW Location #	Top of casing	Well Depth			Depth to GW (feet)					
			11/30/22	12/15/22	Bottom Elev	30-Nov-22	15-Dec-23	14-Jan-23	25-Jan-23	13-Mar-23	13-Apr-23
New Wells											
Western edge of Adams Ballfield	1	191.21	24	23.2	168.01	21.5	20.72	19.93	19.29	20.5	20.63
Kindergarten playground area	2	188.76	18	17.8	170.96	11.0	10.67	9.58	9.27	9.65	9.77
Entrance - across from Holl Oil	3	187.86	20	18.75	169.11	14.5	10.71	9.19	8.82	9.03	9.22
N main entrance - across from USGS well	4		10								
eastern edge property	5	176.64	17	16.6	160.04	9.0	7.00	5.00			
Flagg School area	6	198.10	13.5		184.60	>13.5	>13.5	13.5	12.85	>13.5	>13.5
SE entrance along woodland St	7	193.70	19.2	18.8	174.90	14.49	14.31	13.3	13.02	12.53	12.45
NE of Miller School	8	171.01	14	13.8	157.21	3.57	3.62	2.63	2.15	2.89	3.2
Existing wells											
Flagg School	1	198.1	19.4		178.70	dry	dry	18.18		18.16	18.23
Miller School	2	178.93	16.92		178.93	11.95		11.13	10.67	11.49	11.8
USGS	HTW 50	181.07	13	13	168.07	8.4	7.92	6.6	6.02	6.88	7.15

Brief Description	GW Elevation (feet)					
	30-Nov-23	15-Dec-23	14-Jan-23	25-Jan-23	13-Mar-23	13-Apr-23
New Wells						
Western edge of Adams Ballfield	169.71	170.49	171.28	171.92	170.71	170.58
Kindergarten playground area	177.76	178.09	179.18	179.49	179.11	178.99
Entrance - across from Holl Oil	173.36	177.15	178.67	179.14	178.83	178.64
N main entrance - across from USGS well						
eastern edge property	167.64	169.64	171.64	172.29	172.07	171.91
Flagg School area	<184.6	<184.6	<184.6	185.25	<184.6	<184.6
SE entrance along woodland St	179.21	179.39	180.40	180.68	181.17	181.25
NE of Miller School	167.44	167.39	168.38	168.86	168.12	167.81
Existing wells						
Flagg School			179.92		179.94	179.87
Miller School	166.98		167.80	168.26	167.44	167.13
USGS	172.67	173.15	174.47	175.05	174.19	173.92

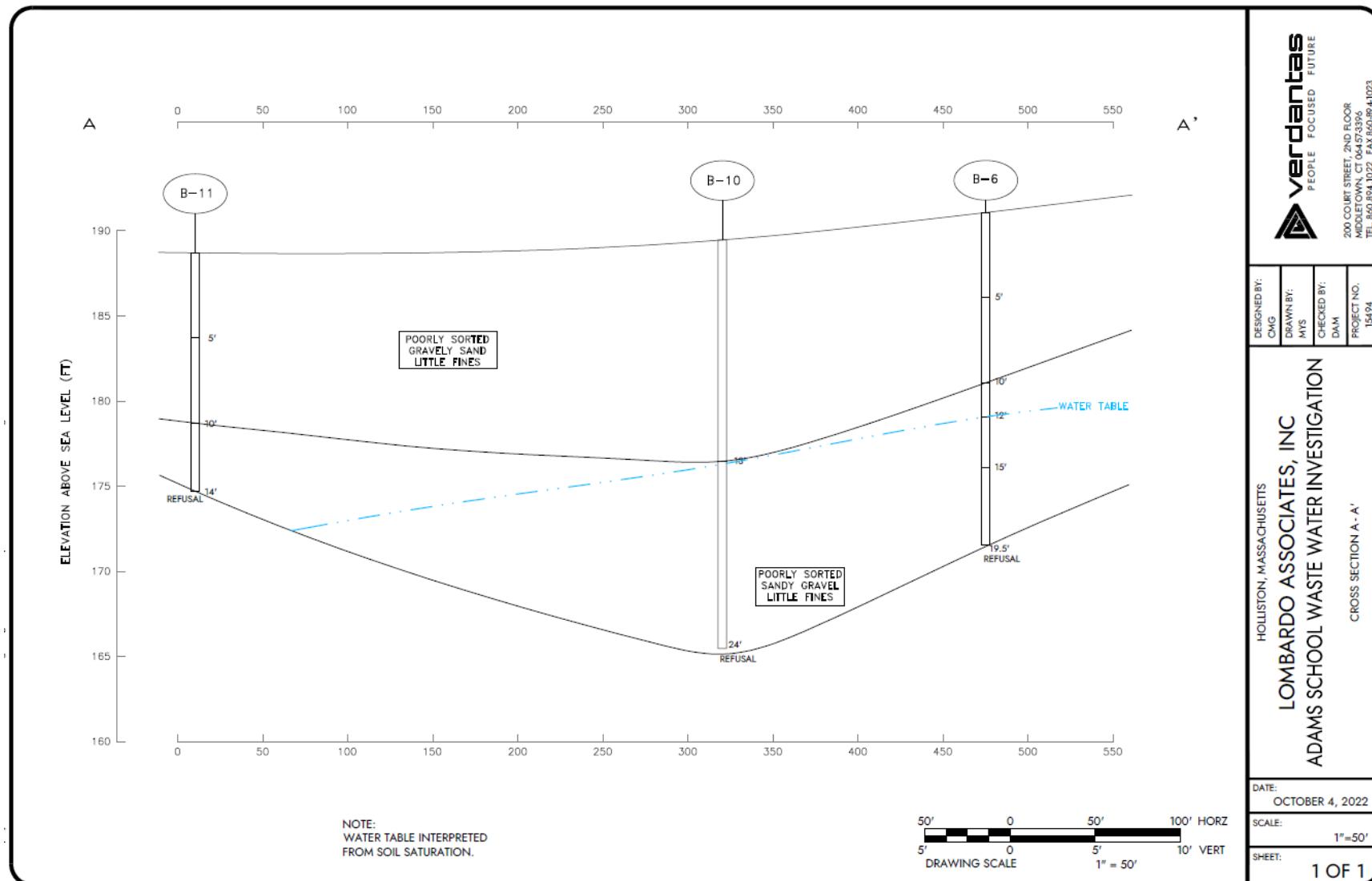


Figure 4-3 Soil Boring Cross Section A – A'

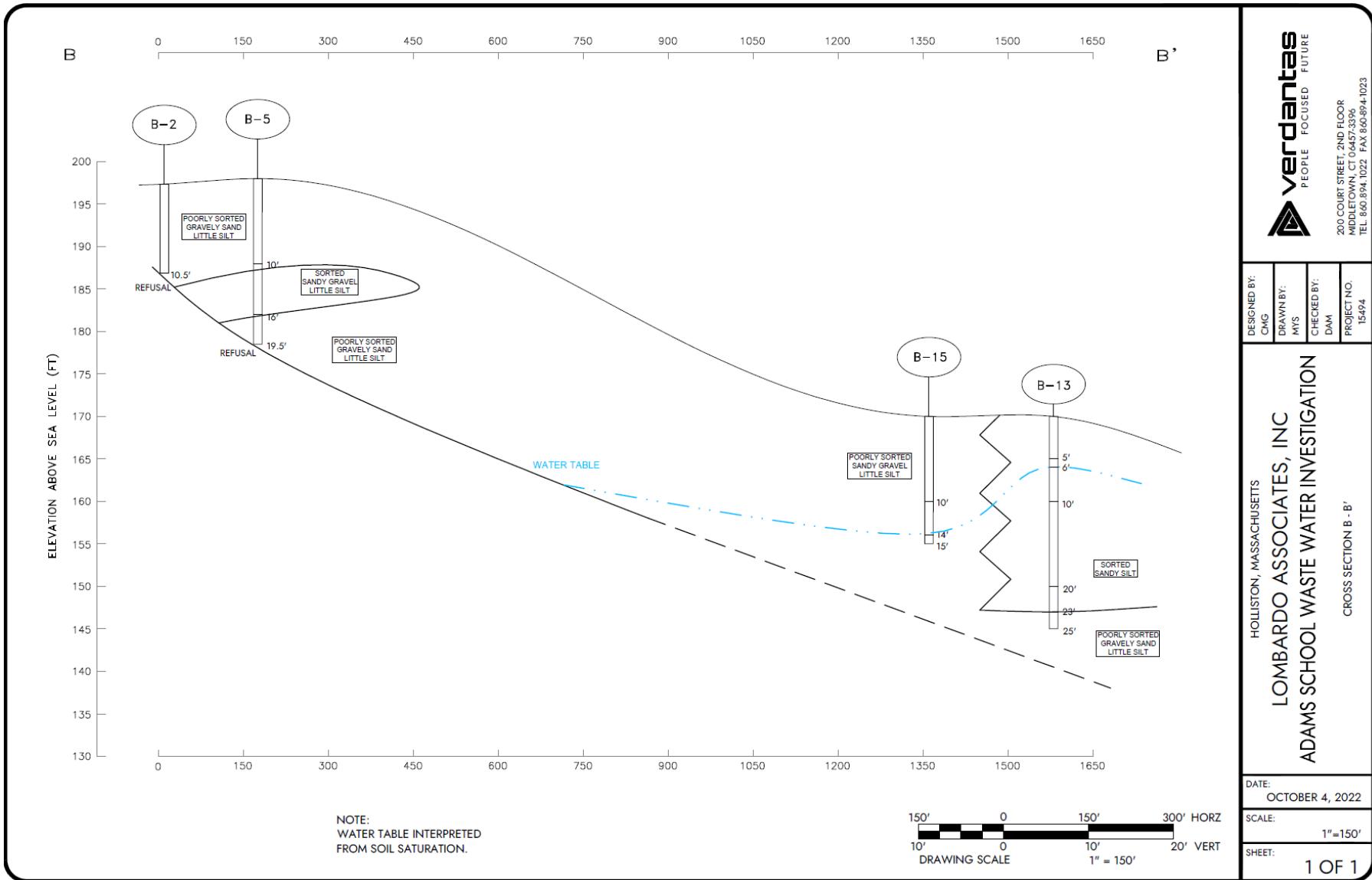


Figure 4-4 Soil Boring Cross Section B – B'

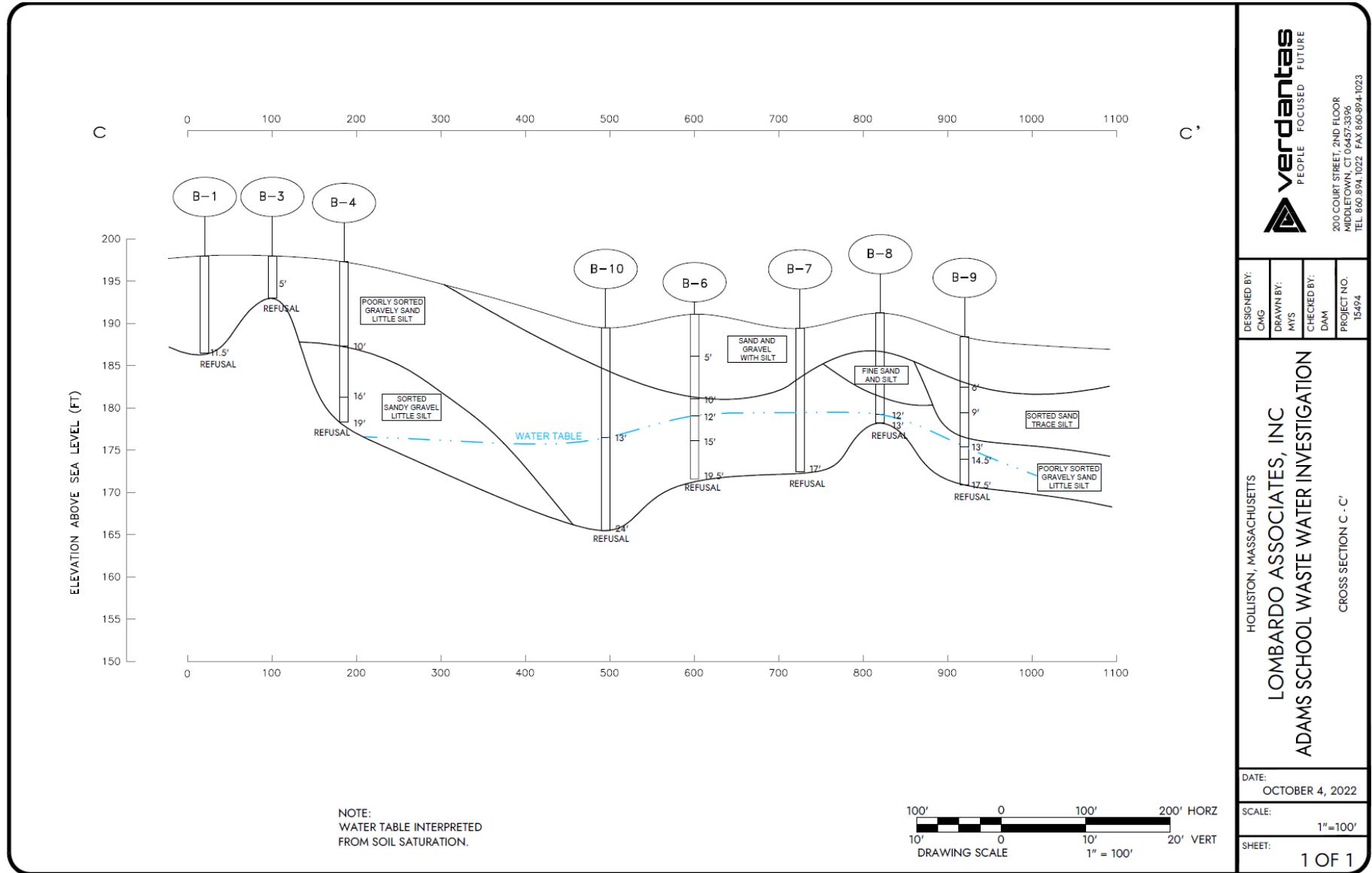


Figure 4-5 Soil Boring Cross Section C – C'

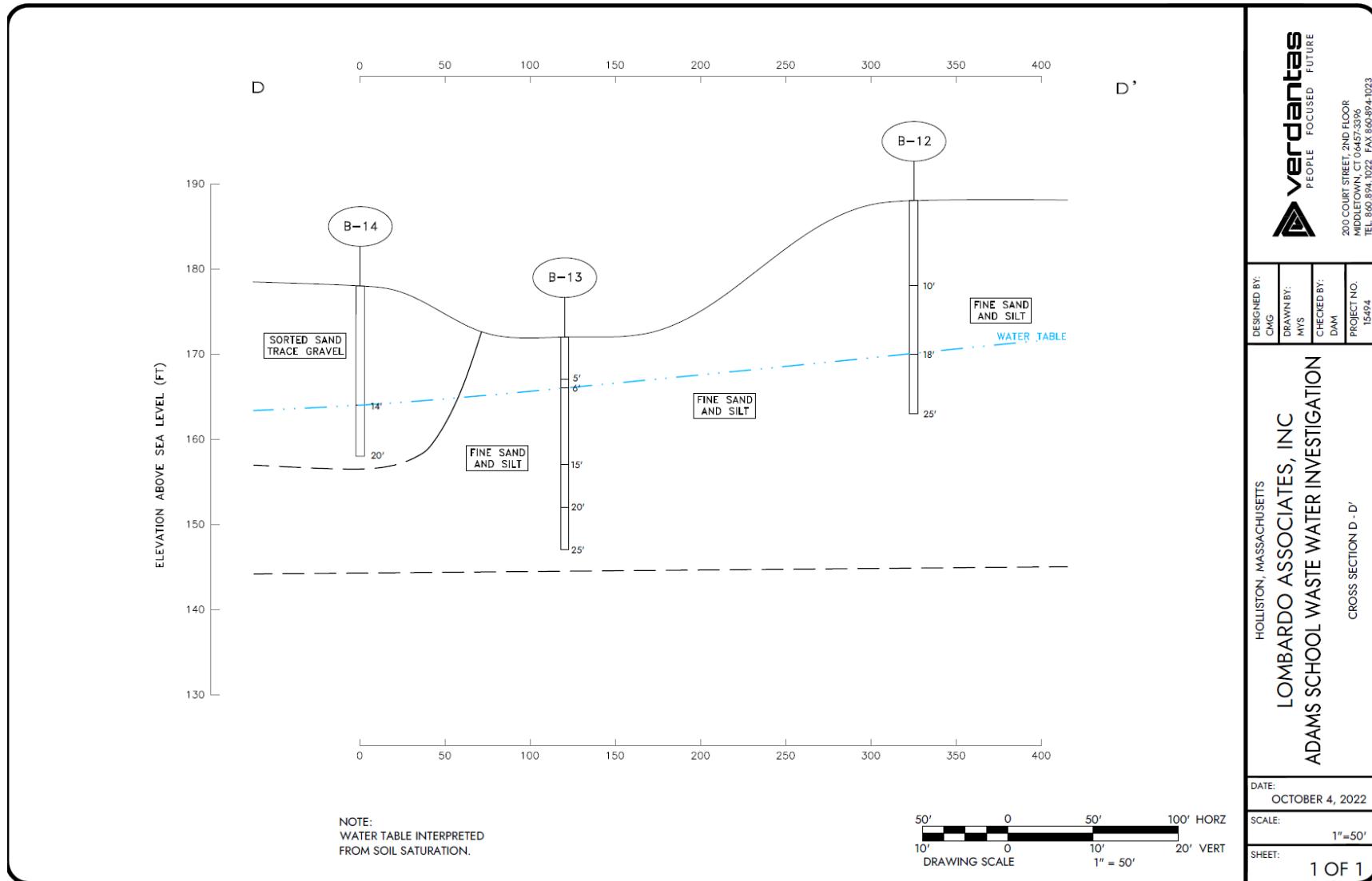


Figure 4-6 Soil Boring Cross Section D – D'

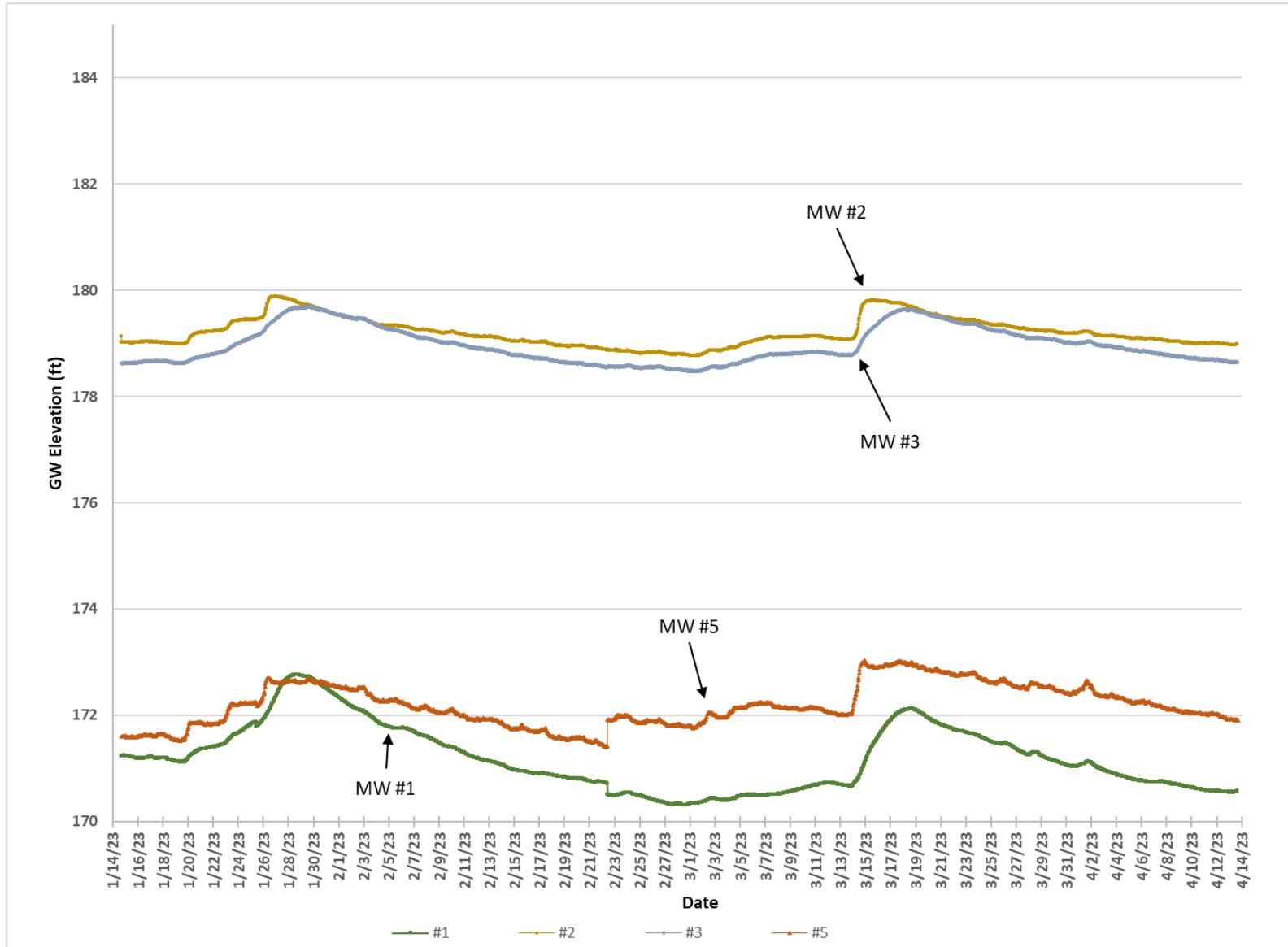


Figure 4-7 Groundwater Elevations at Wells # 1, #2, #3 & #5

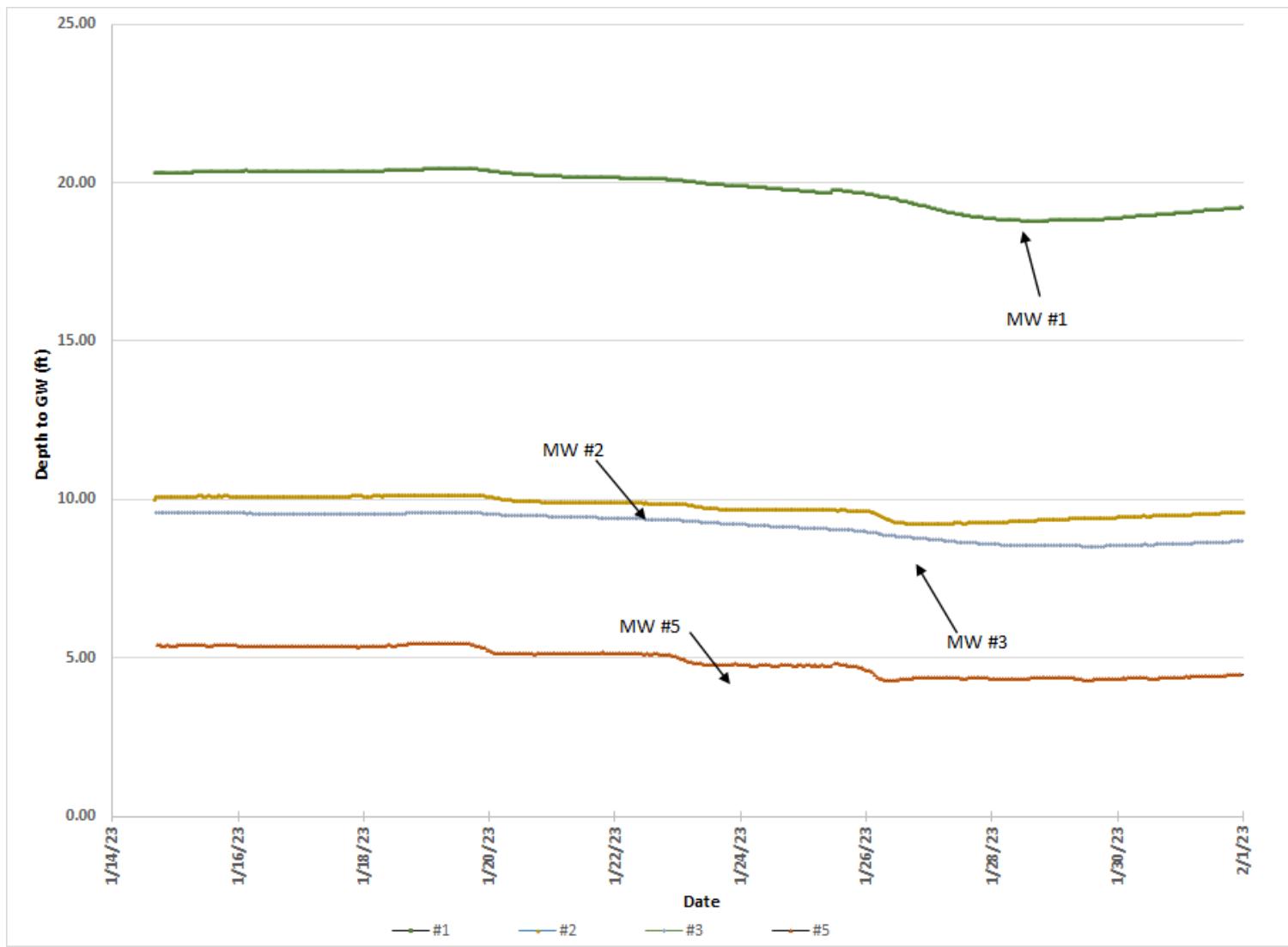
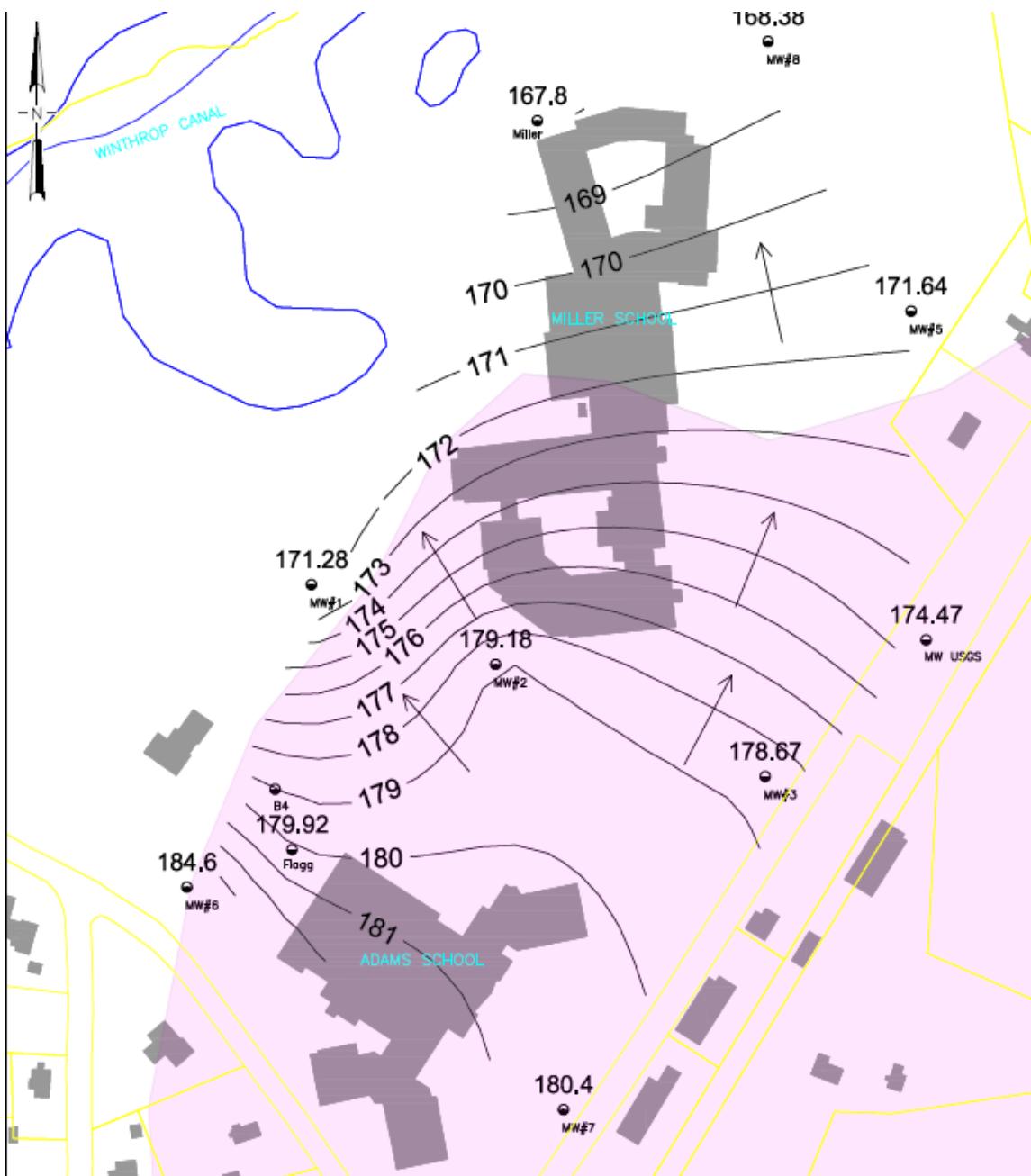


Figure 4-8 Depth to Groundwater at Wells # 1, #2, #3 & #4

4.2 GROUNDWATER CONTOURS



LEGEND

- MW#1
- GROUNDWATER FLOW DIRECTION
- GROUNDWATER CONTOUR 1/14/23
- SURFACE WATER
- PROPERTY BOUNDARY
- EXISTING ZONE II

NOTES:

1. THIS PLAN WAS BASED UPON MASS GIS DATA AND A GROUND SURVEY OF MONITORING WELLS. ALL SITE FEATURES ARE APPROXIMATE.
2. GROUNDWATER MEASURED JANUARY 14, 2023

0 200 400
APPROX. SCALE IN FEET

Figure 4-9 Groundwater Contour Map

4.3 GROUNDWATER SLOPE

Table 4-2 Groundwater Slope

30-Nov-23	15-Dec-23	14-Jan-23	25-Jan-23	13-Mar-23	13-Apr-23
GW Elev Change (ft)					
8.05	7.6	7.9	7.57	8.4	8.41
7.51	7.03	6.85	6.85	6.76	6.73
Slope					
0.0230	0.0217	0.0226	0.0216	0.0240	0.0240
0.0100	0.0094	0.0091	0.0091	0.0090	0.0090

4.4 GROUNDWATER VELOCITY & TRAVEL TIME

Groundwater Velocity = $K * i / n$

$$= 10 \text{ feet/day} * .02 / 0.25 = 0.8 \text{ feet / day}$$

6 months = 183 days = 146.4 feet

12 months = 365 days = 292 feet

Water supply well # 5 is at 42°11'56"N, 71°23'52"W, approximately 13,000 feet downstream in Bogastow Brook from School area. Well is off Central Street near Powderhorn Lane

5 SITE CAPACITY & MOUNDING ANALYSIS

Given the criticality of hydraulic conductivity, it is proposed to be measured in the field by slug tests in the wells using an electronic data logger for measurement of groundwater levels. The test data will be reduced and analyzed using standard methods to estimate hydraulic conductivity of the aquifer.

5.1 PRELIMINARY SITE CAPACITY ANALYSIS

Preliminary Darcy's Law capacity estimates are presented to provide initial insights and sensitivity analysis to the disposal capacities of the School Property.

Darcy's Law Capacity Estimates Methodology

Treated wastewater disposal capacity of candidate areas on the School Property were estimated in the following manner:

1. Available Area determined as shown on Figure 1-2.
2. Darcy's Law disposal capacity of the dispersal zones was calculated at the downgradient face of the zones as follows:

$Q = K * A * i$, where Q = volumetric flow (cf/day),
 K = Hydraulic conductivity (ft/day) of unsaturated zone,
 A = cross sectional area (sf) of discharge cross sectional area
 i = groundwater slope

 - i. Hydraulic conductivity (K) of 20 feet/day, with sensitivity analysis to 140 feet/day per USGS model (Figure 2-7);
 - ii. Test pit data (as presented in Section 4) reviewed to estimate depth to groundwater (GW) / bedrock – see Figures 4-3 through 4-6,
 - iii. Cross Sectional area computed by:
 - ✓ Length measured as the furthest downgradient face of zone
 - ✓ Depth estimated at 3 - 5 feet
4. Darcy's Law flow estimates calculated, with recognition that GW elevations represent ~ 7,500 gpd average daily discharge at the schools which is 15,000 gpd wastewater design flow, which is 12,000 gpd of modelled discharge.

To place the analysis in context, the analysis objective is to determine the maximum amount of wastewater flow coming from the Holliston downtown area that can be discharged at the School site and be permitted by MassDEP.

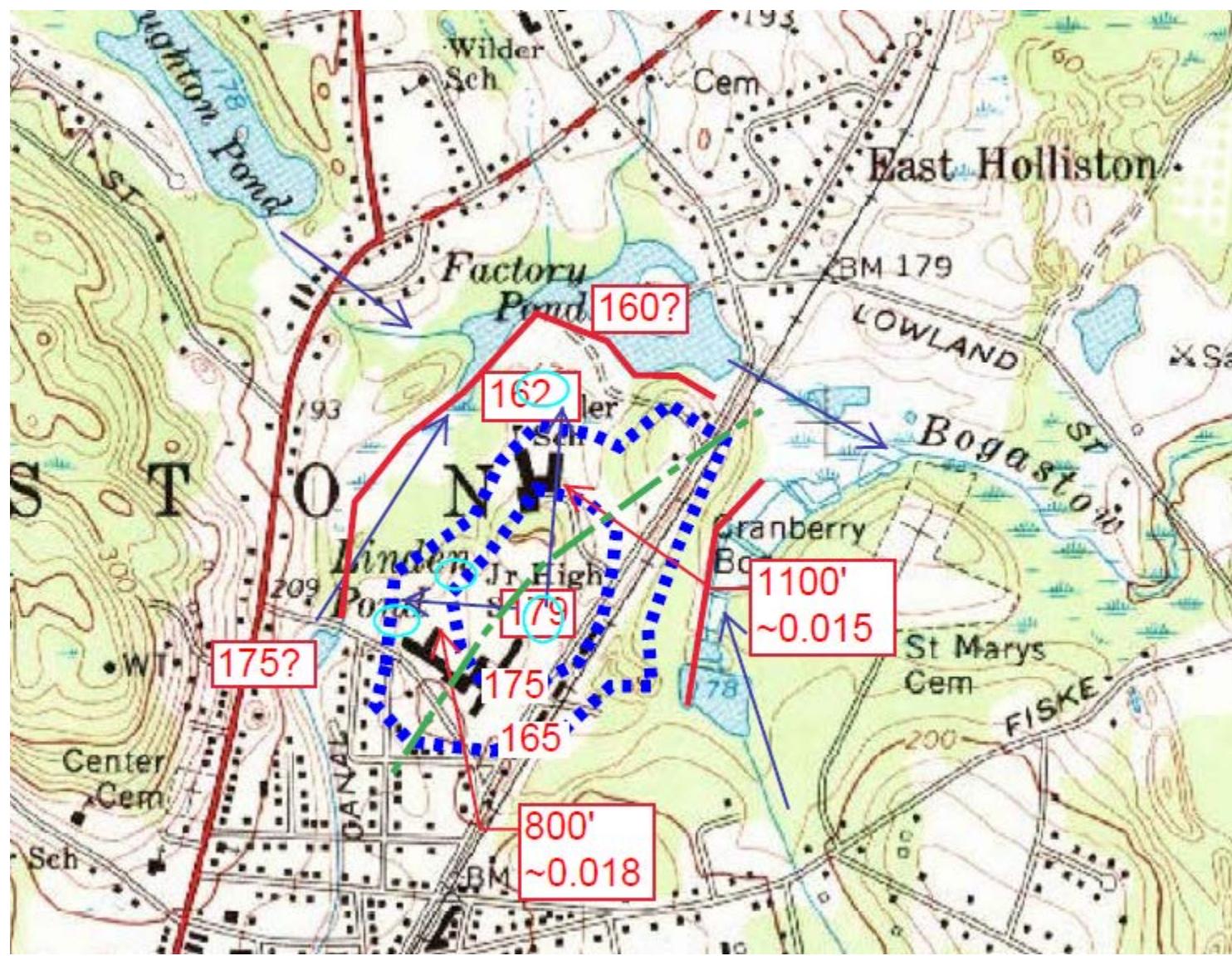


Figure 5-1 Estimated GW Elevations and GW Discharging Flux Lengths

Table 5-1 Disposal Capacity Preliminary Estimates

Holliston Woodsand Street School Complex Site - Discharge Capacity Initial Estimates with Sensitivity Analysis of Values for K & Unsaturated Thickness Available for Discharge					
$Q = K * A * i$	Q = volumetric flow (cf/day),				
	K = Hydraulic conductivity (ft/day) of unsaturated zone				
	A = cross sectional area (sf) of discharge/flux area				
	i = groundwater slope				
K (ft/day)	5	10			
Area (sf)	17,500	17,500			
Length	3,500	3,500			
Width	5	5			
i	0.0217	0.0217			
Q (cf/day)	1,899	3,798			
Q (gpd)	14,203	28,405			
Add'l WW design Flow Capacity (gpd)	18,000	36,000			
Existing WW Design Capacity (gpd)	15,000	15,000			
Total Capacity (gpd)	33,000	51,000			
Existing School WW design Flow (gpd)	15,000				
Existing Downtown WW design Flow (gpd)	30,000				
Total Exist School & Downtown WW Design Flow (gpd)	45,000				

Holliston Woodsand Street School Complex Site						
MW #	GW Elev	Distance (ft)		GW Elev change	slope (i)	
1	170.49	MW #1 to #2	350	7.6	0.0217	
2	178.09					
3	177.15	MW #3 to #5	750	7.51	0.0100	
5	169.64					

5.2 PRELIMINARY MOUNDING ANALYSIS

It is noted that depth from surface to GW needs to consider 1 foot separation between grade and bottom of drip system and 4 foot separation drip bottom to max mounded GW elevation. Therefore minimum five (5) foot separation grade to maximum mounded GW elevation.

The sophisticated mounding analytical model MODFLOW needs to be used due to the limitations of the Hantush method, which assumes a flat groundwater. Site groundwater slopes, multiple groundwater directions and interaction amongst potential drainfields at the site requires the more sophisticated MODFLOW model versus Hantush method.

APPENDIX A SOIL BORING LOGS – AUGUST & NOVEMBER 2022



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: B-1					
Project: Adams School Waste Water Investigation				Sheet: 1 of 1					
Location: Holliston, Massachusetts				Checked By: DGH					
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan					
Foreman: Ed				Ground Surface Elevation: 197.99					
GeoInsight Engineer/Geologist: Chris Griffin				Datum: NA					
				Date Started: 8/15/22					
				Date Completed: 8/15/22					
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS					
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization		
Model: 7822DT		Hammer (lb): NA				Ground Surface			
Method: Direct Push		Fall (in): NA							
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)		
#	Pen/Rec (in)	Depth (ft)	Blows/6"			Sand and Gravel with Silt			
0	S-1	60/30	0-5	0-8": Topsoil					
1				8-18": Brown, very fine to coarse SAND, some Silt, some Gravel, trace Cobbles, trace Brick, trace asphalt, damp.					
2				18-30": Light brown, fine to coarse SAND, some Gravel, some Silt, trace cobbles, damp.					
3									
4									
5	S-2	60/28	5-10	0-28": Light brown, very fine to coarse SAND, some Silt, little Gravel, trace cobbles, damp.					
6									
7									
8									
9									
10	S-3	18/10	10-11.5	0-10": Gray/brown, very fine to coarse SAND, some Gravel, little Silt, trace cobbles, damp.					
11									
12				Refusal encountered at 11.5 feet below ground surface.					
13									
14									
15									
16									
17									
18									
19									
20	GRANULAR SOILS		COHESIVE SOILS		NOTES				
	Blows/ft.	Density	Blows/ft.	Consistency	First attempt refusal encountered at 6.5 feet below ground surface. Groundwater not encountered.				
	0-4	V. LOOSE	<2	V. SOFT					
	5-10	LOOSE	2-4	SOFT					
	11-30	M. DENSE	4-8	M. STIFF					
	31-50	DENSE	8-15	STIFF					
	>50	V. DENSE	15-30	V. STIFF					
			>30	HARD					

SOIL BORING LOG							
 <p>Client: Lombardo Associates, Inc.</p> <p>Project: Adams School Waste Water Investigation</p> <p>Location: Holliston, Massachusetts</p>		Boring Identification: B-2		Sheet: 1 of 1			
				Checked By: DGH		Project Number:	
Drilling Company: Geosearch, Inc.			Boring Location: See Site Plan				
Foreman: Ed			Ground Surface Elevation: NA Datum: NA				
GeoInsight Engineer/Geologist: Chris Griffin			Date Started: 8/15/22 Date Completed: 8/15/22				
			GROUNDWATER MEASUREMENTS				
DRILLING METHOD		SAMPLER		Date	Depth (ft)	Reference	Stabilization
Vehicle: Track Geoprobe		Type: 5' acetate sleeve					
Model: 7822DT		Hammer (lb): NA				Ground Surface	
Method: Direct Push		Fall (in): NA					
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
	#	Pen/Rec (in)	Depth (ft)				
0	S-1	60/30	0-5	0-8": Topsoil 8-24": Brown, very fine to coarse SAND, some Silt, little Gravel, trace Cobbles, damp.	Sand and Gravel with Silt		
1				24-30": Light Brown, fine to medium SAND, little Silt, damp.			
2							
3							
4							
5	S-2	60/35	5-10	0-35": Brown, very fine to coarse SAND, some Gravel, some Silt, trace Cobbles, damp.			
6							
7							
8							
9							
10	S-3	18/10	10-10.5	0-6": Light brown, very fine to medium SAND, some Silt, trace Gravel, damp. 6-10": Brown, fine SAND, trace Silt, damp. Refusal encountered at 10.5 feet below ground surface.			
11							
12							
13							
14							
15							
16							
17							
18							
19							
20							
GRANULAR SOILS			COHESIVE SOILS		NOTES		
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater not encountered.			
0-4	V. LOOSE	<2	V. SOFT				
5-10	LOOSE	2-4	SOFT				
11-30	M. DENSE	4-8	M. STIFF				
31-50	DENSE	8-15	STIFF				
>50	V. DENSE	15-30	V. STIFF				
		>30	HARD				

SOIL BORING LOG								
		Client: Lombardo Associates, Inc.			Boring Identification: B-3			
		Project: Adams School Waste Water Investigation						Sheet: 1 of 1
		Location: Holliston, Massachusetts			Checked By: DGH			Project Number:
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan				
Foreman: Ed				Ground Surface Elevation: NA Datum: NA				
GeoInsight Engineer/Geologist: Chris Griffin				Date Started: 8/15/22 Date Completed: 8/15/22				
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization	
Model: 7822DT		Hammer (lb): NA				Ground Surface		
Method: Direct Push		Fall (in): NA						
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE	
	#	Pen/Rec (in)	Depth (ft)					Blows/6"
0	S-1	48/24	0-5	0-4": Topsoil 4-24": Brown, fine to coarse SAND and GRAVEL, some Silt, trace cobbles, damp.	Sand and Gravel with Silt			
1								
2								
3								
4								
5				Refusal encountered at 5 feet below ground surface.				
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
GRANULAR SOILS			COHESIVE SOILS		NOTES			
Blows/ft.	Density	Blows/ft.	Consistency	First attempt refusal at 4 feet below ground surface. Groundwater not encountered.				
0-4	V. LOOSE	<2	V. SOFT					
5-10	LOOSE	2-4	SOFT					
11-30	M. DENSE	4-8	M. STIFF					
31-50	DENSE	8-15	STIFF					
>50	V. DENSE	15-30	V. STIFF					
		>30	HARD					



SOIL BORING LOG

Client: Lombardo Associates, Inc. Boring Identification: B-4 Project: Adams School Waste Water Investigation Sheet: 1 of 1 Location: Holliston, Massachusetts Checked By: DGH Project Number:								
Drilling Company: Geosearch, Inc.		Boring Location: See Site Plan						
Foreman: Ed		Ground Surface Elevation: NA Datum: NA						
GeoInsight Engineer/Geologist: Chris Griffin		Date Started: 8/15/22 Date Completed: 8/15/22						
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization	
Model: 7822DT		Hammer (lb): NA				Ground Surface		
Method: Direct Push		Fall (in): NA						
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE	
	#	Pen/Rec (in)	Depth (ft)					Blows/6"
0	S-1	60/24	0-5	0-8": Topsoil	Sand and Gravel with Silt			
1				8-12": Gray, very fine to coarse SAND and GRAVEL, some Silt, trace cobbles, damp.				
2				12-24": Brown, very fine to coarse SAND and GRAVEL, some Silt, trace cobbles, damp.				
3								
4								
5	S-2	60/18	5-10	0-8": Dark brown, very fine to coarse SAND, some Silt, trace Gravel, damp.				
6				8-18": Light brown, very fine to coarse SAND, little Gravel, little Silt, trace Cobbles, little Silt, damp.				
7								
8								
9								
10	S-3	60/24	10-15	0-12": Brown/gray, very fine to coarse SAND, some Silt, trace Gravel, trace cobbles, damp.	Sorted Sand and Gravel			
11				12-24": Light Brown, medium to coarse SAND, some Gravel, trace fine Sand, trace Silt, damp. (sorted)				
12								
13								
14								
15	S-4	48/40	15-19	0-16": Brown, medium to coarse SAND, little Gravel, trace fine Sand, trace Silt, damp. (sorted)	Sand and Gravel with Silt			
16				16-40": Brown, very fine to coarse SAND, some Gravel, some Silt, trace cobbles, damp.				
17								
18								
19				Refusal encountered at 19 feet below ground surface.				
20	GRANULAR SOILS		COHESIVE SOILS		NOTES			
	Blows/ft.	Density	Blows/ft.	Consistency	Groundwater not encountered.			
	0-4	V. LOOSE	<2	V. SOFT				
	5-10	LOOSE	2-4	SOFT				
	11-30	M. DENSE	4-8	M. STIFF				
	31-50	DENSE	8-15	STIFF				
	>50	V. DENSE	15-30	V. STIFF				
			>30	HARD				

SOIL BORING LOG							
 <p>Client: Lombardo Associates, Inc.</p> <p>Project: Adams School Waste Water Investigation</p> <p>Location: Holliston, Massachusetts</p>	Boring Identification: B-5			Sheet: 1 of 1			
	Checked By: DGH			Project Number:			
	Drilling Company: Geosearch, Inc			Boring Location: See Site Plan			
	Foreman: Ed			Ground Surface Elevation: NA			Datum: NA
GeoInsight Engineer/Geologist: Chris Griffin			Date Started: 8/15/22			Date Completed: 8/15/22	
DRILLING METHOD		SAMPLER	GROUNDWATER MEASUREMENTS				
Vehicle: Track Geoprobe		Type: 5' acetate sleeve	Date	Depth (ft)	Reference	Stabilization	
Model: 7822DT		Hammer (lb): NA			Ground Surface		
Method: Direct Push		Fall (in): NA					
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
#	Pen/Rec (in)	Depth (ft)	Blows/6"				
0	S-1	60/24	0-5	0-6": Topsoil 6-24": Brown, very fine to coarse SAND, some Gravel, little Silt, trace cobbles, damp.			
1							
2							
3							
4							
5	S-2	60/24	5-10	0-24": Dark brown/brown, fine to coarse SAND, some Silt, little Gravel, trace cobbles, damp.			
6							
7							
8							
9							
10	S-3	60/18	10-15	0-18": Light brown, medium to coarse SAND, little Gravel, trace fine Sand, trace Silt, trace cobbles, damp. (sorted)			
11							
12							
13							
14							
15	S-4	24/20	15-17	0-6": Brown, medium to coarse SAND, little Gravel, trace fine Sand, trace Silt, trace cobbles, wet. (more sorted) 6-20": Brown, fine to coarse SAND and GRAVEL, little Silt, trace cobbles, wet.			
16							
17							
18							
19				Refusal encountered at 17 feet below ground surface.			
20	GRANULAR SOILS		COHESIVE SOILS	NOTES			
Blows/ft.	Density	Blows/ft.	Consistency	First attempt refusal encountered at 3 feet below ground surface. Groundwater not encountered.			
0-4	V. LOOSE	<2	V. SOFT				
5-10	LOOSE	2-4	SOFT				
11-30	M. DENSE	4-8	M. STIFF				
31-50	DENSE	8-15	STIFF				
>50	V. DENSE	15-30	V. STIFF				
		>30	HARD				



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: B-6					
Project: Adams School Waste Water Investigation				Sheet: 1 of 1					
Location: Holliston, Massachusetts				Checked By: DGH					
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan					
Foreman: Ed				Ground Surface Elevation: NA					
GeoInsight Engineer/Geologist: Chris Griffin				Datum: NA					
Date Started: 8/15/22				Date Completed: 8/15/22					
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS					
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization		
Model: 7822DT		Hammer (lb): NA				Ground Surface			
Method: Direct Push		Fall (in): NA							
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)		
#	Pen/Rec (in)	Depth (ft)	Blows/6"			Sand and Gravel with some Silt			
0	S-1	60/18	0-5	0-6": Topsoil					
				12-18": Brown, very fine to coarse SAND, some Silt, little Gravel, trace cobbles, damp.					
1									
2									
3									
4									
5	S-2	60/18	5-10	0-18": Brown, fine to coarse SAND, some Gravel, little Silt, trace cobbles, damp.					
6									
7									
8									
9									
10	S-3	60/24	10-15	0-24": Gray, fine to coarse SAND, little Gravel, little Silt, trace cobbles, wet at 12 feet below ground surface.		Sand and Gravel with little Silt			
11									
12									
13									
14									
15	S-4	54/30	15-19.5	0-30": Gray, fine to coarse SAND, little Gravel, little Silt, trace cobbles, wet.					
16									
17									
18									
19									
20	GRANULAR SOILS		COHESIVE SOILS		NOTES				
	Blows/ft.	Density	Blows/ft.	Consistency	First attempt refusal encountered at 6 feet below ground surface. Concrete drain encountered at 6 feet below ground surface. Groundwater encountered at 12 feet below ground surface. Septic odor observed.				
	0-4	V. LOOSE	<2	V. SOFT					
	5-10	LOOSE	2-4	SOFT					
	11-30	M. DENSE	4-8	M. STIFF					
	31-50	DENSE	8-15	STIFF					
	>50	V. DENSE	15-30	V. STIFF					
			>30	HARD					



SOIL BORING LOG

Client: Lombardo Associates, Inc. Boring Identification: B-7 Project: Adams School Waste Water Investigation Sheet: 1 of 1 Location: Holliston, Massachusetts Checked By: DGH Project Number:							
Drilling Company: Geosearch, Inc.		Boring Location: See Site Plan					
Foreman: Ed		Ground Surface Elevation: NA Datum: NA					
GeoInsight Engineer/Geologist: Chris Griffin		Date Started: 8/15/22 Date Completed: 8/15/22					
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS			
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization
Model: 7822DT		Hammer (lb): NA				Ground Surface	
Method: Direct Push		Fall (in): NA					
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
	#	Pen/Rec (in)	Depth (ft)				
0				No samples collected.			
1							
2							
3							
4							
5							
6							
7							
8							
9							
10							
11							
12							
13							
14							
15							
16							
17				Refusal encountered at 17 feet below ground surface.			
18							
19							
20	GRANULAR SOILS		COHESIVE SOILS		NOTES		
	Blows/ft.	Density	Blows/ft.	Consistency	Groundwater was not measured. Augers used.		
	0-4	V. LOOSE	<2	V. SOFT			
	5-10	LOOSE	2-4	SOFT			
	11-30	M. DENSE	4-8	M. STIFF			
	31-50	DENSE	8-15	STIFF			
	>50	V. DENSE	15-30	V. STIFF			
			>30	HARD			



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: B-8				
Project: Adams School Waste Water Investigation				Sheet: 1 of 1				
Location: Holliston, Massachusetts				Checked By: DGH Project Number:				
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan				
Foreman: Ed				Ground Surface Elevation: NA Datum: NA				
GeoInsight Engineer/Geologist: Chris Griffin				Date Started: 8/16/22 Date Completed: 8/16/22				
				GROUNDWATER MEASUREMENTS				
DRILLING METHOD		SAMPLER		Date	Depth (ft)	Reference	Stabilization	
Vehicle: Track Geoprobe		Type: 5' acetate sleeve				Ground Surface		
Model: 7822DT		Hammer (lb): NA						
Method: Direct Push		Fall (in): NA						
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE	
	#	Pen/Rec (in)	Depth (ft)					Blows/6"
0	S-1	60/24	0-5	0-6": Topsoil 12-24": Brown, very fine to coarse SAND, little Gravel, some Silt, trace cobbles, damp.	Sand and Gravel with Silt Fine Sand and Silt Sand and Gravel with Silt Cobble			
1								
2								
3								
4								
5	S-2	60/42	5-10	0-24": Brown/orange, very fine to fine SAND, some Silt, trace roots, damp.				
6								
7				24-42": Light brown, very fine to coarse SAND, little Gravel, little Silt, damp.				
8								
9								
10	S-3	60/26	10-15	0-12": Brown, fine to coarse SAND, little Gravel, trace Silt, trace Cobbles, damp. 12-24": Brown, medium to coarse SAND, little Gravel, trace Silt, trace Cobbles, damp. 24-26": Gray, fine SAND, rock.				
11								
12								
13				Refusal encountered at 13 feet below ground surface.				
14								
15								
16								
17								
18								
19								
20								
GRANULAR SOILS			COHESIVE SOILS		NOTES			
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately 12 feet below ground surface.				
0-4	V. LOOSE	<2	V. SOFT					
5-10	LOOSE	2-4	SOFT					
11-30	M. DENSE	4-8	M. STIFF					
31-50	DENSE	8-15	STIFF					
>50	V. DENSE	15-30	V. STIFF					
		>30	HARD					

SOIL BORING LOG								
		Client: Lombardo Associates, Inc.			Boring Identification: B-9			
		Project: Adams School Waste Water Investigation						Sheet: 1 of 1
		Location: Holliston, Massachusetts			Checked By: DGH			Project Number:
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan				
Foreman: Ed				Ground Surface Elevation: NA Datum: NA				
GeoInsight Engineer/Geologist: Chris Griffin				Date Started: 8/16/22 Date Completed: 8/16/22				
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization	
Model: 7822DT		Hammer (lb): NA				Ground Surface		
Method: Direct Push		Fall (in): NA						
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE	
	#	Pen/Rec (in)	Depth (ft)					Blows/6"
0	S-1	60/36	0-5	0-6": Topsoil 6-12": Light brown, very fine to coarse SAND, little Gravel, little Silt, damp. 12-24": Light brown, very fine to coarse SAND, some Gravel, little Silt, trace Cobbles, damp. 24-36": Brown/orange, very fine to coarse SAND, some Gravel, trace Silt.	Sand and Gravel with Silt			
1								
2								
3								
4								
5	S-2	60/24	5-10	0-18": Brown, fine to coarse SAND, some Gravel, trace cobbles, trace Silt, damp. 18-24": Brown/orange medium to coarse SAND, trace Gravel, trace fine Sand, trace Silt, (sorted) damp.				
6								
7								
8								
9								
10	S-3	60/30	10-15	0-24": Light brown, medium to coarse SAND, little Gravel, trace Silt, (sorted) damp. 24-30": Gray, fine to medium SAND, some Silt, little Gravel, trace cobbles, wet.	Sorted Sand and Gravel			
11								
12								
13								
14								
15	S-4	54/18	15-17.5	0-18": Gray, fine to coarse SAND, some Gravel, some Silt, trace cobbles, wet.	Sand and Gravel with Silt			
16								
17								
18				Refusal encountered at 17.5 feet below ground surface.				
19								
20								
GRANULAR SOILS			COHESIVE SOILS		NOTES			
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately 13 feet below ground surface.				
0-4	V. LOOSE	<2	V. SOFT					
5-10	LOOSE	2-4	SOFT					
11-30	M. DENSE	4-8	M. STIFF					
31-50	DENSE	8-15	STIFF					
>50	V. DENSE	15-30	V. STIFF					
		>30	HARD					

SOIL BORING LOG												
 <p>Client: Lombardo Associates, Inc.</p> <p>Project: Adams School Waste Water Investigation</p> <p>Location: Holliston, Massachusetts</p>	Boring Identification: B-10			Sheet: 1 of 2								
	Checked By: DGH			Project Number:								
	Drilling Company: Geosearch, Inc			Boring Location: See Site Plan								
	Foreman: Ed			Ground Surface Elevation: NA			Datum: NA					
GeoInsight Engineer/Geologist: Chris Griffin			Date Started: 8/16/22			Date Completed: 8/16/22						
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS								
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization					
Model: 7822DT		Hammer (lb): NA				Ground Surface						
Method: Direct Push		Fall (in): NA										
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)					
#	Pen/Rec (in)	Depth (ft)	Blows/6"			Sand and Gravel with Silt	NOTE					
0	S-1	60/42	0-5	0-6": Topsoil								
1				6-12": Gray, very fine to coarse SAND, some Gravel, little Silt, damp.								
2				12-42": Brown, very fine to coarse SAND, little Gravel, little Silt, damp.								
3												
4												
5	S-2	60/10	5-10	0-10": Brown, very fine to coarse SAND, some Gravel, some Silt, trace cobbles, damp.								
6												
7												
8												
9												
10	S-3	60/8	10-15	0-8": Brown, fine to coarse SAND and GRAVEL, some Silt, trace cobbles, damp.								
11												
12												
13												
14												
15	S-4	54/24	15-24	0-8": Brown/gray, medium to coarse SAND, little Silt, trace cobbles, wet.		Sand and Gravel	NOTE					
16				8-24": Brown/gray fine to coarse SAND and GRAVEL, little Silt, trace cobbles, wet.								
17												
18												
19												
20	GRANULAR SOILS		COHESIVE SOILS		NOTES							
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately at 15 feet below ground surface. Low recovery in 10-15 sleeve groundwater could be as high as 13 feet.								
0-4	V. LOOSE	<2	V. SOFT									
5-10	LOOSE	2-4	SOFT									
11-30	M. DENSE	4-8	M. STIFF									
31-50	DENSE	8-15	STIFF									
>50	V. DENSE	15-30	V. STIFF									
		>30	HARD									

SOIL BORING LOG											
		Client: Lombardo Associates, Inc.		Boring Identification: B-10							
Project: Adams School Waste Water Investigation				Sheet: 2 of 2							
Location: Holliston, Massachusetts				Checked By: DGH							
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan							
Foreman: Ed				Ground Surface Elevation: NA							
GeoInsight Engineer/Geologist: Chris Griffin				Datum: NA							
Date Started: 8/16/22				Date Completed: 8/16/22							
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS							
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization				
Model: 7822DT		Hammer (lb): NA				Ground Surface					
Method: Direct Push		Fall (in): NA									
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)				
#	Pen/Rec (in)	Depth (ft)	Blows/6"								
20	S-1	60/42	20-25	0-32": Brown medium to coarse SAND, little Gravel, trace Silt, wet.		Sand and Gravel					
21											
22						Sand and Gravel with Silt					
23				32-42": Gray, fine SAND, some Silt, trace cobbles, wet.							
24				Refusal encountered at 24 feet below ground surface.							
25											
26											
27											
28											
29											
30											
31											
32											
33											
34											
35											
36											
37											
38											
39											
40	GRANULAR SOILS		COHESIVE SOILS		NOTES						
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately at 15 feet below ground surface. Low recovery in 10-15 sleeve groundwater could be as high as 13 feet.							
0-4	V. LOOSE	<2	V. SOFT								
5-10	LOOSE	2-4	SOFT								
11-30	M. DENSE	4-8	M. STIFF								
31-50	DENSE	8-15	STIFF								
>50	V. DENSE	15-30	V. STIFF								
		>30	HARD								



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: B-11								
Project: Adams School Waste Water Investigation				Sheet: 1 of 1								
Location: Holliston, Massachusetts				Checked By: DGH								
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan								
Foreman: Ed				Ground Surface Elevation: NA								
GeoInsight Engineer/Geologist: Chris Griffin				Datum: NA								
Date Started: 8/16/22				Date Completed: 8/16/22								
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS								
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization					
Model: 7822DT		Hammer (lb): NA				Ground Surface						
Method: Direct Push		Fall (in): NA										
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)					
#	Pen/Rec (in)	Depth (ft)	Blows/6"			Sand and Gravel with Silt						
0	S-1	60/24	0-5	0-6": Topsoil								
				6-24": Light brown, very fine to coarse SAND, some Gravel, some Silt, trace cobbles, damp.								
1												
2												
3												
4												
5	S-2	60/32	5-10	0-32": Light brown, very fine to coarse SAND, some Silt, little Gravel, trace cobbles, damp.								
6												
7												
8												
9												
10	S-3	60/30	10-15	0-30": Light brown, very fine to coarse SAND, some Gravel, little Silt, trace cobbles, damp.								
11												
12												
13				Refusal encountered at 13 feet below ground surface.								
14												
15												
16												
17												
18												
19												
20	GRANULAR SOILS		COHESIVE SOILS		NOTES							
Blows/ft.	Density	Blows/ft.	Consistency	First attempt refusal encountered at 4 feet below ground surface. Second attempt refusal encountered at 2 feet below ground surface. Groundwater not encountered.								
0-4	V. LOOSE	<2	V. SOFT									
5-10	LOOSE	2-4	SOFT									
11-30	M. DENSE	4-8	M. STIFF									
31-50	DENSE	8-15	STIFF									
>50	V. DENSE	15-30	V. STIFF									
		>30	HARD									



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: B-12				
Project: Adams School Waste Water Investigation				Sheet: 1 of 2				
Location: Holliston, Massachusetts				Checked By: DGH Project Number:				
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan				
Foreman: Ed				Ground Surface Elevation: NA Datum: NA				
GeoInsight Engineer/Geologist: Chris Griffin				Date Started: 8/16/22 Date Completed: 8/16/22				
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization	
Model: 7822DT		Hammer (lb): NA				Ground Surface		
Method: Direct Push		Fall (in): NA						
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
	#	Pen/Rec (in)	Depth (ft)	Blows/6"				
0	S-1	60/48	0-5	0-48": Light brown, very fine SAND, some Silt, damp.		Fine Sand and Silt		
1								
2								
3								
4								
5	S-2	60/50	5-10	0-50": Light brown, very fine SAND, some Silt, damp.				
6								
7								
8								
9								
10	S-3	60/50	10-15	0-50": Light brown, very fine SAND, some Silt, damp.				
11								
12								
13								
14								
15	S-4	60/48	15-20	0-24": Brown, very fine SAND and SILT, damp.				
16								
17				24-48": Brown, very fine SAND and SILT, wet.				
18								
19								
20								
GRANULAR SOILS			COHESIVE SOILS		NOTES			
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately 17 feet below ground surface.				
0-4	V. LOOSE	<2	V. SOFT					
5-10	LOOSE	2-4	SOFT					
11-30	M. DENSE	4-8	M. STIFF					
31-50	DENSE	8-15	STIFF					
>50	V. DENSE	15-30	V. STIFF					
		>30	HARD					

SOIL BORING LOG									
 <p>Client: Lombardo Associates, Inc.</p> <p>Project: Adams School Waste Water Investigation</p> <p>Location: Holliston, Massachusetts</p>	Boring Identification: B-12		Sheet: 2 of 2		Project Number:				
	Drilling Company: Geosearch, Inc		Boring Location: See Site Plan						
	Foreman: Ed		Ground Surface Elevation: NA		Datum: NA				
	GeoInsight Engineer/Geologist: Chris Griffin		Date Started: 8/16/22		Date Completed: 8/16/22				
DRILLING METHOD		SAMPLER	GROUNDWATER MEASUREMENTS						
Vehicle: Track Geoprobe		Type: 5' acetate sleeve	Date	Depth (ft)	Reference	Stabilization			
Model: 7822DT		Hammer (lb): NA			Ground Surface				
Method: Direct Push		Fall (in): NA							
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)		
#	Pen/Rec (in)	Depth (ft)	Blows/6"			Fine Sand and Silt			
20	S-1	60/50	20-25	0-50": Brown, very fine SAND and SILT, wet.					
21									
22									
23									
24									
25				Boring terminated at 25 feet below ground surface. Refusal not encountered.					
26									
27									
28									
29									
30									
31									
32									
33									
34									
35									
36									
37									
38									
39									
40	GRANULAR SOILS		COHESIVE SOILS	NOTES					
	Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately 17 feet below ground surface.				
	0-4	V. LOOSE	<2	V. SOFT					
	5-10	LOOSE	2-4	SOFT					
	11-30	M. DENSE	4-8	M. STIFF					
	31-50	DENSE	8-15	STIFF					
	>50	V. DENSE	15-30	V. STIFF					
			>30	HARD					



SOIL BORING LOG

Client: Lombardo Associates, Inc. Boring Identification: B-13 Project: Adams School Waste Water Investigation Sheet: 1 of 2 Location: Holliston, Massachusetts Checked By: DGH Project Number:								
Drilling Company: Geosearch, Inc.		Boring Location: See Site Plan						
Foreman: Ed		Ground Surface Elevation: NA Datum: NA						
GeoInsight Engineer/Geologist: Chris Griffin		Date Started: 8/16/22 Date Completed: 8/16/22						
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization	
Model: 7822DT		Hammer (lb): NA				Ground Surface		
Method: Direct Push		Fall (in): NA						
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
	#	Pen/Rec (in)	Depth (ft)	Blows/6"				
0	S-1	60/36	0-5	0-36": Light brown, very fine SAND, some Silt, damp.		Fine Sand and Silt		
1								
2								
3								
4								
5	S-2	60/48	5-10	0-48": Brown/gray, fine SAND, some Silt, wet.				
6								
7								
8								
9								
10	S-3	60/50	10-15	0-50": Brown/orange, fine SAND, some Silt, wet.				
11								
12								
13								
14								
15	S-4	60/24	15-20	0-24": Gray, very fine SAND and SILT, wet.				
16								
17								
18								
19								
20								
GRANULAR SOILS			COHESIVE SOILS		NOTES			
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately 7 feet below ground surface.				
0-4	V. LOOSE	<2	V. SOFT					
5-10	LOOSE	2-4	SOFT					
11-30	M. DENSE	4-8	M. STIFF					
31-50	DENSE	8-15	STIFF					
>50	V. DENSE	15-30	V. STIFF					
		>30	HARD					



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: B-13								
Project: Adams School Waste Water Investigation				Sheet: 2 of 2								
Location: Holliston, Massachusetts				Checked By: DGH								
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan								
Foreman: Ed				Ground Surface Elevation: NA								
GeoInsight Engineer/Geologist: Chris Griffin				Datum: NA								
Date Started: 8/16/22				Date Completed: 8/16/22								
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS								
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization					
Model: 7822DT		Hammer (lb): NA				Ground Surface						
Method: Direct Push		Fall (in): NA										
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)					
#	Pen/Rec (in)	Depth (ft)	Blows/6"			Sand and Silt						
20	S-1	60/48	20-25	0-20": Brown, very fine SAND and SILT, wet.								
21				20-48": Brown/orange, fine to coarse SAND, some Silt, little Gravel, trace cobbles, wet.			Sand and Gravel with Silt					
22												
23												
24												
25				Boring terminated at 25 feet below ground surface. Refusal not encountered.								
26												
27												
28												
29												
30												
31												
32												
33												
34												
35												
36												
37												
38												
39												
40	GRANULAR SOILS		COHESIVE SOILS		NOTES							
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately 7 feet below ground surface.								
0-4	V. LOOSE	<2	V. SOFT									
5-10	LOOSE	2-4	SOFT									
11-30	M. DENSE	4-8	M. STIFF									
31-50	DENSE	8-15	STIFF									
>50	V. DENSE	15-30	V. STIFF									
		>30	HARD									



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: B-14							
Project: Adams School Waste Water Investigation				Sheet: 1 of 1							
Location: Holliston, Massachusetts				Checked By: DGH							
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan							
Foreman: Ed				Ground Surface Elevation: NA							
GeoInsight Engineer/Geologist: Chris Griffin				Datum: NA							
Date Started: 8/16/22				Date Completed: 8/16/22							
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS							
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization				
Model: 7822DT		Hammer (lb): NA				Ground Surface					
Method: Direct Push		Fall (in): NA									
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)				
#	Pen/Rec (in)	Depth (ft)	Blows/6"								
0	S-1	60/24	0-5	0-24": Light brown, fine to coarse SAND, trace Silt, trace Gravel, damp.		SAND					
1											
2											
3											
4											
5	S-2	60/24	5-10	0-24": Light brown, fine to coarse SAND, trace Gravel, little Silt, damp.							
6											
7											
8											
9											
10	S-3	60/12	10-15	0-12": Brown, fine SAND, some Silt, wet.							
11											
12											
13											
14											
15	S-4	60/24	15-20	0-24": Brown medium to coarse SAND, trace Gravel, trace Silt, wet.							
16											
17											
18											
19				Boring terminated at 20 feet below ground surface refusal not encountered.							
20	GRANULAR SOILS		COHESIVE SOILS		NOTES						
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately 15 feet below ground surface.							
0-4	V. LOOSE	<2	V. SOFT								
5-10	LOOSE	2-4	SOFT								
11-30	M. DENSE	4-8	M. STIFF								
31-50	DENSE	8-15	STIFF								
>50	V. DENSE	15-30	V. STIFF								
		>30	HARD								



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: B-15							
Project: Adams School Waste Water Investigation				Sheet: 1 of 1							
Location: Holliston, Massachusetts				Checked By: DGH							
Drilling Company: Geosearch, Inc				Boring Location: See Site Plan							
Foreman: Ed				Ground Surface Elevation: NA							
GeoInsight Engineer/Geologist: Chris Griffin				Datum: NA							
Date Started: 8/16/22				Date Completed: 8/16/22							
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS							
Vehicle: Track Geoprobe		Type: 5' acetate sleeve		Date	Depth (ft)	Reference	Stabilization				
Model: 7822DT		Hammer (lb): NA				Ground Surface					
Method: Direct Push		Fall (in): NA									
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)				
#	Pen/Rec (in)	Depth (ft)	Blows/6"								
0	S-1	60/30	0-5	0-6": Top Soil. 6-30": Light Brown, fine to coarse SAND, little Gravel, trace Silt, damp.		SAND					
1											
2											
3											
4											
5	S-2	60/32	5-10	0-32": Brown, fine to coarse SAND, little Gravel, trace Silt, damp.							
6											
7											
8											
9											
10	S-3	60/24	10-15	0-24": Brown, fine to coarse SAND, some Gravel, little Silt, wet.		Sand and Gravel					
11											
12											
13											
14											
15				Boring terminated at 15 feet below ground surface, refusal not encountered.							
16											
17											
18											
19											
20	GRANULAR SOILS		COHESIVE SOILS		NOTES						
Blows/ft.	Density	Blows/ft.	Consistency	Groundwater encountered at approximately 14 feet below ground surface.							
0-4	V. LOOSE	<2	V. SOFT								
5-10	LOOSE	2-4	SOFT								
11-30	M. DENSE	4-8	M. STIFF								
31-50	DENSE	8-15	STIFF								
>50	V. DENSE	15-30	V. STIFF								
		>30	HARD								

SOIL BORING / WELL COMPLETION LOG



Client: Lombardo Associates, Inc.	Boring Identification: SB- 101	Well ID: MW-1
Project: Adams School Waste Water Investigation		Sheet: 1 of 1
Location: Holliston, Massachusetts	Chkd. By: DGH	Project Number: 15494

Drilling Company: Geosearch, Inc	Boring Location:
Foreman: Mike	Top of PVC Riser Elevation:
GeoInsight Engineer/Geologist: Chris Griffin	Top of Protector Elevation:
	Date Started: 11/28/22

DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS						
Vehicle: Truck		Type: Split-Spoon		Date	Depth (ft)	Reference	Stabilization			
Model: CME 75		Hammer (lb): 140		11/30/2022	21.38	Ground Surface				
Method: Hollow Stem Auger		Fall (in): 30								
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)		
#	Pen/Rec (in)	Depth (ft)	Blows/6"				NOTE			
0				5-7': M. Dense, brown, fine to coarse SAND, little Gravel, little Silt, damp. 10-12': Loose, light brown/orange, fine to medium SAND, little Silt, trace roots, damp. 15-17': M. Dense, brown, fine to coarse SAND, trace Gravel, trace Silt, damp.						
1										
2										
3										
4										
5	S-1	24/6	5-7							
			6							
6			7							
7			13							
8										
9										
10	S-2	24/18	10-12	10-12': Loose, light brown/orange, fine to medium SAND, little Silt, trace roots, damp.						
			1							
11			2							
12			3							
13			4							
14										
15	S-3	24/10	15-17	15-17': M. Dense, brown, fine to coarse SAND, trace Gravel, trace Silt, damp.						
			7							
16			8							
17			5							
18										
19										
20	GRANULAR SOILS		COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND		
	Blows/ft.	Density	Blows/ft.	Consistency						
	0-4	V. LOOSE	<2	V. SOFT		Concrete	0-0.5			
	5-10	LOOSE	2-4	SOFT		Backfill	0.5-11			
	11-30	M. DENSE	5-8	M. STIFF		Grout	NA			
	31-50	DENSE	9-15	STIFF		Bentonite: Chips	11-12			
	>50	V. DENSE	16-30	V. STIFF		Sandpack: # 2 Sand	12-24			
			>30	HARD		Riser	0-14			
						Screen	14-24			

NOTES

SOIL BORING / WELL COMPLETION LOG



Client: Lombardo Associates, Inc.				Boring Identification: SB- 101		Well ID: MW-1		
Project: Adams School Waste Water Investigation						Sheet: 1 of 2		
Location: Holliston, Massachusetts				Chkd. By: DGH		Project Number: 15494		
Drilling Company: Geosearch, Inc				Boring Location:				
Foreman: Mike				Top of PVC Riser Elevation:				
GeoInsight Engineer/Geologist: Chris Griffin				Datum:				
				Ground Elevation:				
				Date Started: 11/28/22 Date Completed: 11/28/22				
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Truck		Type: Split-Spoon		Date	Depth (ft)	Reference	Stabilization	
Model: CME 75		Hammer (lb): 140		11/30/2022	21.38	Ground Surface		
Method: Hollow Stem Auger		Fall (in): 30						
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
	#	Pen/Rec (in)	Depth (ft)					
20	S-4	19/4	20-22	4	20-22': Refusal, gray/brown, fine to coarse SAND, little Silt, little Gravel, rock in tip, wet.			
21				13				
22				35				
23				50+				
24					Refusal encountered with augers at 24 feet bgs.			
25								
26								
27								
28								
29								
30								
31								
32								
33								
34								
35	S-3	24/10	15-17	7				
36				7				
37				8				
38				5				
39								
40	GRANULAR SOILS		COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND
Blows/ft.	Density	Blows/ft.	Consistency					
0-4	V. LOOSE	<2	V. SOFT					
5-10	LOOSE	2-4	SOFT					
11-30	M. DENSE	5-8	M. STIFF					
31-50	DENSE	9-15	STIFF					
>50	V. DENSE	16-30	V. STIFF					
		>30	HARD					
NOTES								

SOIL BORING / WELL COMPLETION LOG



Client: Lombardo Associates, Inc. **Boring Identification:** SB- 101 **Well ID:** MW-1
Project: Adams School Waste Water Investigation **Sheet:** 1 of 1
Location: 1101 Adams Street, Adams, MA 01222 **Project Number:** 15101

Location: Holliston, Massachusetts	Chkd. By: DGH	Project Number: 15494
Drilling Company: Geosearch, Inc	Boring Location:	
Foreman: Mike	Top of PVC Riser Elevation:	Datum:
GeoInsight Engineer/Geologist: Chris Griffin	Top of Protector Elevation:	Ground Elevation:
	Date Started: 11/29/22	Date Completed: 11/29/22

DRILLING METHOD			SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Truck			Type: Split-Spoon	Date	Depth (ft)	Reference	Stabilization		
Model: CME 75			Hammer (lb): 140	11/30/2022	11.11	Ground Surface			
Method: Hollow Stem Auger			Fall (in): 30						
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
#	Pen/Rec (in)	Depth (ft)	Blows/6"						
0									
1									
2									
3									
4									
5	S-1	24/7	5-7	2	5-7': Loose, brown, fine to medium SAND, trace Silt, damp.				
				3					
				5					
				5					
6									
7									
8									
9									
10	S-2	24/14	10-12	6	10-12': M. Dense, brown, fine to coarse SAND, little Silt, wet.				
				13					
				13					
				16					
11									
12									
13									
14									
15	S-3	24/10	15-17	1	15-17': Loose, brown, fine to coarse SAND, trace Silt, trace Gravel, wet.				
				3					
				3					
				7					
16									
17									
18					Boring terminated at 18 feet bgs, refusal not encountered.				
19									
20	GRANULAR SOILS		COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND	
Blows/ft.	Density	Blows/ft.	Consistency						
0-4	V. LOOSE	<2	V. SOFT		Concrete	0-0.5			
5-10	LOOSE	2-4	SOFT		Backfill	0.5-4			
11-30	M. DENSE	5-8	M. STIFF		Grout	NA			
31-50	DENSE	9-15	STIFF		Bentonite: Chips	4-5			
>50	V. DENSE	16-30	V. STIFF		Sandpack: # 2 Sand	4-15			
		>30	HARD		Riser	0-8			
					Screen	8-18			

NOTES

SOIL BORING / WELL COMPLETION LOG



Client: Lombardo Associates, Inc.	Boring Identification: SB- 103	Well ID: MW-3
Project: Adams School Waste Water Investigation		Sheet: 1 of 1
Location: Holliston, Massachusetts	Chkd. By: DGH	Project Number: 15494

Drilling Company: Geosearch, Inc	Boring Location:
Foreman: Mike	Top of PVC Riser Elevation:
GeoInsight Engineer/Geologist: Chris Griffin	Top of Protector Elevation:
	Date Started: 11/29/22

DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Truck		Type: Split-Spoon		Date	Depth (ft)	Reference	Stabilization	
Model: CME 75		Hammer (lb): 140		0.5-12	11.20	Ground Surface		
Method: Hollow Stem Auger		Fall (in): 30						
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)
#	Pen/Rec (in)	Depth (ft)	Blows/6"					NOTE
0								
1								
2								
3								
4								
5	S-1	24/4	5-7	9	5-7': M. Dense, light brown/gray, fine to coarse SAND, little Gravel, little Silt, damp.			
				10				
				10				
				10				
10	S-2	24/3	10-12	10	10-12': M. Dense, light brown, fine to coarse SAND, little Gravel, trace trace, damp.			
				11				
				14				
				13				
15	S-3	24/10	15-17	6	15-17': M. Dense, gray, fine SAND, little Silt, trace Gravel, wet.			
				12				
				10				
				10				
20	GRANULAR SOILS		COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND
	Blows/ft.	Density	Blows/ft.	Consistency	Boring terminated at 20 feet bgs, refusal not encountered.	Concrete	0-0.5	
	0-4	V. LOOSE	<2	V. SOFT		Backfill	0.5-8	
	5-10	LOOSE	2-4	SOFT		Grout	NA	
	11-30	M. DENSE	5-8	M. STIFF		Bentonite: Chips	8-9	
	31-50	DENSE	9-15	STIFF		Sandpack: # 2 Sand	9-10	
	>50	V. DENSE	16-30	V. STIFF		Riser	0-10	
			>30	HARD		Screen	10-20	

NOTES

SOIL BORING / WELL COMPLETION LOG



Client: Lombardo Associates, Inc.	Boring Identification: SB- 105	Well ID: MW-5
Project: Adams School Waste Water Investigation		Sheet: 1 of 1
Location: Holliston, Massachusetts	Chkd. By: DGH	Project Number: 15494

Drilling Company: Geosearch, Inc	Boring Location:
Foreman: Mike	Top of PVC Riser Elevation:
GeoInsight Engineer/Geologist: Chris Griffin	Top of Protector Elevation:
	Date Started: 11/29/22

DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Truck		Type: Split-Spoon		Date	Depth (ft)	Reference	Stabilization	
Model: CME 75		Hammer (lb): 140		11/30/2022	7.71	Ground Surface		
Method: Hollow Stem Auger		Fall (in): 30						
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)
#	Pen/Rec (in)	Depth (ft)	Blows/6"					NOTE
0								
1								
2								
3								
4								
5	S-1	24/10	5-7	8				
				33				
				37				
				50+				
6								
7								
8								
9								
10	S-2	24/14	10-12	24				
				35				
				36				
				27				
11								
12								
13								
14								
15	S-3	24/10	15-17	20				
				17				
				17				
				14				
16								
17								
18								
19								
20								
GRANULAR SOILS			COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND
Blows/ft.	Density	Blows/ft.	Consistency					
0-4	V. LOOSE	<2	V. SOFT			Concrete	0-0.5	
5-10	LOOSE	2-4	SOFT			Backfill	0.5-6	
11-30	M. DENSE	5-8	M. STIFF			Grout	NA	
31-50	DENSE	9-15	STIFF			Bentonite: Chips	6-7	
>50	V. DENSE	16-30	V. STIFF			Sandpack: # 2 Sand	7-18	
		>30	HARD			Riser	0-8	
						Screen	8-18	

NOTES



Client: Lombardo Associates, Inc.				Boring Identification: SB- 106				Well ID: MW-6		
Project: Adams School Waste Water Investigation								Sheet: 1 of 1		
Location: Holliston, Massachusetts				Chkd. By: DGH				Project Number: 15494		
Drilling Company: Geosearch, Inc				Boring Location:						
Foreman: Mike				Top of PVC Riser Elevation:				Datum:		
GeoInsight Engineer/Geologist: Chris Griffin				Top of Protector Elevation:				Ground Elevation:		
				Date Started: 11/28/22				Date Completed: 11/28/22		
DRILLING METHOD			SAMPLER	GROUNDWATER MEASUREMENTS						
Vehicle: Truck		Type: Split-Spoon	Date	Depth (ft)	Reference	Stabilization				
Model: CME 75		Hammer (lb): 140	11/30/2022	Dry	Ground Surface					
Method: Hollow Stem Auger		Fall (in): 30								
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE	
	#	Pen/Rec (in)	Depth (ft)		Blows/6"	Date				Depth (ft)
0										
1										
2										
3										
4										
5	S-1	24/0	5-7	7						
				10						
6				6						
7				5						
8										
9										
10	S-2	24/6	10-12	7						
				9						
11				12						
12				17						
13										
14										
15										
16										
17										
18										
19										
20										
GRANULAR SOILS			COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND		
Blows/ft.	Density	Blows/ft.	Consistency							
0-4	V. LOOSE	<2	V. SOFT					Concrete	0-0.5	
5-10	LOOSE	2-4	SOFT					Backfill	0.5-1.5	
11-30	M. DENSE	5-8	M. STIFF					Grout	NA	
31-50	DENSE	9-15	STIFF					Bentonite: Chips	1.5-2.5	
>50	V. DENSE	16-30	V. STIFF					Sandpack: # 2 Sand	2.5-13.5	
		>30	HARD					Riser	0-3.5	
					Screen	3.5-13.5				
NOTES										

SOIL BORING / WELL COMPLETION LOG



Client: Lombardo Associates, Inc.				Boring Identification: SB- 107				Well ID: MW-7			
Project: Adams School Waste Water Investigation								Sheet: 1 of 1			
Location: Holliston, Massachusetts				Chkd. By: DGH				Project Number: 15494			
Drilling Company: Geosearch, Inc				Boring Location:							
Foreman: Mike				Top of PVC Riser Elevation:							
GeoInsight Engineer/Geologist: Chris Griffin				Datum:							
				Top of Protector Elevation:							
				Ground Elevation:							
				Date Started: 11/30/22							
				Date Completed: 11/30/22							
DRILLING METHOD			SAMPLER		GROUNDWATER MEASUREMENTS						
Vehicle: Truck			Type: Split-Spoon	Date	Depth (ft)	Reference	Stabilization				
Model: CME 75			Hammer (lb): 140	11/30/2022	14.49	Ground Surface					
Method: Hollow Stem Auger			Fall (in): 30								
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION		STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE		
	#	Pen/Rec (in)	Depth (ft)							Blows/6"	
0					Pre-cleared wth vactor to 8 feet bgs.						
1											
2											
3											
4											
5											
6											
7											
8											
9											
10	S-1	24/8	10-12	3	5-7': Loose, light brown, fine to medium SAND, trace Silt, damp.						
				4							
11				4							
12				4							
13											
14											
15	S-2	24/12	15-17	3	15-17': Loose, light brown, fine to medium SAND, trace Silt, wet.						
				4							
16				3							
17				3							
18											
19											
20	GRANULAR SOILS			COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND		
	Blows/ft.	Density	Blows/ft.	Consistency		Concrete	0-0.5				
	0-4	V. LOOSE	<2	V. SOFT		Backfill	0.5-9				
	5-10	LOOSE	2-4	SOFT		Grout	NA				
	11-30	M. DENSE	5-8	M. STIFF		Bentonite: Chips	9-10				
	31-50	DENSE	9-15	STIFF		Sandpack: # 2 Sand	10-22				
	>50	V. DENSE	16-30	V. STIFF		Riser	0-12				
			>30	HARD		Screen	12-22				
NOTES											

SOIL BORING / WELL COMPLETION LOG								
 Client: Lombardo Associates, Inc. Boring Identification: SB- 107 Well ID: MW-7 Project: Adams School Waste Water Investigation Sheet: 1 of 1 Location: Holliston, Massachusetts Chkd. By: DGH Project Number: 15494								
Drilling Company: Geosearch, Inc		Boring Location:						
Foreman: Mike		Top of PVC Riser Elevation: Datum:						
GeoInsight Engineer/Geologist: Chris Griffin		Top of Protector Elevation: Ground Elevation:						
		Date Started: 11/30/22 Date Completed: 11/30/22						
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Truck		Type: Split-Spoon	Date	Depth (ft)	Reference	Stabilization		
Model: CME 75		Hammer (lb): 140			Ground Surface			
Method: Hollow Stem Auger		Fall (in): 30						
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
	#	Pen/Rec (in)	Depth (ft)					
20	S-4	19/4	20-22	3	20-22': Loose, light brown, fine SAND, little Silt, wet.			
				5				
21				5				
				5				
22					Boring terminated at 22 feet bgs, refusal not encountered.			
23								
24								
25								
26								
27								
28								
29								
30								
31								
32								
33								
34								
35								
36								
37								
38								
39								
40								
GRANULAR SOILS		COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND	
Blows/ft.	Density	Blows/ft.	Consistency					
0-4	V. LOOSE	<2	V. SOFT		Concrete	0-0.5		
5-10	LOOSE	2-4	SOFT		Backfill	0.5-9		
11-30	M. DENSE	5-8	M. STIFF		Grout	NA		
31-50	DENSE	9-15	STIFF		Bentonite: Chips	9-10		
>50	V. DENSE	16-30	V. STIFF		Sandpack: # 2 Sand	10-22		
		>30	HARD		Riser	0-12		
					Screen	12-22		
NOTES								

SOIL BORING / WELL COMPLETION LOG								
 Client: Lombardo Associates, Inc.		Boring Identification: SB- 101		Well ID: MW-1				
Project: Adams School Waste Water Investigation				Sheet: 1 of 1				
Location: Holliston, Massachusetts				Chkd. By: DGH				
Drilling Company: Geosearch, Inc		Boring Location:						
Foreman: Mike		Top of PVC Riser Elevation:						
GeoInsight Engineer/Geologist: Chris Griffin		Datum: Top of Protector Elevation: Ground Elevation:						
		Date Started: 11/30/22 Date Completed: 11/30/22						
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS				
Vehicle: Truck		Type: Split-Spoon		Date	Depth (ft)	Reference		
Model: CME 75		Hammer (lb): 140		11/30/2022	3.87	Ground Surface		
Method: Hollow Stem Auger		Fall (in): 30						
DEPTH (ft)	SAMPLE INFORMATION			WELL COMPLETION DETAIL	SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
	#	Pen/Rec (in)	Depth (ft)					
0								
1								
2								
3								
4								
5	S-1	24/14	5-7	8				
				9				
6				13				
7				15				
8								
9								
10	S-1	24/8	10-12	9				
				8				
11				11				
12				9				
13								
14								
15								
16								
17								
18								
19								
20								
GRANULAR SOILS			COHESIVE SOILS		WELL CONSTRUCTION NOTES	WELL MATERIALS	INTERVAL (feet bgs)	LEGEND
Blows/ft.	Density	Blows/ft.	Consistency					
0-4	V. LOOSE	<2	V. SOFT		Concrete	0-0.5		
5-10	LOOSE	2-4	SOFT		Backfill	0.5-4		
11-30	M. DENSE	5-8	M. STIFF		Grout	NA		
31-50	DENSE	9-15	STIFF		Bentonite: Chips	4-5		
>50	V. DENSE	16-30	V. STIFF		Sandpack: # 2 Sand	4-15		
		>30	HARD		Riser	0-5		
					Screen	5-15		
NOTES								



SOIL BORING LOG

Client: Lombardo Associates, Inc.				Boring Identification: SB-104			
Project: Adams School Waste Water Investigation				Sheet: 1 of 1			
Location: Holliston, Massachusetts				Chkd. By: DGH Project Number:			
Drilling Company: Geosearch, Inc				Boring Location:			
Foreman: Mike				Ground Surface Elevation: Datum:			
GeoInsight Engineer/Geologist: Chris Griffin				Date Started: 11/28/22 Date Completed: 11/28/22			
DRILLING METHOD		SAMPLER		GROUNDWATER MEASUREMENTS			
Vehicle: Truck		Type: Split-Spoon		Date	Depth (ft)	Reference	Stabilization
Model: CME 75		Hammer (lb): 140				Ground Surface	
Method: Hollow Stem Auger		Fall (in): 30					
DEPTH (ft)	SAMPLE INFORMATION			SAMPLE DESCRIPTION	STRATUM DESCRIPTION	FIELD SCREENING (ppm)	NOTE
	#	Pen/Rec (in)	Depth (ft)				
0							
1							
2							
3							
4							
5	S-1	24/10	5-7	7	5-7': M. Dense, brown, fine to coarse SAND, little Gravel, trace Silt, damp.		
				12			
				17			
				26			
6							
7							
8							
9							
10	S-2	6/4	10-10.5	37	10-10.5': Refusal, brown/gray, fine to coarse SAND, little Silt, trace Gravel, rock in tip, damp.		
				50+			
11					Refusal encountered with augers, did not set well.		
12							
13							
14							
15							
16							
17							
18							
19							
20							
GRANULAR SOILS			COHESIVE SOILS		NOTES		
Blows/ft.	Density	Blows/ft.	Consistency				
0-4	V. LOOSE	<2	V. SOFT				
5-10	LOOSE	2-4	SOFT				
11-30	M. DENSE	4-8	M. STIFF				
31-50	DENSE	8-15	STIFF				
>50	V. DENSE	15-30	V. STIFF				
		>30	HARD				

APPENDIX B VERDANTAS SITE HYDROGEOLOGY REPORT WITH LOGS & SIEVE ANALYSIS



February 8, 2023

Project Number 15494-000

Pio Lombardo, P.E.
Lombardo Associates, Inc.
188 Church Street
Newton, Massachusetts 02458

Re: Hydraulic Conductivity Estimates
Adams School Wastewater Investigation, Holliston Massachusetts

Dear Mr. Lombardo:

Verdantas LLC (Verdantas) prepared the following hydraulic conductivity estimates to assist Lombardo and Associates, Inc. (Lombardo) with hydrogeologic analysis related to wastewater discharge at the Adams Middle School (the School), in Holliston, Massachusetts. Lombardo is evaluating expansion of a wastewater treatment system to potentially serve downtown businesses in Holliston as it appears the School's wastewater system is potentially over designed. Lombardo previously provided Verdantas with groundwater data, mounding calculations, boring logs, and supporting information from studies used to design the original system for the school. Lombardo requested Verdantas' assistance in overseeing a field investigation to evaluate the extent and hydraulic properties of suitable surficial outwash material for wastewater discharge. With this information, Lombardo can estimate future wastewater loading rates, develop preliminary designs, and proceed with regulatory permitting provided subsurface conditions are suitable.

SOIL BORINGS AND GRAIN SIZE ANALYSIS

Fifteen soil borings were completed in August 2022. All were drilled to refusal with depths ranging from 5 feet at B3 to 25 feet in both B12 and B13. Refer to figures prepared by Lombardo for the location of these borings. Sediment samples were selected in communication with Lombardo in areas thought to be representative of future wastewater discharge locations. Samples were submitted to Miller Engineering and Testing, Inc. for laboratory grain size analysis. Hydraulic conductivity (K) estimates were made using the Excel spreadsheet calculator "HydroGeoSieveXL" by J.F. Devlin (2015)¹. This spreadsheet uses grain size distributions from sieve analysis to estimate K by as many as 15 different published methods depending on how many method's criteria are met by the sample. These results are summarized in Table 1. Laboratory reports and spreadsheet analysis are provided in Attachment 1.

¹ Devlin, J.F. 2015. *HydroGeoSieveXL: an Excel-based tool to estimate hydraulic conductivity from grain size analysis*. Hydrogeology Journal, DOI 10.1007/s10040-015-1255-0.

WELL INSTALLATIONS AND SLUG TESTS

Seven monitoring wells (MW-1 through MW-8; MW-4 was not installed) were installed in November 2022. These wells were standard construction 2-inch PVC with 10-foot, 10-slot screens. Depths ranged from 24 feet in MW-1 to 13.5-feet in MW-6. In December 2022, depths to groundwater ranged from 3.62 feet in MW-8 to 20.72 feet in MW-1. MW-6 was dry at 13.5 feet. Boring logs and well construction diagrams were previously provided to Lombardo.

Slug tests were conducted in wells MW-1, MW-2, MW-3, MW-5, MW-7, MW-8 and an existing USGS well located generally between MW-3 and MW-5 in December 2022. These tests were conducted by installing a pressure transducer with a datalogger in the well to record water levels, then inserting a solid bar (slug) below the water level to displace the water in the well. Once the water in the well has stabilized to a static condition, the slug is then quickly removed and the water level recovery over time in the well is recorded. These data are analyzed by accepted methods to estimate the hydraulic conductivity of the material in which the well is screened. Slug test results are included as Attachment 2 and results of this analysis are given in Table 2.

CONCLUSIONS

Grain size analysis supports a K estimation of approximately 30 feet per day and slug tests support a lower K estimation of approximately 5 feet per day. There is a large range of grain size analysis results and these only apply to material included in the soil sample. Slug test results evaluate a larger thickness of aquifer material (the length of the saturated well screen in comparison to the volume of a soil sample) and typically provide a more accurate estimate of hydraulic conductivity.

LIMITATIONS

Verdantas provided the opinions and conclusions contained within this report based upon an evaluation of subsurface conditions observed and/or reported, as documented in the report text and attached materials. Verdantas believes the subsurface explorations, testing and evaluations described herein were performed in a manner consistent with the services that would have been provided by other hydrogeologic professionals under similar circumstances. However, given the variable nature of soil deposits and hydrogeology, we cannot represent that the subsurface conditions and hydrogeologic properties described in this report are exact.

The findings presented herein do not present certainties, but rather our opinion of likely conditions founded on available information, limited test data, our professional judgement, and a reasonable degree of confidence in our conclusions. While our evaluation accounts for reasonable variations in input parameters for our analyses, disparities in the Site subsurface conditions from those included in our analyses could result in differing findings from those presented.

Sincerely,
VERDANTAS LLC

A handwritten signature in blue ink that reads "David G. Harwood".

David G. Harwood, P.G.
Senior Hydrogeologist
(978) 506-5064

A handwritten signature in blue ink that reads "David A. Maclean".

David A. Maclean, P.G., L.S.P.
Senior Consultant
(603) 657-2021

Attachment 1- Grain Size Analyses
Attachment 2- Slug Test Analysis

\geoinc.com\DFS\Project Files\15000\15494\Nov 22 2022 authorization deliverable\K report\15494 2023-2-8 K Results.docx

Table 1
Summary of Hydraulic Conductivity Results from Grain Size Analysis
Holliston School August 2022

Boring	Sample Depth	K ft/day	Soil Description
B4	0-10	33	poorly sorted gravelly sand low in fines
	10-16	98	poorly sorted sandy gravel low in fines
	16+	55	poorly sorted sandy gravel low in fines
B6	5-12	18	poorly sorted gravelly sand low in fines
	10-15	33	poorly sorted gravelly sand low in fines
	15+	32	poorly sorted sandy gravel low in fines
B9	15-17.5	15	poorly sorted gravelly sand low in fines
	0-9	27	poorly sorted gravelly sand low in fines
	9-14.5	87	poorly sorted gravelly sand low in fines
B11	5-10	20	poorly sorted gravelly sand low in fines
	10-14	32	poorly sorted gravelly sand low in fines
B12	0-10	2	poorly sorted gravelly sand low in fines
B-13	5-15	0.4	poorly sorted sandy silt low in fines
	15-20	1	poorly sorted sand low in fines

Table 2
Summary of Slug Test Results in Feet per Day
Holliston School December 2022

Well	Trial 1	Trial 2	Notes
MW-1	5	n/a	
MW-2	20	15	note 1
MW-3	7	n/a	note 1
MW-4	n/a	n/a	no well installed
MW-5	2	2	
MW-6	n/a	n/a	dry
MW-7	5	4	
MW-8	6	5	
USGS	1	n/a	assumed 10 foot screen

Notes

1. Analysis from slug in data which is not valid for wells without a fully saturated screen.

Results are presented because slug out data was unusable.

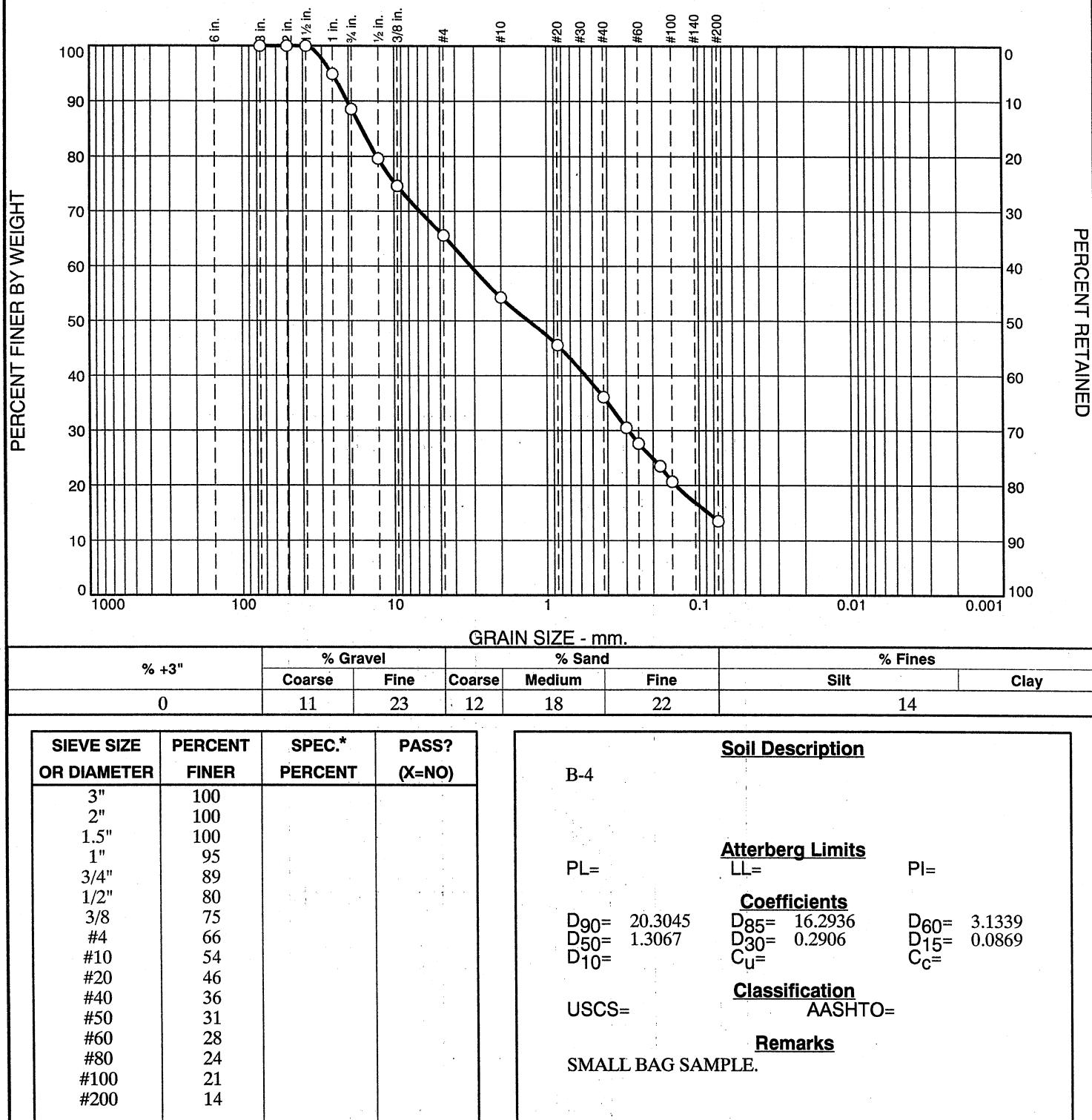
Despite this, MW-3 results are comparable to others.

However, higher MW-2 results should be viewed as questionable.

ATTACHMENT 1

GRAIN SIZE ANALYSIS

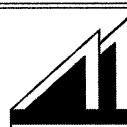
GRAINSIZE DISTRIBUTION REPORT



Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286A

Depth: 0-10'

Date: 9-8-22



MILLER ENGINEERING & TESTING, INC.

Client:

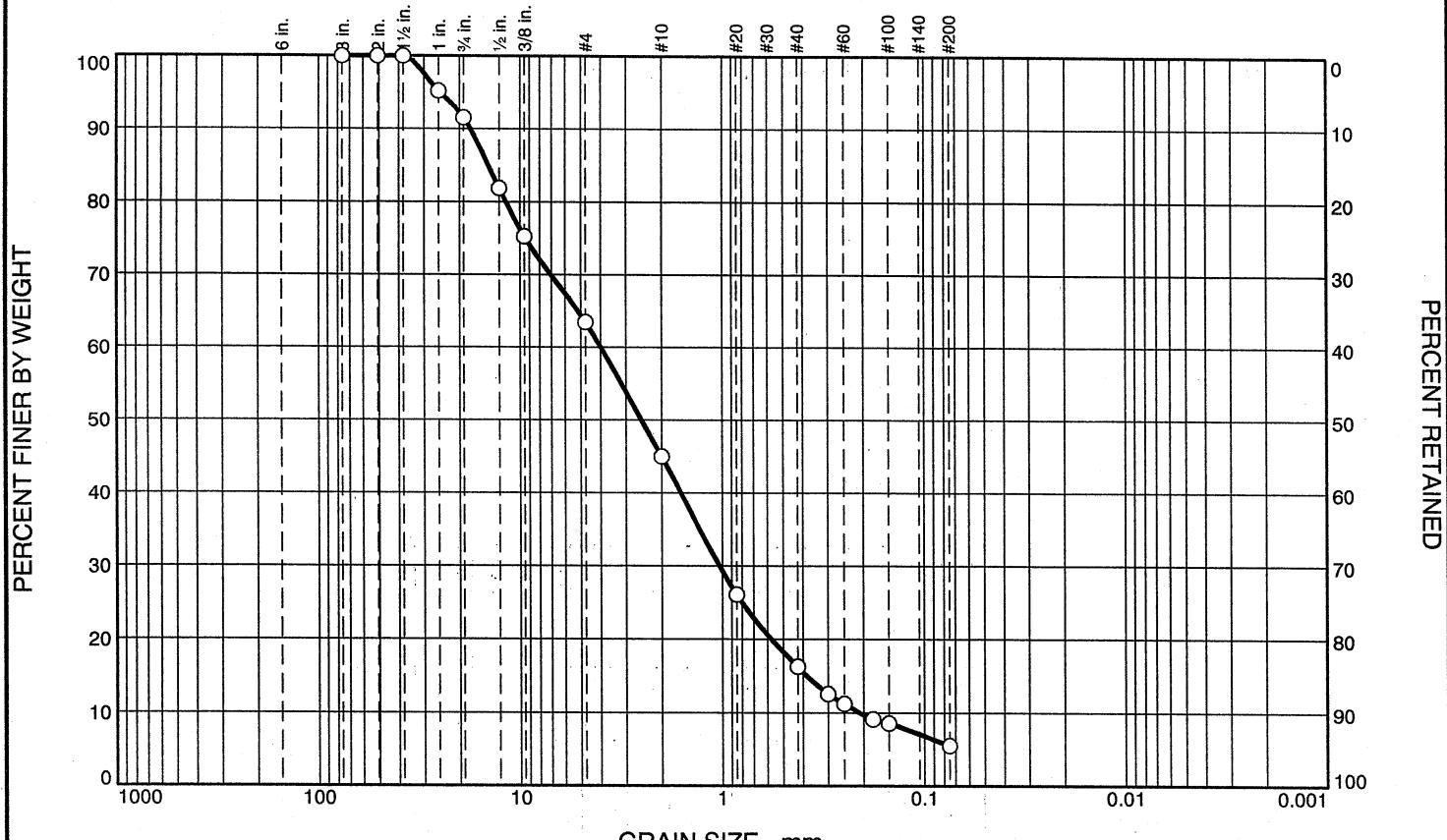
Project: 22 154.NH

Project No: VERDANTAS QC

Figure L22286A

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT



SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100		
2"	100		
1.5"	100		
1"	95		
3/4"	92		
1/2"	82		
3/8	75		
#4	63		
#10	45		
#20	26		
#40	16		
#50	13		
#60	11		
#80	9		
#100	9		
#200	5.5		

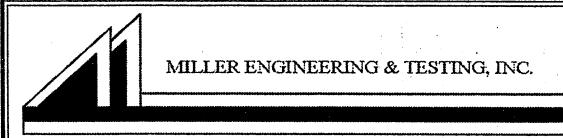
* (no specification provided)

<u>Soil Description</u>	
B-4	
PL=	LL=
D ₉₀ = 17.6282	D ₆₀ = 3.9886
D ₅₀ = 2.5105	D ₃₀ = 1.0341
D ₁₀ = 0.2098	C _u = 19.01 C _c = 1.28
<u>Atterberg Limits</u>	
	PI=
<u>Coefficients</u>	
USCS=	AASHTO=
<u>Classification</u>	
SMALL BAG SAMPLE.	
<u>Remarks</u>	

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286B

Depth: 10'-16'

Date: 9-8-22



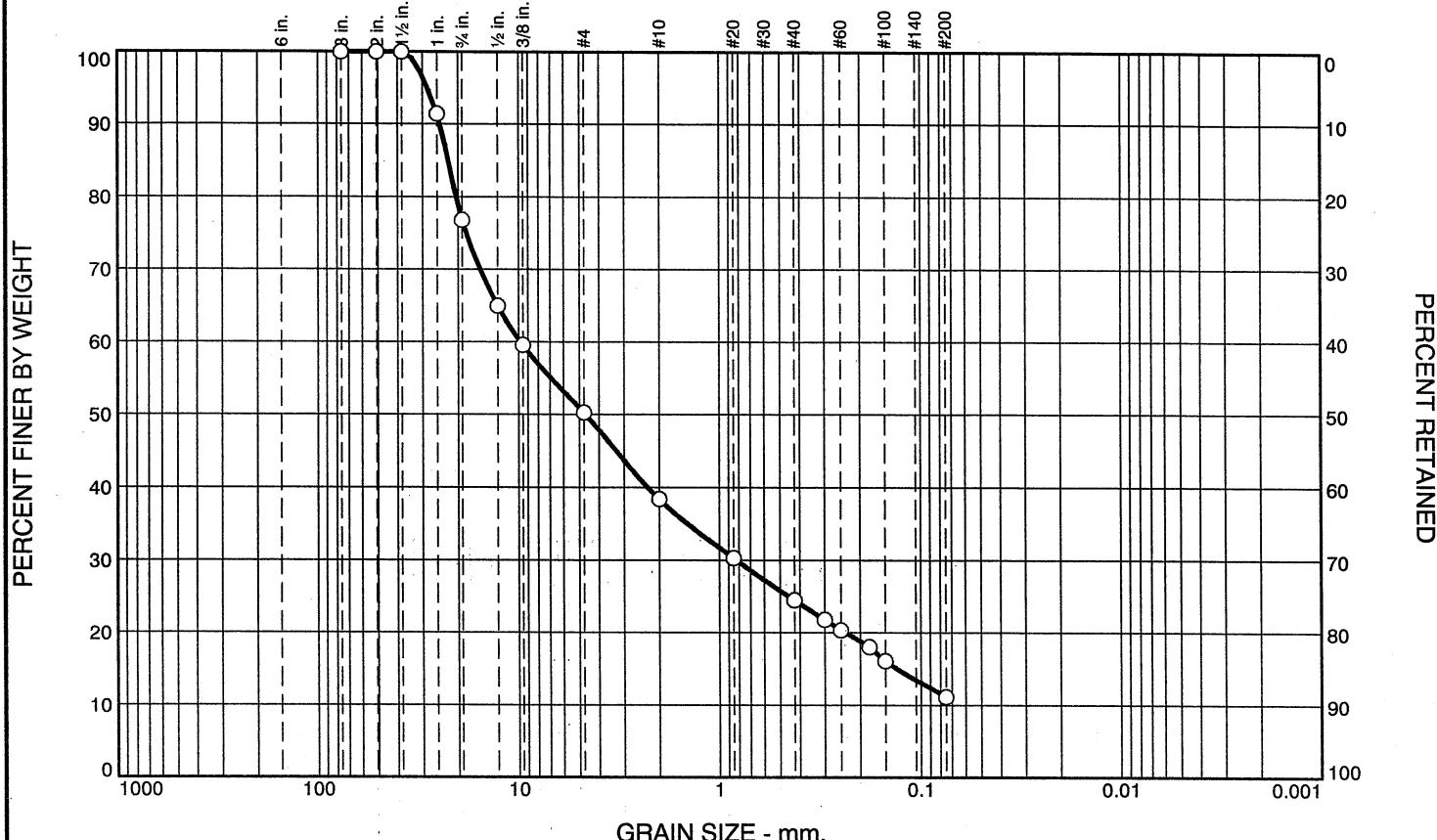
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286B

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT



SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100		
2"	100		
1.5"	100		
1"	91		
3/4"	77		
1/2"	65		
3/8	60		
#4	50		
#10	38		
#20	30		
#40	24		
#50	22		
#60	20		
#80	18		
#100	16		
#200	11		

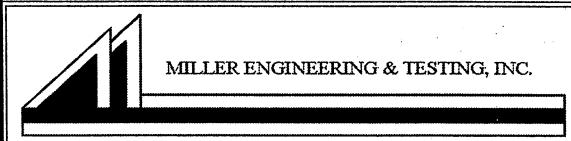
* (no specification provided)

Soil Description	
B-4	
PL=	
LL=	PI=
D ₉₀ = 24.5290	D ₆₀ = 9.7867
D ₅₀ = 4.6511	D ₃₀ = 0.8213
D ₁₀ =	C _u =
C _c =	
Classification	
USCS=	AASHTO=
Remarks	
SMALL BAG SAMPLE.	

Source of Sample: HOLLASTON MA, PROJECT # 15494
 Sample Number: L22286C

Depth: 16' +

Date: 9-8-22



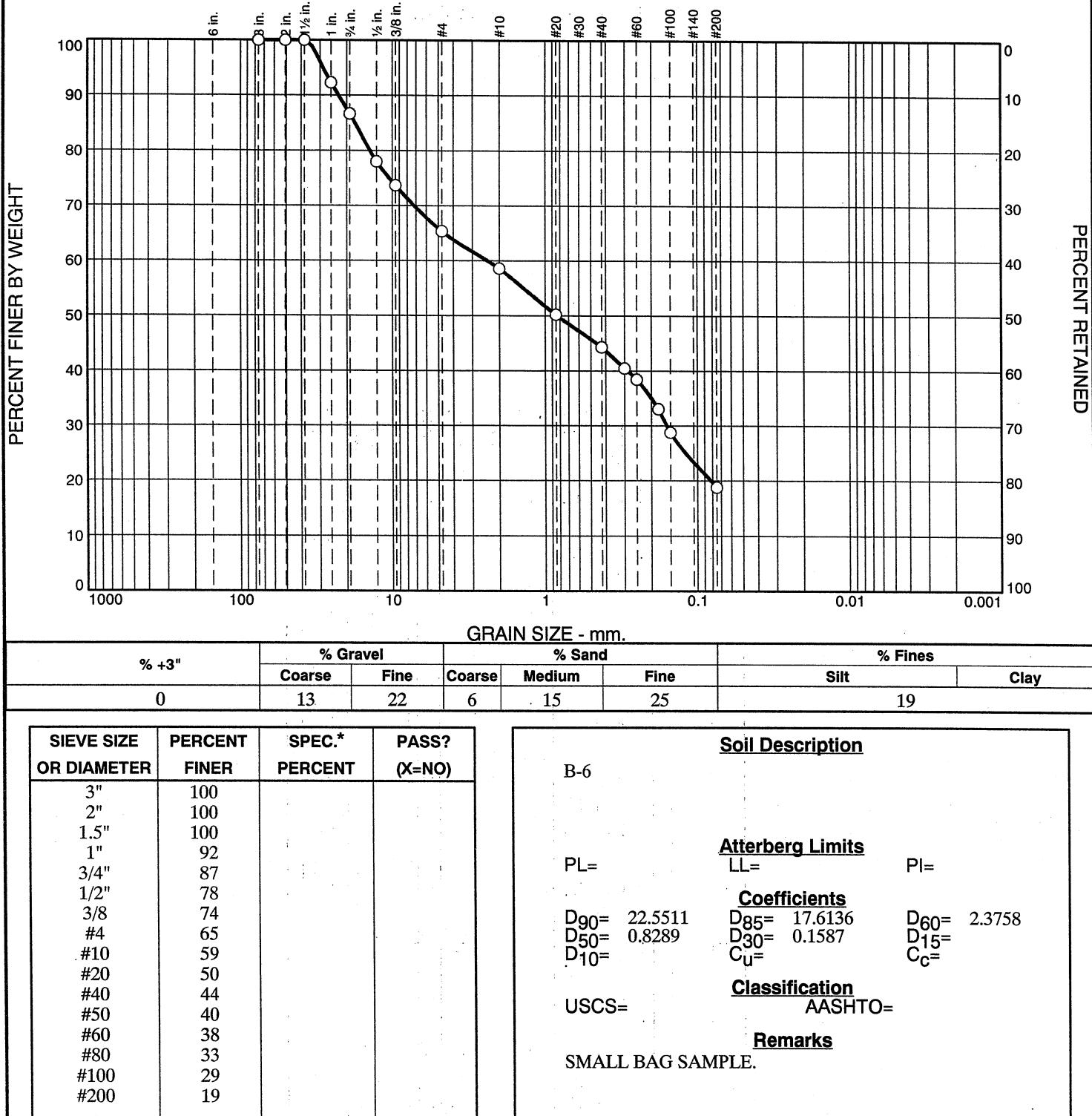
Client:
 Project: 22.154.NH

Project No: VERDANTAS OC

Figure L22286C

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT

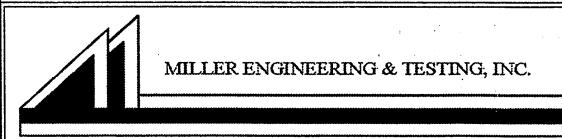


* (no specification provided)

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286D

Depth: 5'-12'

Date: 9-8-22



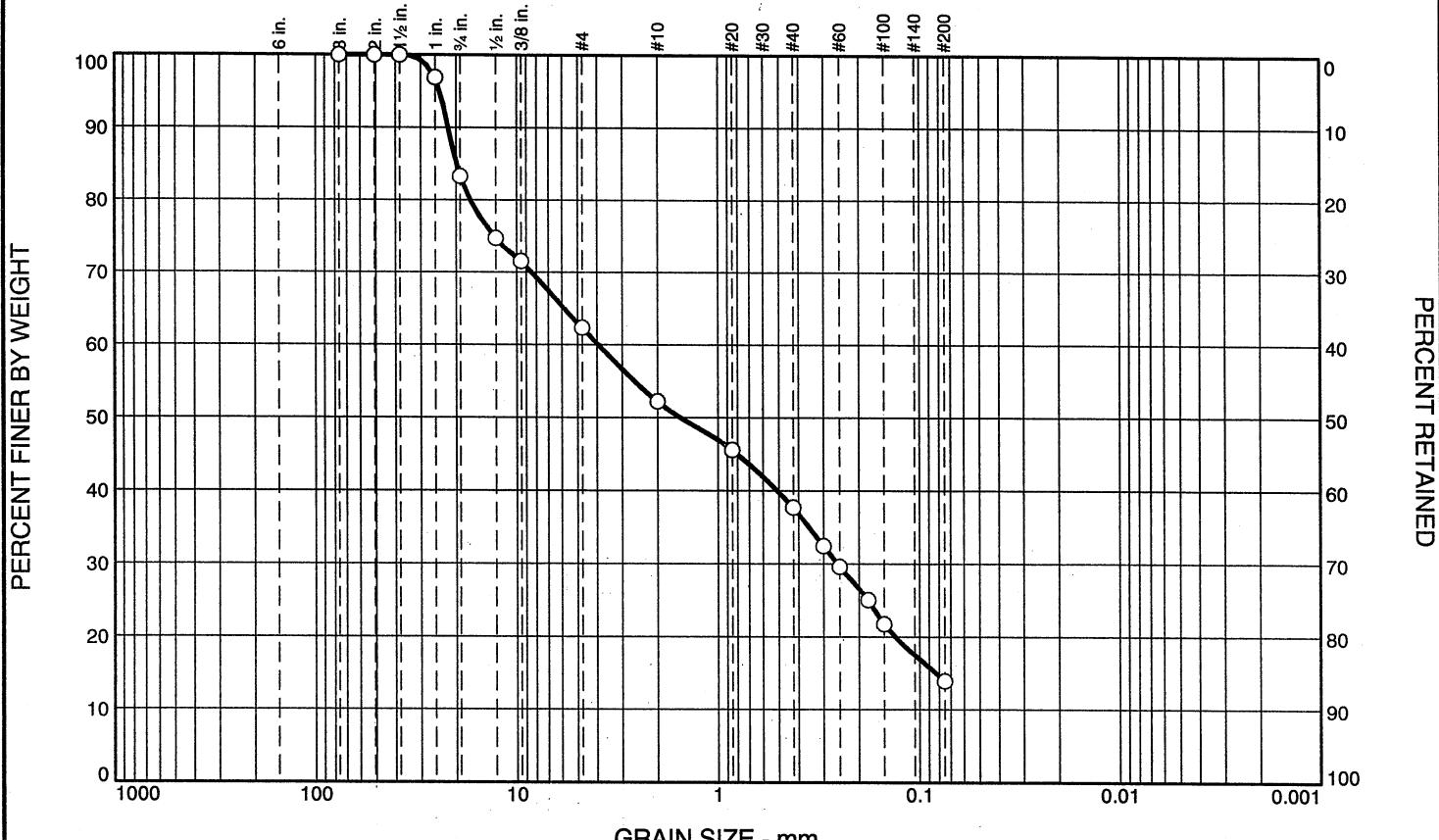
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286D

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT



SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100		
2"	100		
1.5"	100		
1"	97		
3/4"	83		
1/2"	75		
3/8"	72		
#4	62		
#10	52		
#20	46		
#40	38		
#50	32		
#60	30		
#80	25		
#100	22		
#200	14		

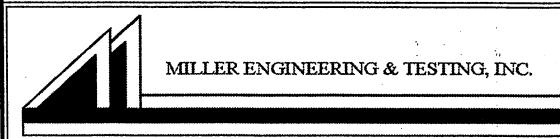
* (no specification provided)

Soil Description	
B-6	
PL=	
LL=	
D ₉₀ = 21.7976	D ₆₀ = 3.9308
D ₅₀ = 1.5290	D ₁₅ = 0.0827
D ₁₀ =	C _u =
C _c =	
Atterberg Limits	
USCS=	AASHTO=
Coefficients	
Classification	
Remarks	
SMALL BAG SAMPLE.	

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286E

Depth: 10'-15'

Date: 9-8-22



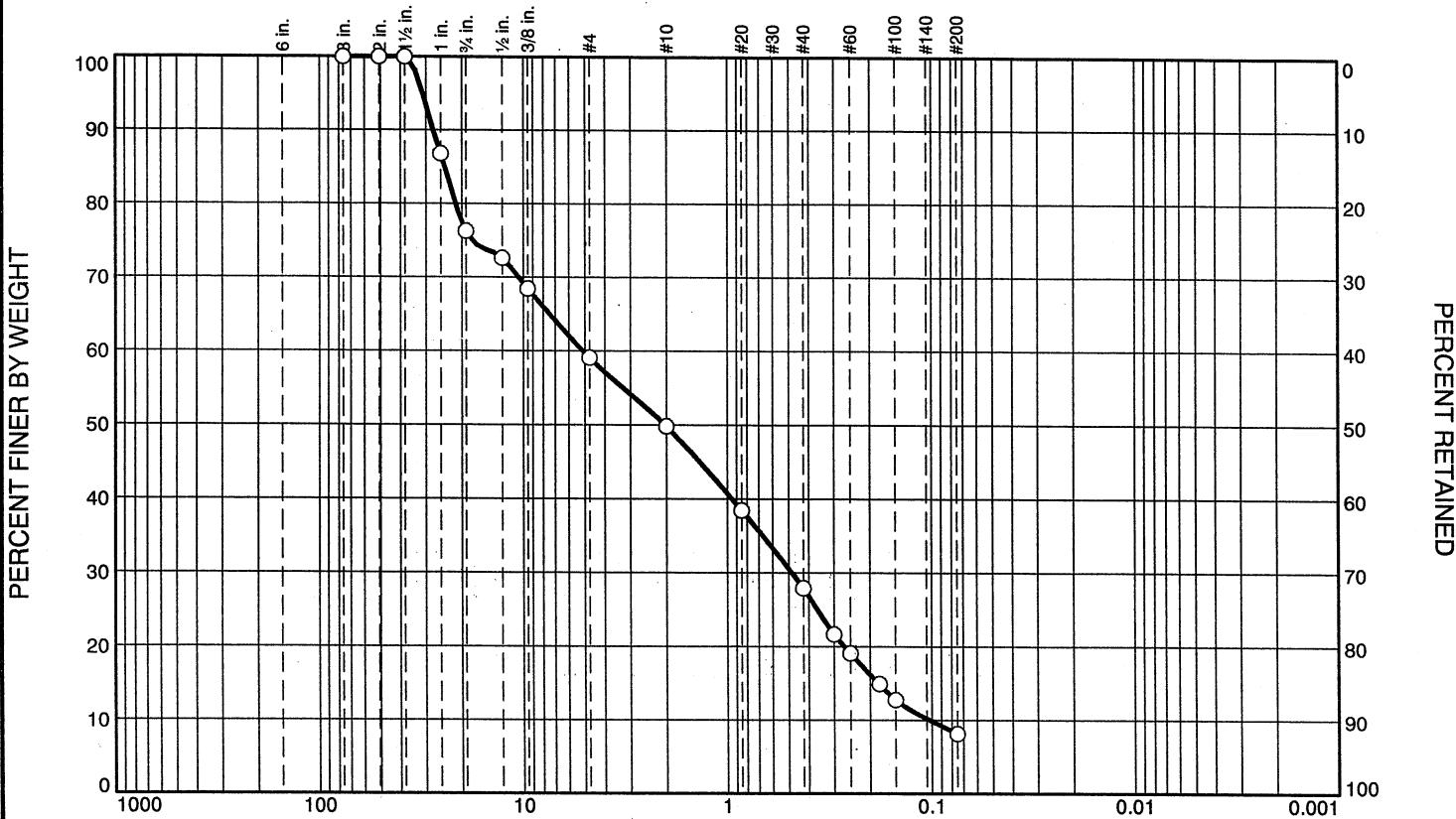
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286E

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	0	24	17	9	22	20	8

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100		
2"	100		
1.5"	100		
1"	87		
3/4"	76		
1/2"	73		
3/8"	68		
#4	59		
#10	50		
#20	38		
#40	28		
#50	22		
#60	19		
#80	15		
#100	13		
#200	8.1		

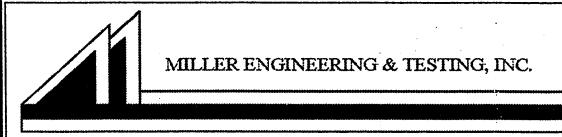
* (no specification provided)

Soil Description			
B-6			
PL=	Atterberg Limits	LL=	PI=
D ₉₀ = 27.5586	D ₈₅ = 24.1943	D ₆₀ = 5.1019	
D ₅₀ = 2.0221	D ₃₀ = 0.4821	D ₁₅ = 0.1809	
D ₁₀ = 0.1023	C _u = 49.89	C _c = 0.45	
USCS=	Classification	AASHTO=	
Remarks			
SMALL BAG SAMPLE.			

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286F

Depth: 15'-+

Date: 9-8-22



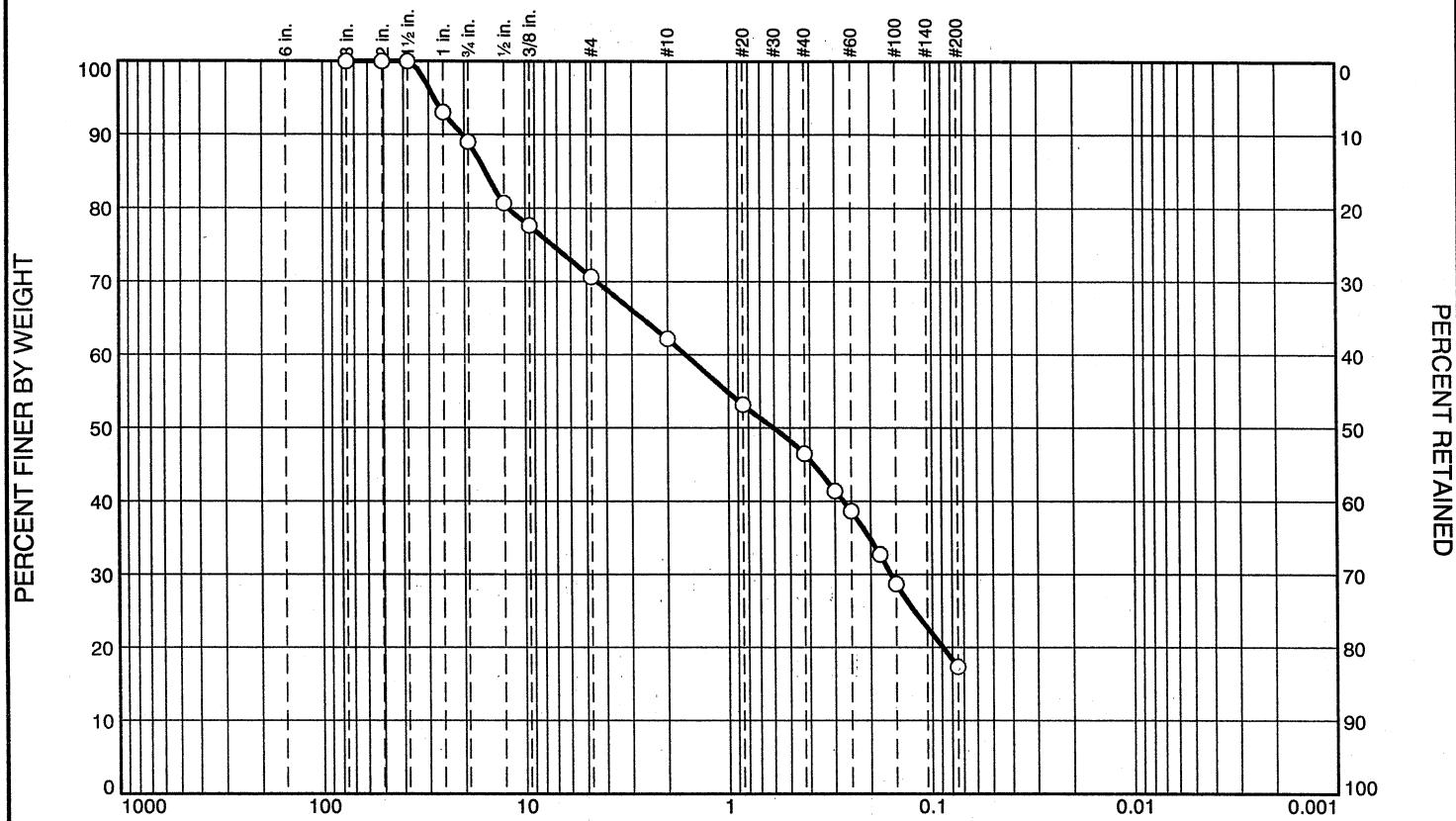
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286F

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	0	11	18	9	15	30	17

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100		
2"	100		
1.5"	100		
1"	93		
3/4"	89		
1/2"	81		
3/8"	78		
#4	71		
#10	62		
#20	53		
#40	47		
#50	41		
#60	39		
#80	33		
#100	29		
#200	17		

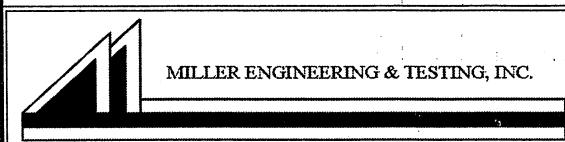
* (no specification provided)

Soil Description			
B-9			
PL=	Atterberg Limits	LL=	PI=
D ₉₀ = 20.2363	D ₈₅ = 15.7123	D ₆₀ = 1.6218	
D ₅₀ = 0.5994	D ₃₀ = 0.1594	D ₁₅ =	C _c =
D ₁₀ =	C _u =	AASHTO=	
USCS=	Classification	AASHTO=	
Remarks			
SMALL BAG SAMPLE.			

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286G

Depth: 1 ST

Date: 9-8-22



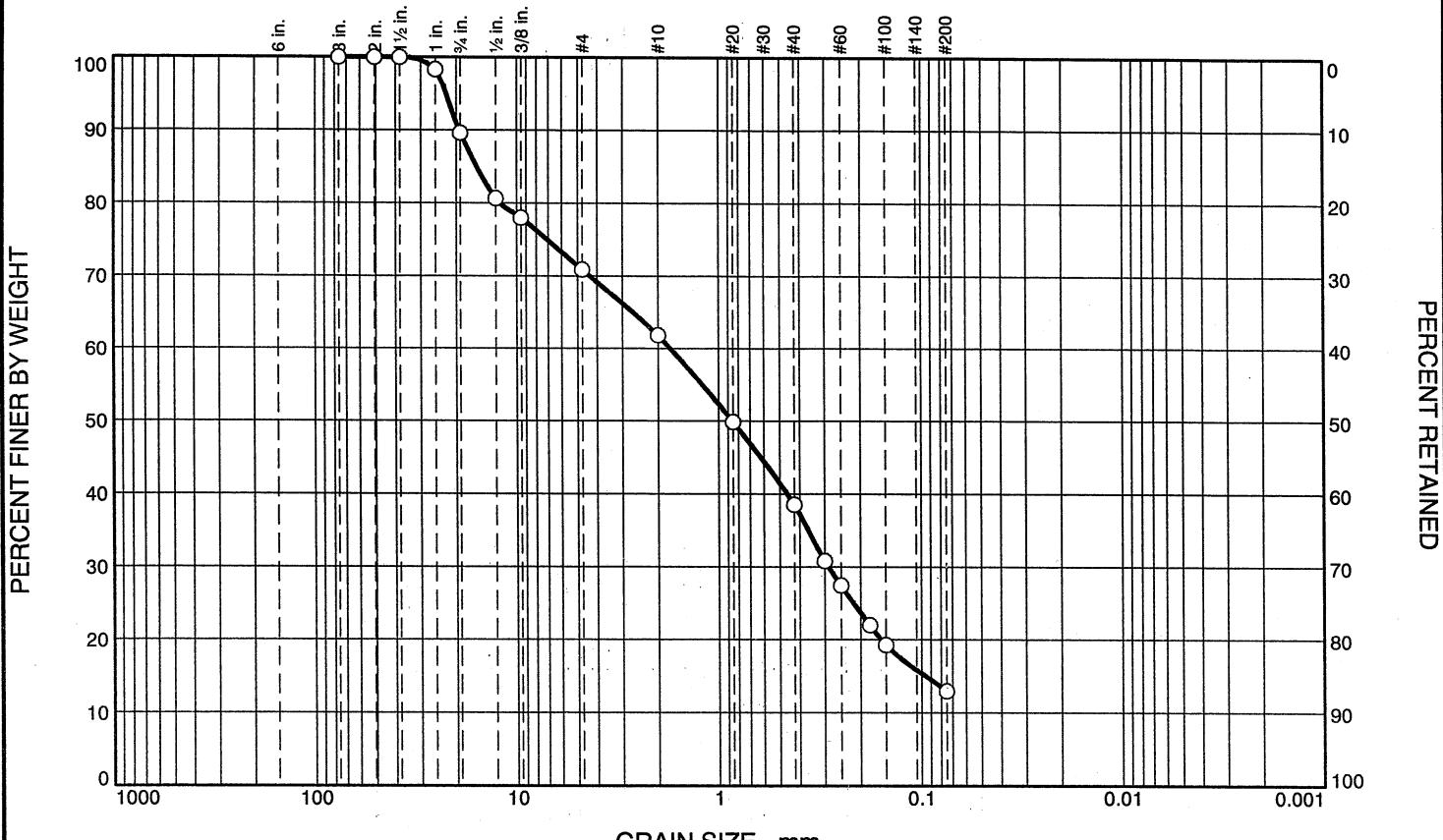
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286G

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT



SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100		
2"	100		
1.5"	100		
1"	98		
3/4"	90		
1/2"	81		
3/8"	78		
#4	71		
#10	62		
#20	50		
#40	39		
#50	31		
#60	27		
#80	22		
#100	19		
#200	13		

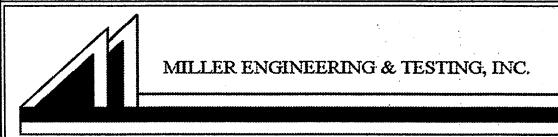
* (no specification provided)

Soil Description			
B-9			
PL=	Atterberg Limits	LL=	PI=
D ₉₀ = 19.3152	D ₈₅ = 15.8350	D ₆₀ = 1.7292	
D ₅₀ = 0.8569	D ₃₀ = 0.2878	D ₁₅ = 0.0956	C _u =
D ₁₀ =	C _c =		
USCS=	Classification	AASHTO=	
	Remarks		
SMALL BAG SAMPLE.			

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286H

Depth: 0-9'

Date: 9-8-22



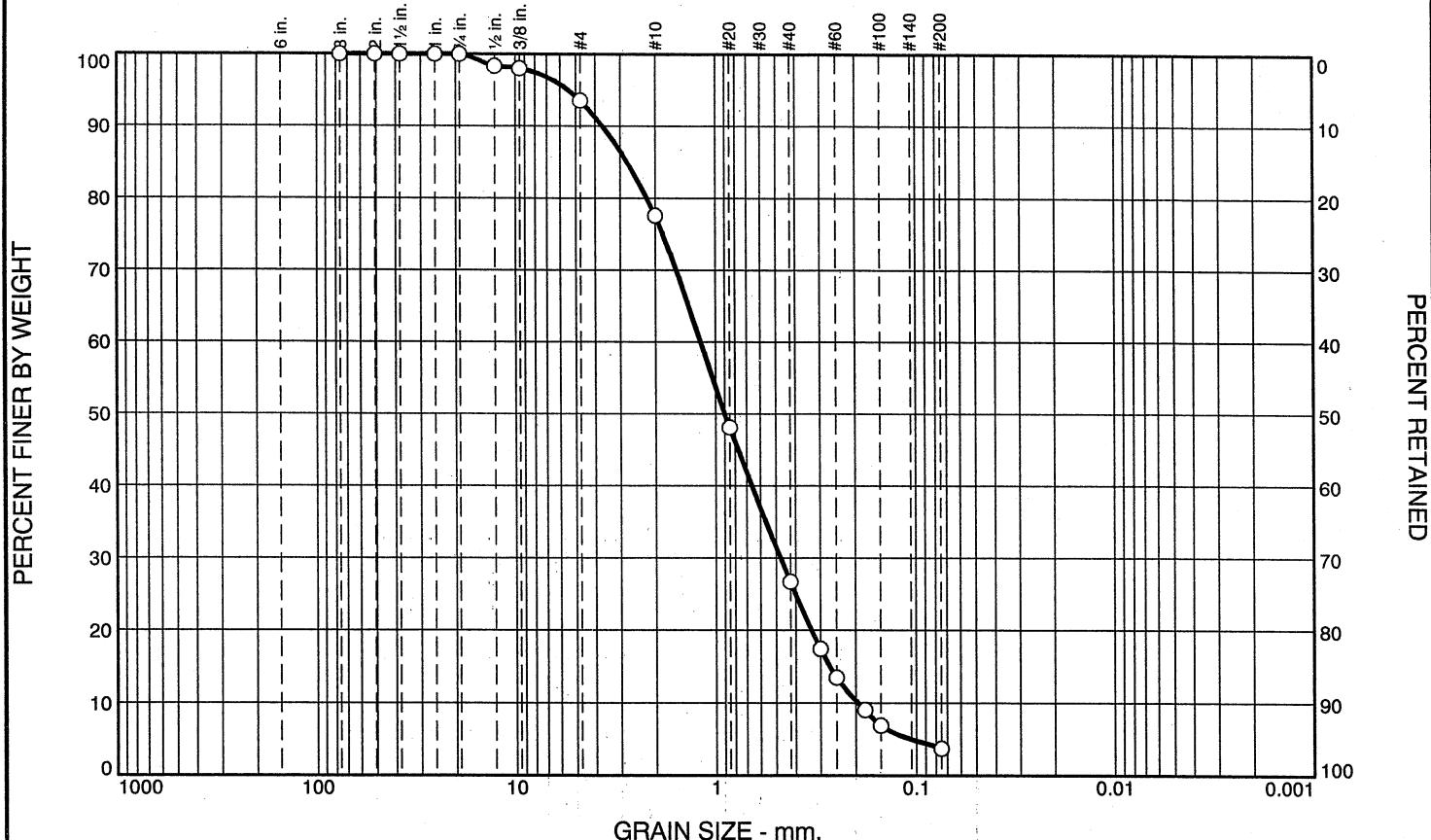
Client:
Project: 22.154.NH

Project No.: VERDANTAS QC

Figure L22286H

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0	0	6	16	51	23	4	

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100		
2"	100		
1.5"	100		
1"	100		
3/4"	100		
1/2"	98		
3/8"	98		
#4	94		
#10	78		
#20	48		
#40	27		
#50	17		
#60	13		
#80	9		
#100	7		
#200	3.7		

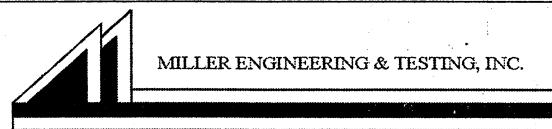
* (no specification provided)

<u>Soil Description</u>	
B-9	
PL=	Atterberg Limits LL=
D ₉₀ = 3.6724	PI=
D ₅₀ = 0.8984	
D ₁₀ = 0.1945	D ₆₀ = 1.1853
C _U = 6.10	D ₁₅ = 0.2698
C _c = 0.98	AASHTO=
<u>Classification</u>	
USCS= SP	
<u>Remarks</u>	
SMALL BAG SAMPLE.	

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286I

Depth: 9'-14.5'

Date: 9-8-22



Client:

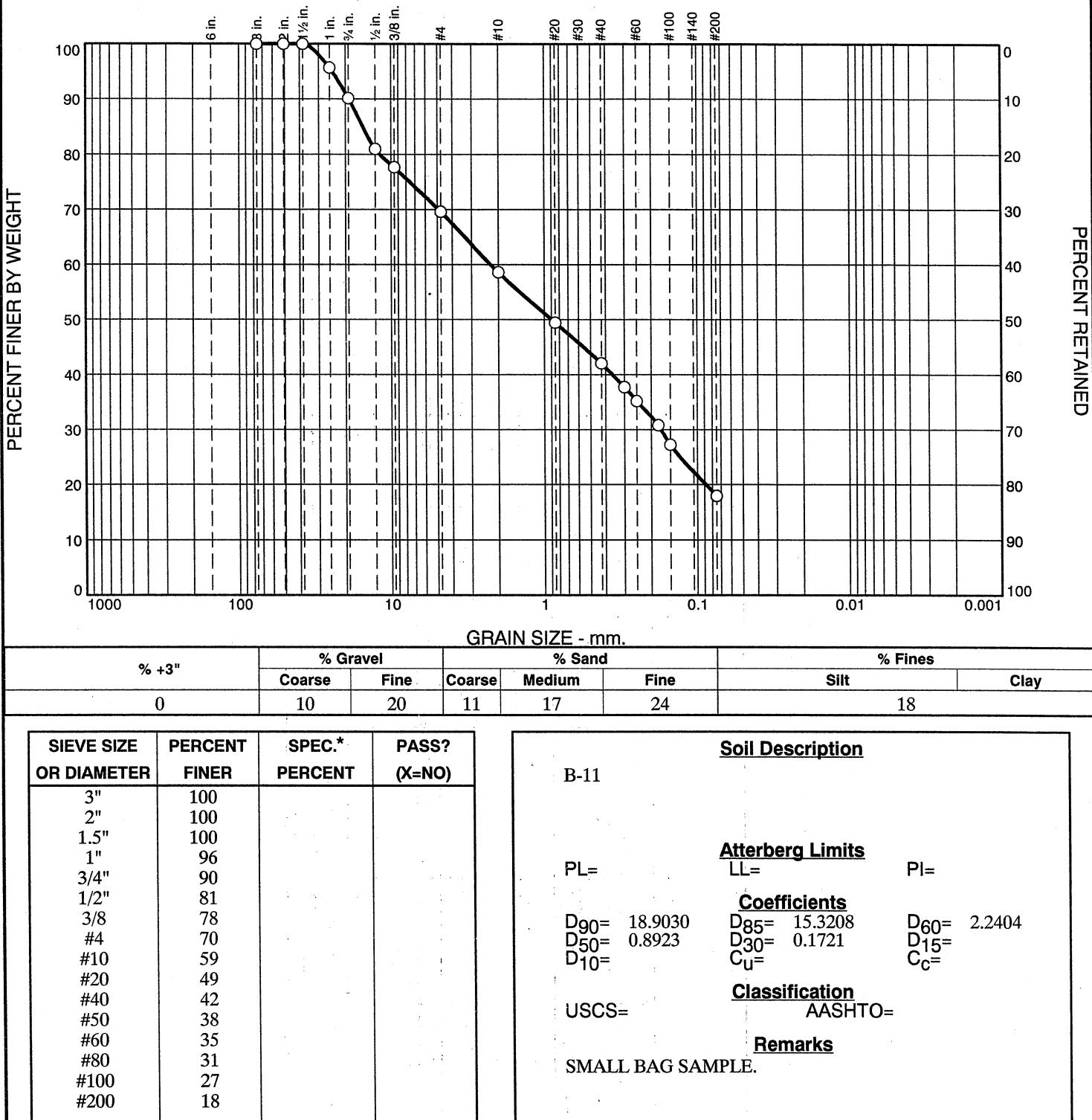
Project: 22.154.NH

Project No.: VERDANTAS QC

Figure L22286I

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT

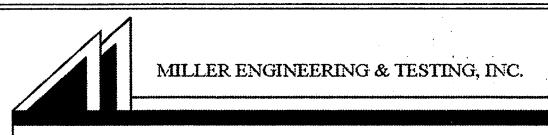


* (no specification provided)

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286J

Depth: 5'-10'

Date: 9-8-22



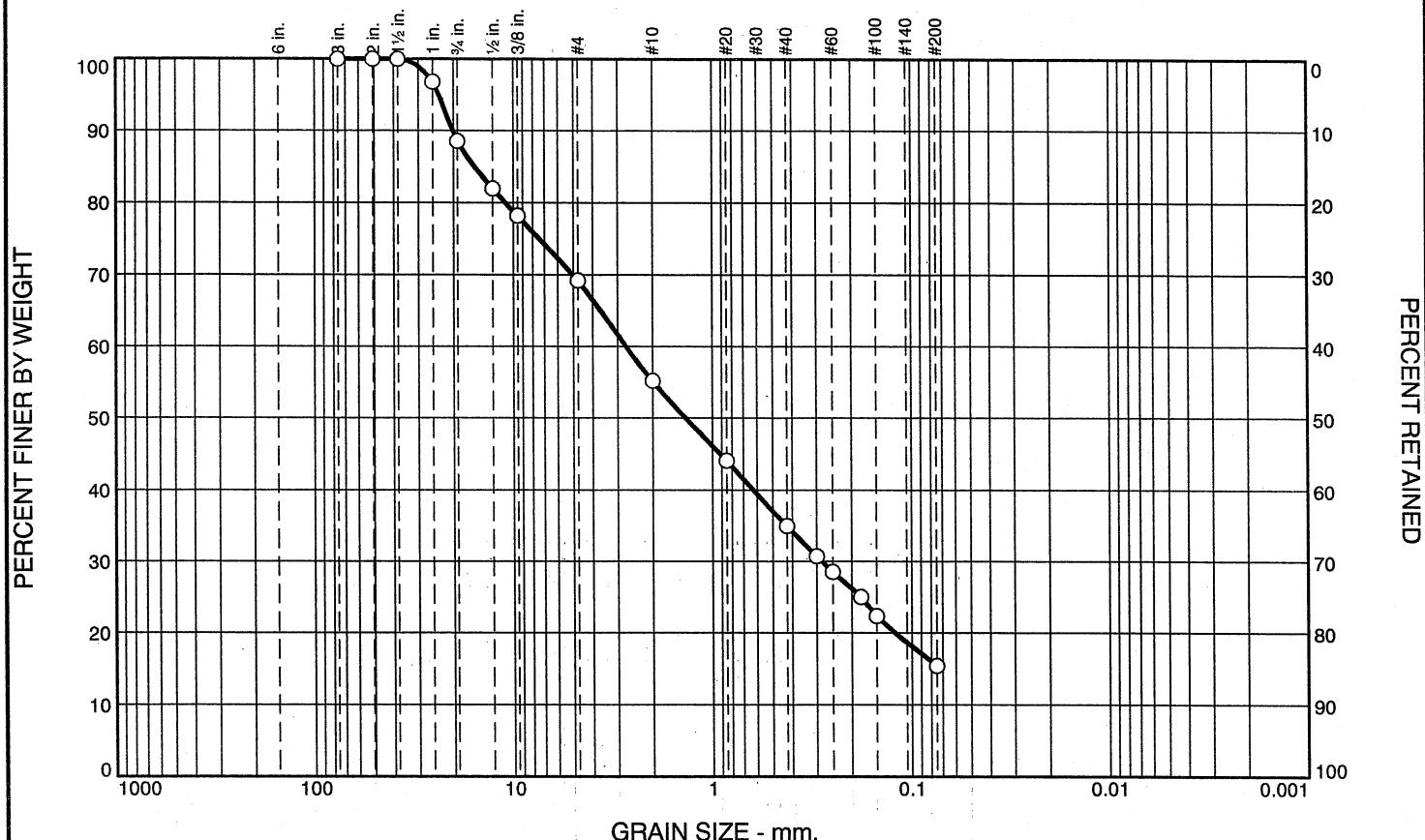
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286J

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT



% +3"	% Gravel			% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay	
	0	11	20	14	20	20	15	

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100		
2"	100		
1.5"	100		
1"	97		
3/4"	89		
1/2"	82		
3/8	78		
#4	69		
#10	55		
#20	44		
#40	35		
#50	31		
#60	29		
#80	25		
#100	22		
#200	15		

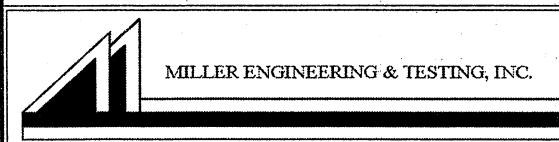
* (no specification provided)

<u>Soil Description</u>	
B-11	
PL=	LL=
D ₉₀ = 20.1206	D ₆₀ = 2.7069
D ₅₀ = 1.3586	D ₃₀ = 0.2824
D ₁₀ =	C _u =
USCS=	AASHTO=
<u>Atterberg Limits</u>	
	PI=
<u>Coefficients</u>	
D ₈₅ = 15.6816	D ₁₅ =
	C _c =
<u>Classification</u>	
SMALL BAG SAMPLE.	
<u>Remarks</u>	

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286K

Depth: 10'-14'

Date: 9-8-22



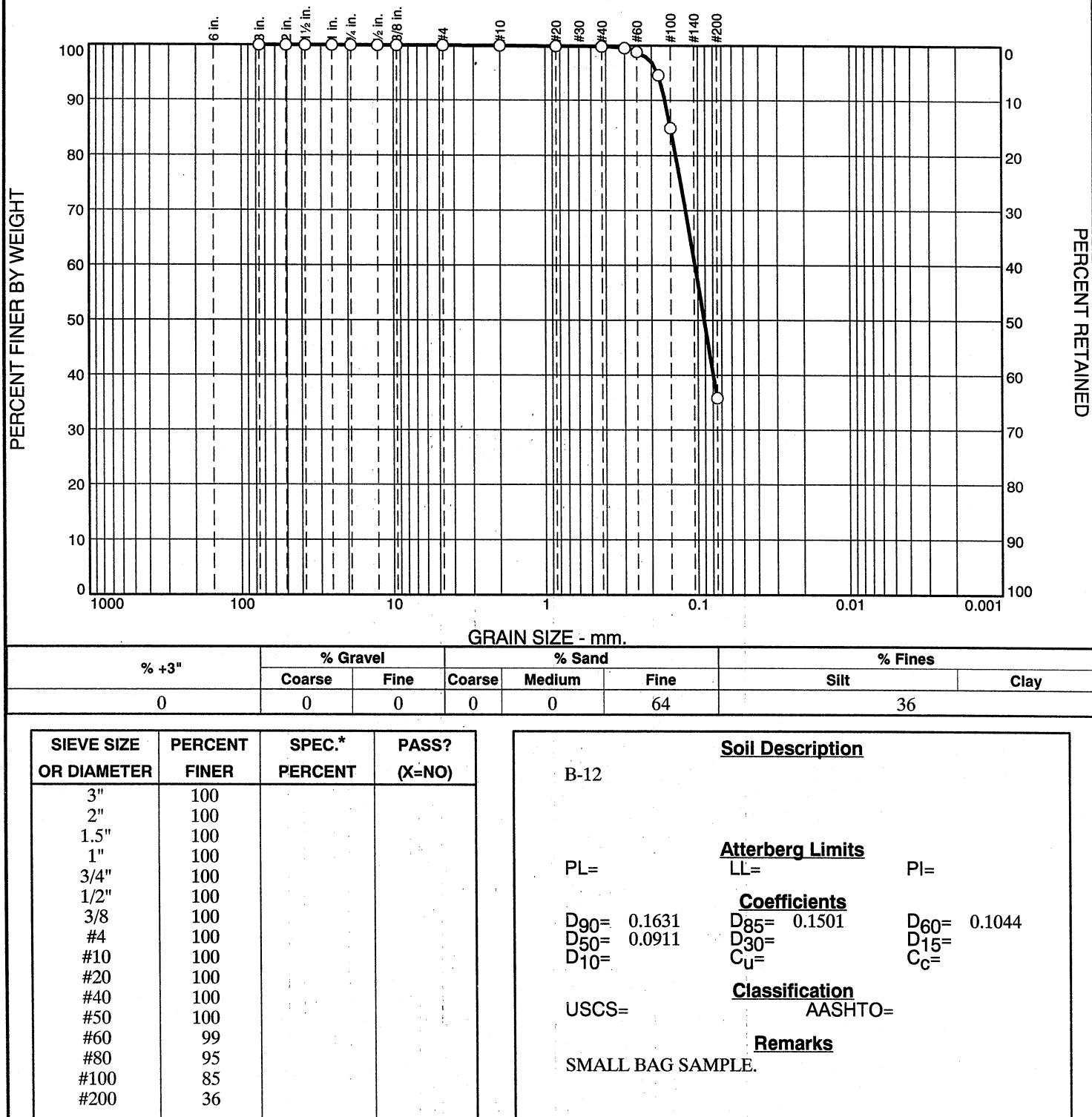
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286K

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT

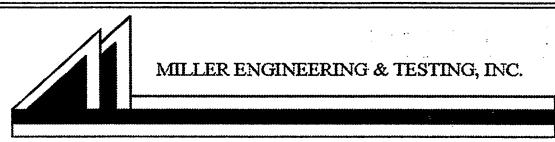


* (no specification provided)

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286L

Depth: 0-10'

Date: 9-8-22



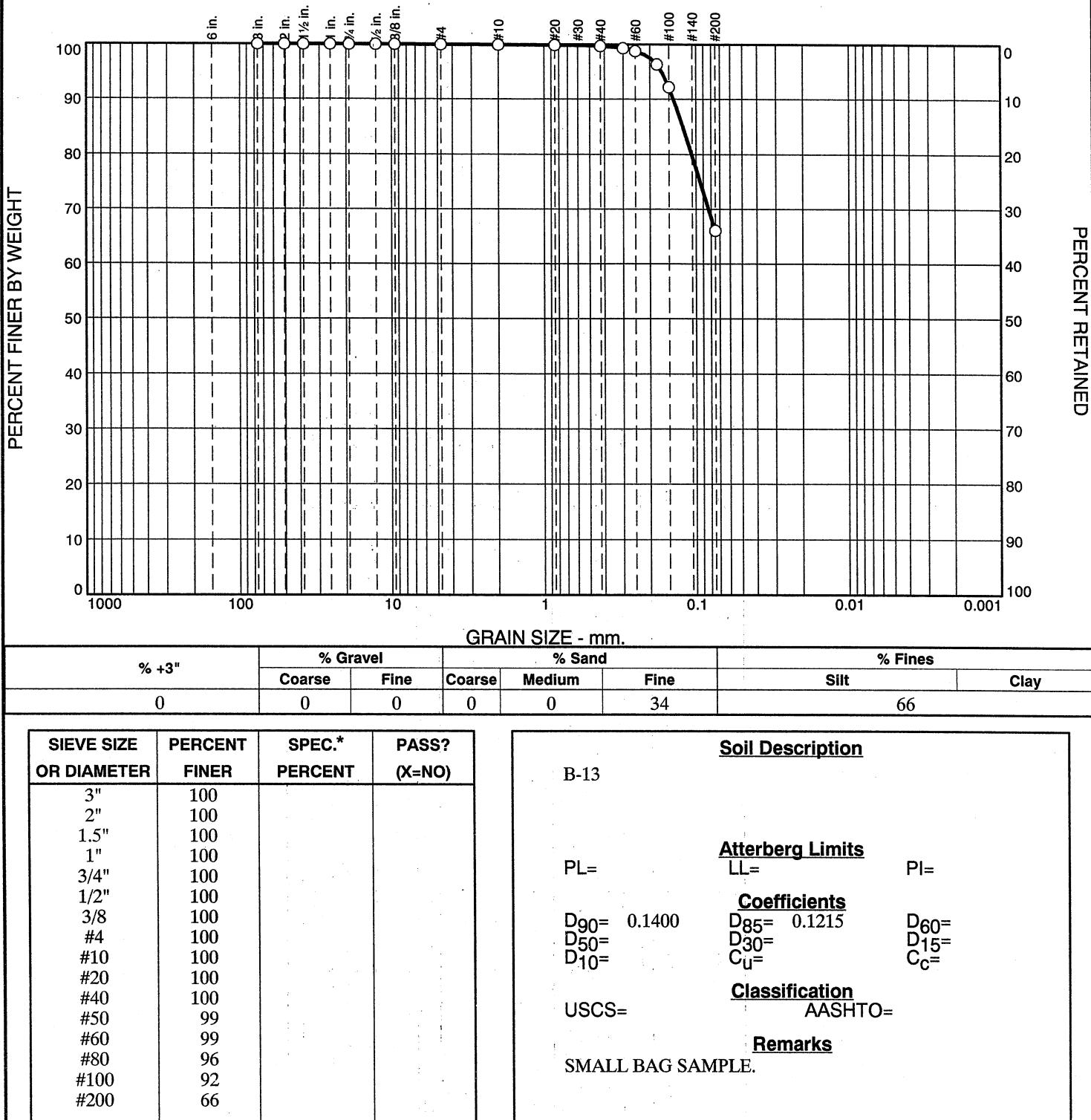
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286L

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT

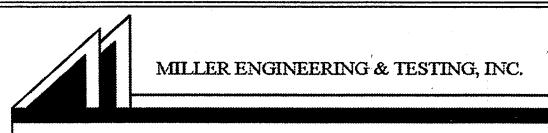


* (no specification provided)

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286M

Depth: 5'-15'

Date: 9-8-22



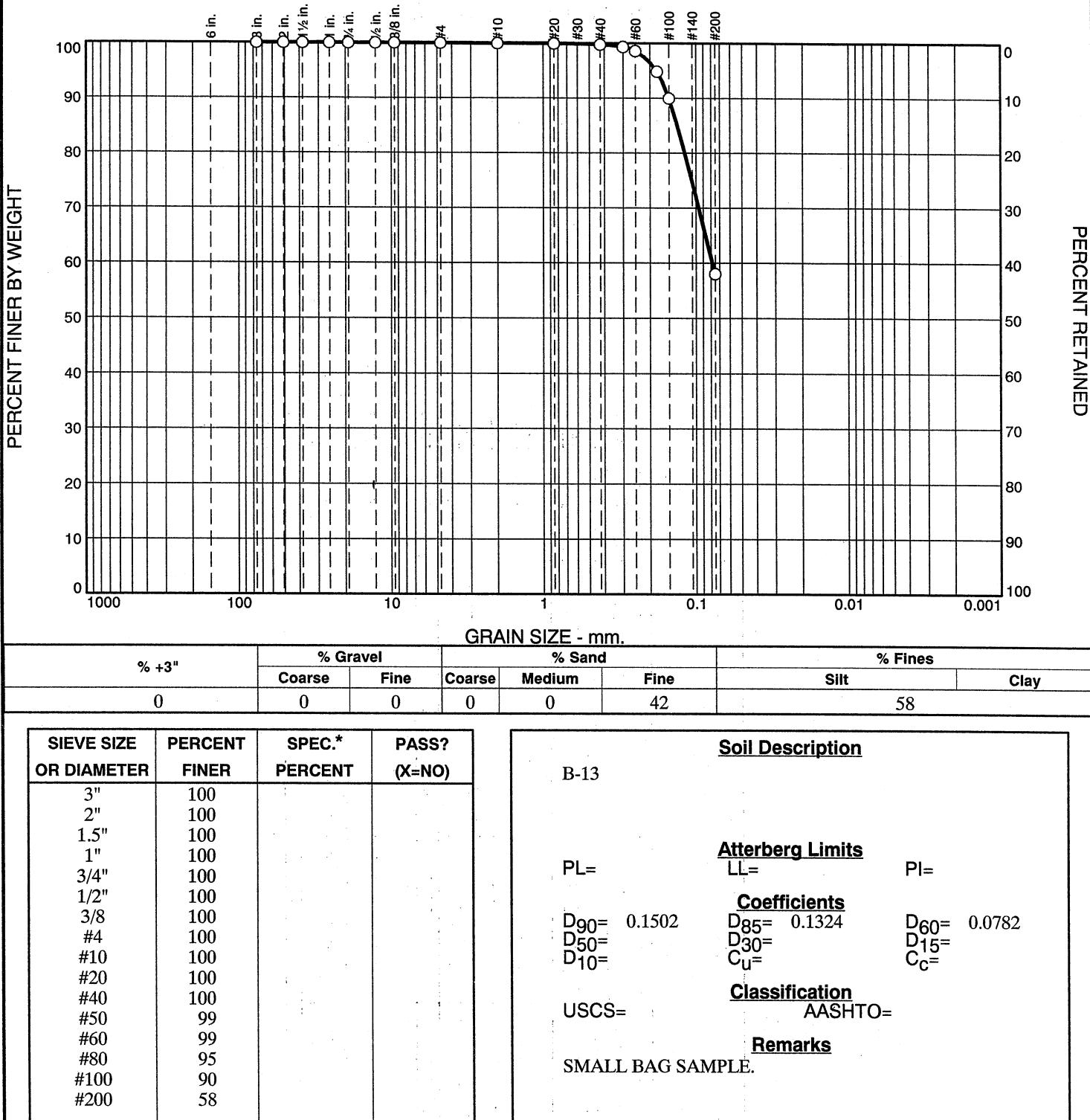
Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286M

Tested By: DM NL

GRAINSIZE DISTRIBUTION REPORT

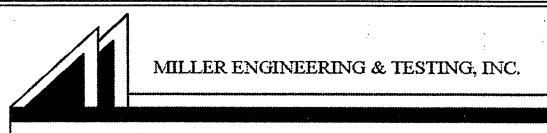


* (no specification provided)

Source of Sample: HOLLASTON MA, PROJECT # 15494
Sample Number: L22286N

Depth: 15'-20'

Date: 9-8-22



Client:
Project: 22.154.NH

Project No: VERDANTAS QC

Figure L22286N

Tested By: DM NL



Grain Size Analysis Report

Date:

9/12/2022

Sample Name: B-4 0-10

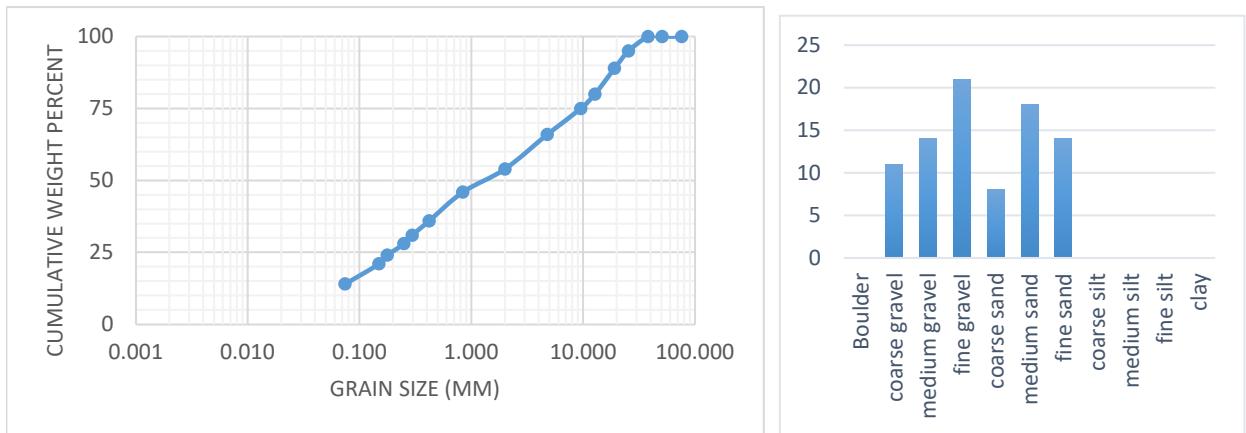
Mass Sample (g):

100

T (°C)

20

Poorly sorted gravelly sand low in fines



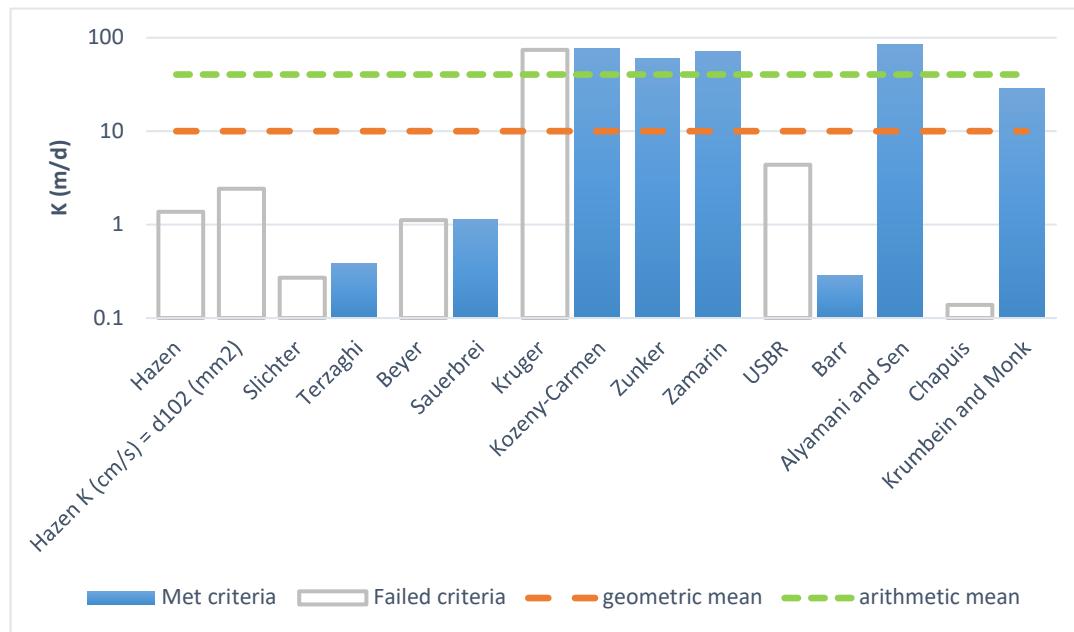
Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.053	Uniformity Coef.	63.95
76.2	0	0	100	d17	0.106	n computed	0.26
50.8	0	0	100	d20	0.138	g (cm/s ²)	980.00
38.1	0	0	100	d50	1.421	ρ (g/cm ³)	0.9981
25.4	5	0.05	95	d60	3.380	μ (g/cm s)	0.0098
19.05	6	0.06	89	de (Kruger)	0.655	ρg/μ (1/cm s)	9.9327E+04
12.7	9	0.09	80	de (Kozeny)	0.603	tau (Sauerbrei)	1.053
9.525	5	0.05	75	de (Zunker)	0.620	d _{geometric mean}	1.885
4.76	9	0.09	66	de (Zamarin)	0.638	σ _ϕ	3.338
2	12	0.12	54	lo (Alyameni)	-0.289		
0.841	8	0.08	46				
0.42	10	0.1	36		mm	0	% in sample
0.297	5	0.05	31		>64	Boulder	0
0.25	3	0.03	28		16 - 64	coarse gravel	11
0.177	4	0.04	24		8 - 16	medium gravel	14
0.149	3	0.03	21		2 - 8	fine gravel	21
0.074	7	0.07	14		0.5 - 2	coarse sand	8
					0.25 - 0.5	medium sand	18
					0.063 - 0.25	fine sand	14
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-4 0-10Mass Sample (g): 100 T (oC) 20

Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.158E-02	.158E-04	1.37	
Hazen K (cm/s) = d_{10} (mm)	.279E-02	.279E-04	2.41	
Slichter	.311E-03	.311E-05	0.27	
Terzaghi	.443E-03	.443E-05	0.38	1.26
Beyer	.129E-02	.129E-04	1.11	
Sauerbrei	.132E-02	.132E-04	1.14	3.74
Kruger	.853E-01	.853E-03	73.67	
Kozeny-Carmen	.896E-01	.896E-03	77.39	253.84
Zunker	.694E-01	.694E-03	59.92	196.55
Zamarin	.832E-01	.832E-03	71.86	235.71
USBR	.504E-02	.504E-04	4.35	
Barr	.333E-03	.333E-05	0.29	0.94
Alyamani and Sen	.977E-01	.977E-03	84.44	276.97
Chapuis	.159E-03	.159E-05	0.14	
Krumbein and Monk	.334E-01	.334E-03	28.88	94.72
geometric mean	.116E-01	.116E-03	10.03	32.89
arithmetic mean	.469E-01	.469E-03	40.54	132.97



Grain Size Analysis Report

Date:

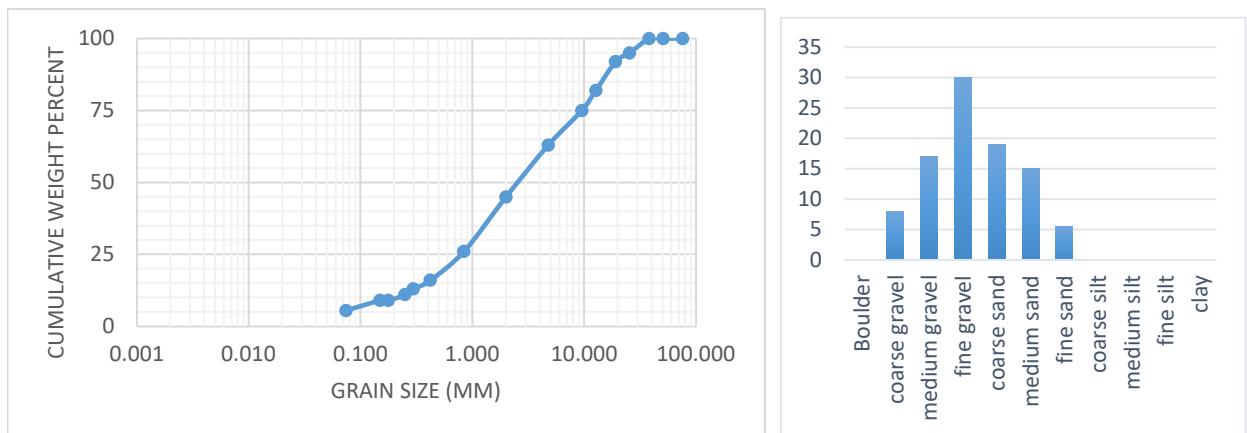
9/12/2022

Sample Name: B-4 10-16

Mass Sample (g): 100

T (oC) 20

Poorly sorted sandy gravel low in fines



Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.214	Uniformity Coef.	20.14
76.2	0	0	100	d17	0.462	n computed	0.26
50.8	0	0	100	d20	0.588	g (cm/s ²)	980.00
38.1	0	0	100	d50	2.767	ρ (g/cm ³)	0.9981
25.4	5	0.05	95	d60	4.300	μ (g/cm s)	0.0098
19.05	3	0.03	92	de (Kruger)	1.061	ρg/μ (1/cm s)	9.9327E+04
12.7	10	0.1	82	de (Kozeny)	0.954	tau (Sauerbrei)	1.053
9.525	7	0.07	75	de (Zunker)	0.989	d _{geometric mean}	2.634
4.76	12	0.12	63	de (Zamarin)	1.025	σ _ϕ	2.561
2	18	0.18	45	lo (Alyameni)	-0.425		
0.841	19	0.19	26		mm	0	% in sample
0.42	10	0.1	16		>64	Boulder	0
0.297	3	0.03	13		16 - 64	coarse gravel	8
0.25	2	0.02	11		8 - 16	medium gravel	17
0.177	2	0.02	9		2 - 8	fine gravel	30
0.149	0	0	9		0.5 - 2	coarse sand	19
0.074	3.5	0.035	5.5		0.25 - 0.5	medium sand	15
					0.063 - 0.25	fine sand	5.5
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



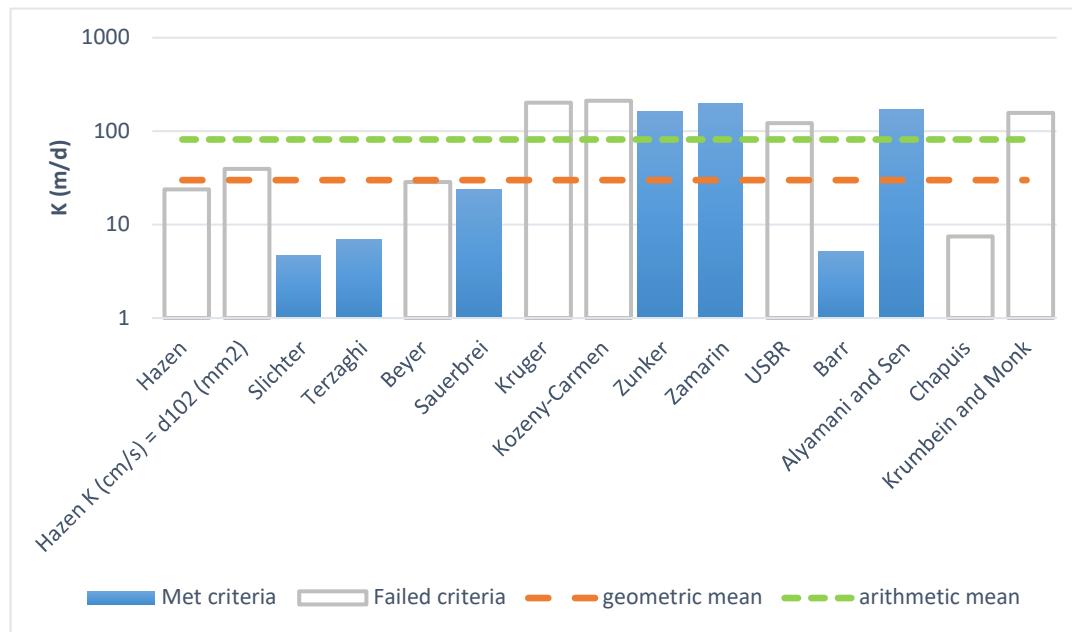
K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-4 10-16

Mass Sample (g):

100T (oC) 20

Poorly sorted sandy gravel low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.274E-01	.274E-03	23.70	
Hazen K (cm/s) = d_{10} (mm)	.456E-01	.456E-03	39.38	
Slichter	.547E-02	.547E-04	4.73	15.51
Terzaghi	.798E-02	.798E-04	6.90	22.62
Beyer	.328E-01	.328E-03	28.37	
Sauerbrei	.273E-01	.273E-03	23.55	77.25
Kruger	.233E+00	.233E-02	200.89	
Kozeny-Carmen	.244E+00	.244E-02	211.04	
Zunker	.188E+00	.188E-02	162.12	531.75
Zamarin	.229E+00	.229E-02	198.12	649.84
USBR	.141E+00	.141E-02	121.64	
Barr	.592E-02	.592E-04	5.12	16.79
Alyamani and Sen	.196E+00	.196E-02	169.38	555.57
Chapuis	.865E-02	.865E-04	7.47	
Krumbein and Monk	.180E+00	.180E-02	155.95	
geometric mean	.346E-01	.346E-03	29.90	98.09
arithmetic mean	.942E-01	.942E-03	81.42	267.05



Grain Size Analysis Report

Date:

9/12/2022

Sample Name:

B-4 16

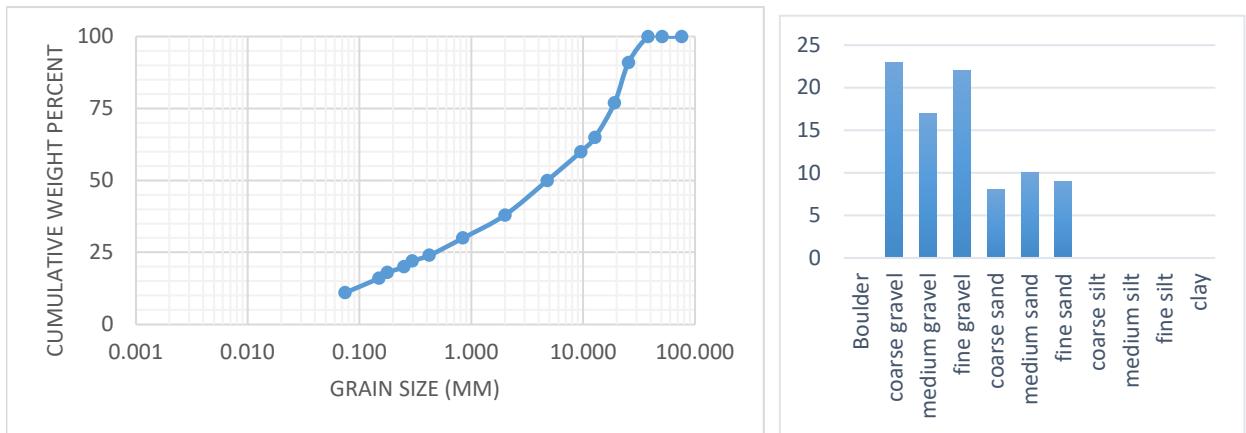
Mass Sample (g):

100

T (°C)

20

Poorly sorted sandy gravel low in fines



Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.067	Uniformity Coef.	141.59
76.2	0	0	100	d17	0.163	n computed	0.26
50.8	0	0	100	d20	0.250	g (cm/s ²)	980.00
38.1	0	0	100	d50	4.760	ρ (g/cm ³)	0.9981
25.4	9	0.09	91	d60	9.525	μ (g/cm s)	0.0098
19.05	14	0.14	77	de (Kruger)	0.984	ρg/μ (1/cm s)	9.9327E+04
12.7	12	0.12	65	de (Kozeny)	0.899	tau (Sauerbrei)	1.053
9.525	5	0.05	60	de (Zunker)	0.927	d _{geometric mean}	3.596
4.76	10	0.1	50	de (Zamarin)	0.956	σ _ϕ	3.298
2	12	0.12	38	lo (Alyameni)	-1.106		
0.841	8	0.08	30		mm	0	% in sample
0.42	6	0.06	24		>64	Boulder	0
0.297	2	0.02	22		16 - 64	coarse gravel	23
0.25	2	0.02	20		8 - 16	medium gravel	17
0.177	2	0.02	18		2 - 8	fine gravel	22
0.149	2	0.02	16		0.5 - 2	coarse sand	8
0.074	5	0.05	11		0.25 - 0.5	medium sand	10
					0.063 - 0.25	fine sand	9
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



K from Grain Size Analysis Report

Date: 9/12/2022

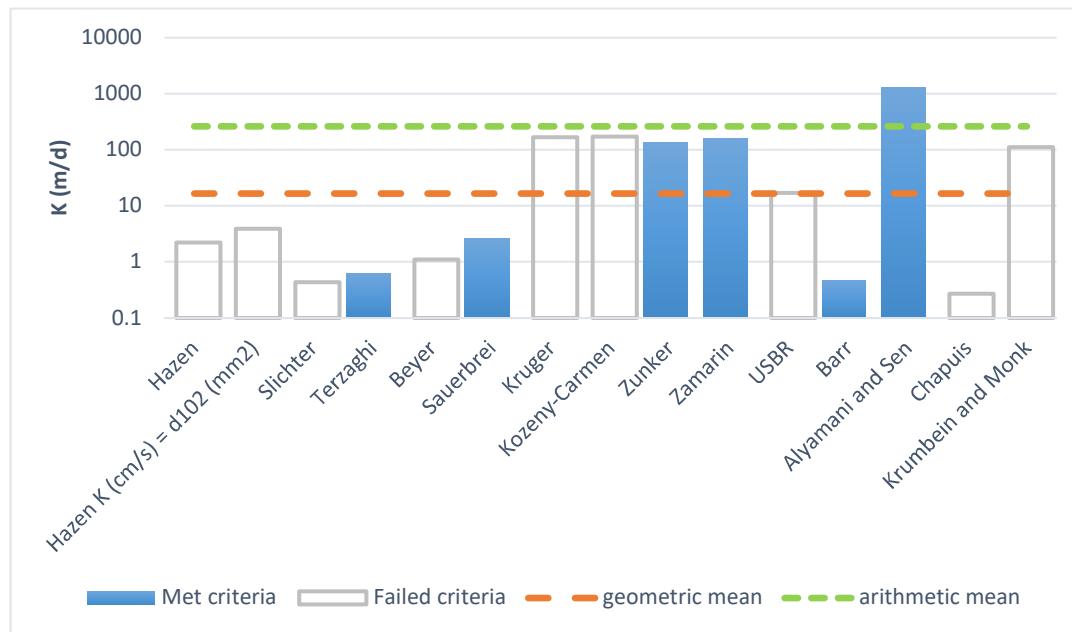
Sample Name:

B-4 16

Mass Sample (g):

100T (oC) 20

Poorly sorted sandy gravel low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.256E-02	.256E-04	2.21	
Hazen K (cm/s) = d ₁₀ (mm)	.453E-02	.453E-04	3.91	
Slichter	.504E-03	.504E-05	0.44	
Terzaghi	.718E-03	.718E-05	0.62	2.03
Beyer	.128E-02	.128E-04	1.11	
Sauerbrei	.311E-02	.311E-04	2.69	8.82
Kruger	.192E+00	.192E-02	166.19	
Kozeny-Carmen	.199E+00	.199E-02	172.17	
Zunker	.155E+00	.155E-02	133.92	439.27
Zamarin	.187E+00	.187E-02	161.38	529.31
USBR	.197E-01	.197E-03	16.99	
Barr	.540E-03	.540E-05	0.47	1.53
Alyamani and Sen	.147E+01	.147E-01	1270.51	4167.26
Chapuis	.313E-03	.313E-05	0.27	
Krumbein and Monk	.128E+00	.128E-02	110.70	
geometric mean	.193E-01	.193E-03	16.66	54.64
arithmetic mean	.303E+00	.303E-02	261.60	858.04



Grain Size Analysis Report

Date:

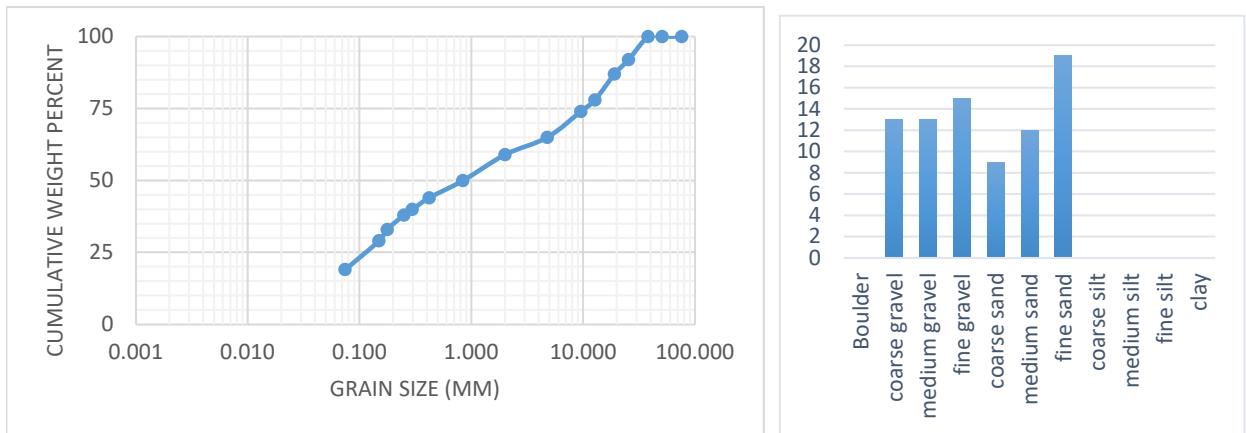
9/12/2022

Sample Name: B-6 5-12

Mass Sample (g): 100

T (°C) 20

Poorly sorted gravelly sand low in fines



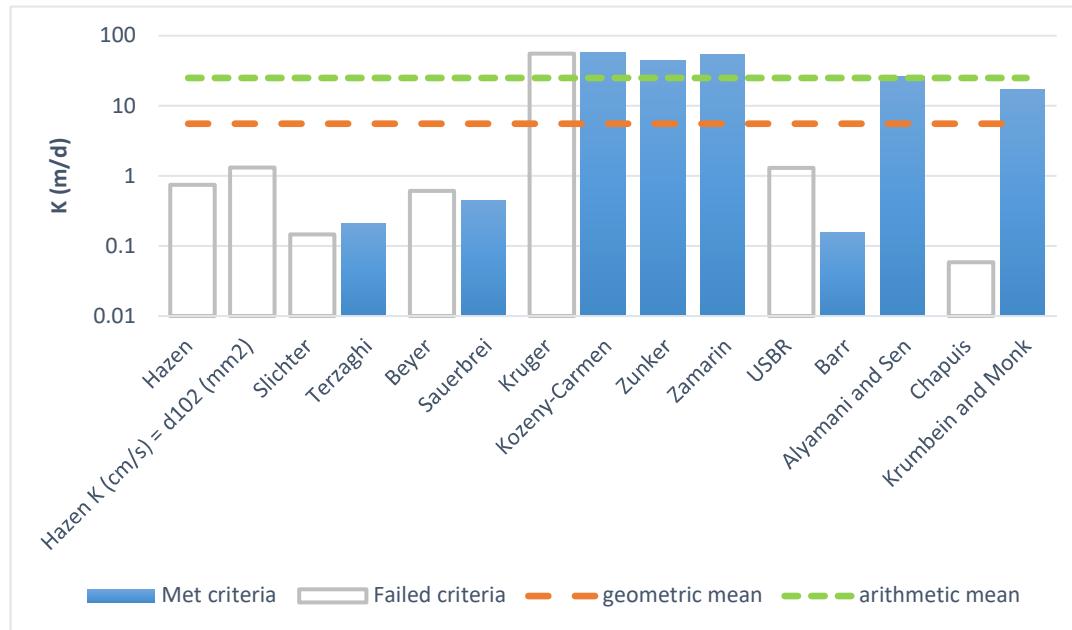
Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.039	Uniformity Coef.	63.16
76.2	0	0	100	d17	0.066	n computed	0.26
50.8	0	0	100	d20	0.082	g (cm/s ²)	980.00
38.1	0	0	100	d50	0.841	ρ (g/cm ³)	0.9981
25.4	8	0.08	92	d60	2.460	μ (g/cm s)	0.0098
19.05	5	0.05	87	de (Kruger)	0.567	ρg/μ (1/cm s)	9.9327E+04
12.7	9	0.09	78	de (Kozeny)	0.520	tau (Sauerbrei)	1.053
9.525	4	0.04	74	de (Zunker)	0.536	d _{geometric mean}	1.751
4.76	9	0.09	65	de (Zamarin)	0.551	σ _ϕ	3.627
2	6	0.06	59	Io (Alyameni)	-0.162		
0.841	9	0.09	50		mm	0	% in sample
0.42	6	0.06	44		>64	Boulder	0
0.297	4	0.04	40		16 - 64	coarse gravel	13
0.25	2	0.02	38		8 - 16	medium gravel	13
0.177	5	0.05	33		2 - 8	fine gravel	15
0.149	4	0.04	29		0.5 - 2	coarse sand	9
0.074	10	0.1	19		0.25 - 0.5	medium sand	12
					0.063 - 0.25	fine sand	19
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-6 5-12Mass Sample (g): 100 T (oC) 20

Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.859E-03	.859E-05	0.74	
Hazen K (cm/s) = d_{10} (mm)	.152E-02	.152E-04	1.31	
Slichter	.169E-03	.169E-05	0.15	
Terzaghi	.241E-03	.241E-05	0.21	0.68
Beyer	.704E-03	.704E-05	0.61	
Sauerbrei	.514E-03	.514E-05	0.44	1.46
Kruger	.638E-01	.638E-03	55.12	
Kozeny-Carmen	.667E-01	.667E-03	57.64	189.06
Zunker	.517E-01	.517E-03	44.69	146.60
Zamarin	.621E-01	.621E-03	53.68	176.08
USBR	.149E-02	.149E-04	1.29	
Barr	.181E-03	.181E-05	0.16	0.51
Alyamani and Sen	.301E-01	.301E-03	26.03	85.39
Chapuis	.671E-04	.671E-06	0.06	
Krumbein and Monk	.197E-01	.197E-03	17.05	55.92
geometric mean	.641E-02	.641E-04	5.54	18.17
arithmetic mean	.289E-01	.289E-03	24.99	81.96



Grain Size Analysis Report

Date:

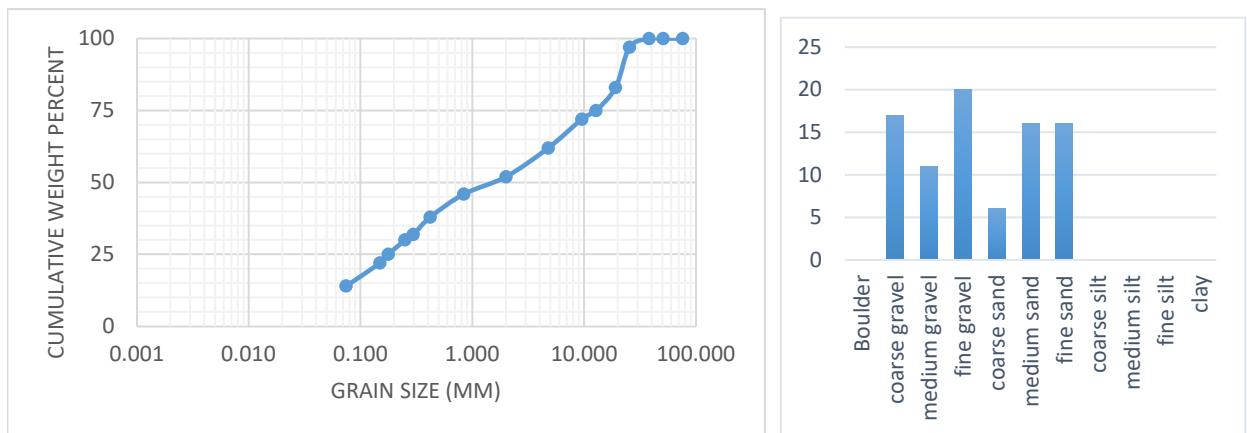
9/12/2022

Sample Name: B-6 10-15

Mass Sample (g): 100

T (°C) 20

Poorly sorted gravelly sand low in fines



Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.053	Uniformity Coef.	79.61
76.2	0	0	100	d17	0.102	n computed	0.26
50.8	0	0	100	d20	0.130	g (cm/s ²)	980.00
38.1	0	0	100	d50	1.614	ρ (g/cm ³)	0.9981
25.4	3	0.03	97	d60	4.208	μ (g/cm s)	0.0098
19.05	14	0.14	83	de (Kruger)	0.623	ρg/μ (1/cm s)	9.9327E+04
12.7	8	0.08	75	de (Kozeny)	0.574	tau (Sauerbrei)	1.053
9.525	3	0.03	72	de (Zunker)	0.590	d _{geometric mean}	2.021
4.76	10	0.1	62	de (Zamarin)	0.607	σ _ϕ	3.422
2	10	0.1	52	lo (Alyameni)	-0.337		
0.841	6	0.06	46		mm	0	% in sample
0.42	8	0.08	38		>64	Boulder	0
0.297	6	0.06	32		16 - 64	coarse gravel	17
0.25	2	0.02	30		8 - 16	medium gravel	11
0.177	5	0.05	25		2 - 8	fine gravel	20
0.149	3	0.03	22		0.5 - 2	coarse sand	6
0.074	8	0.08	14		0.25 - 0.5	medium sand	16
					0.063 - 0.25	fine sand	16
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



Grain Size Analysis Report

Date:

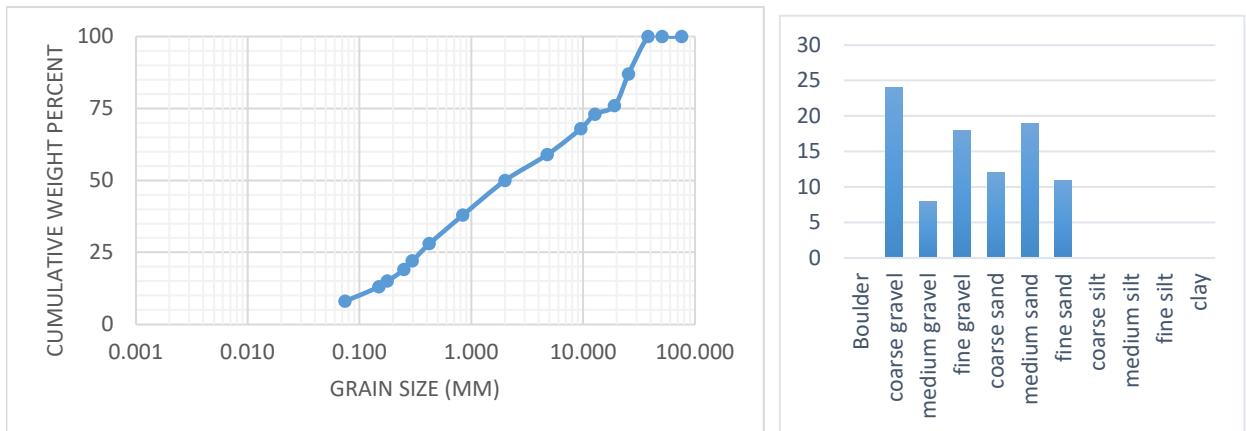
9/12/2022

Sample Name: B-6 15+

Mass Sample (g): 100

T (°C) 20

Poorly sorted sandy gravel low in fines



Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.103	Uniformity Coef.	51.31
76.2	0	0	100	d17	0.214	n computed	0.26
50.8	0	0	100	d20	0.266	g (cm/s ²)	980.00
38.1	0	0	100	d50	2.000	ρ (g/cm ³)	0.9981
25.4	13	0.13	87	d60	5.289	μ (g/cm s)	0.0098
19.05	11	0.11	76	de (Kruger)	0.755	ρg/μ (1/cm s)	9.9327E+04
12.7	3	0.03	73	de (Kozeny)	0.696	tau (Sauerbrei)	1.053
9.525	5	0.05	68	de (Zunker)	0.715	d _{geometric mean}	2.536
4.76	9	0.09	59	de (Zamarin)	0.735	σ _ϕ	3.171
2	9	0.09	50	lo (Alyameni)	-0.371		
0.841	12	0.12	38		mm	0	% in sample
0.42	10	0.1	28		>64	Boulder	0
0.297	6	0.06	22		16 - 64	coarse gravel	24
0.25	3	0.03	19		8 - 16	medium gravel	8
0.177	4	0.04	15		2 - 8	fine gravel	18
0.149	2	0.02	13		0.5 - 2	coarse sand	12
0.074	4.9	0.049	8.1		0.25 - 0.5	medium sand	19
					0.063 - 0.25	fine sand	10.9
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



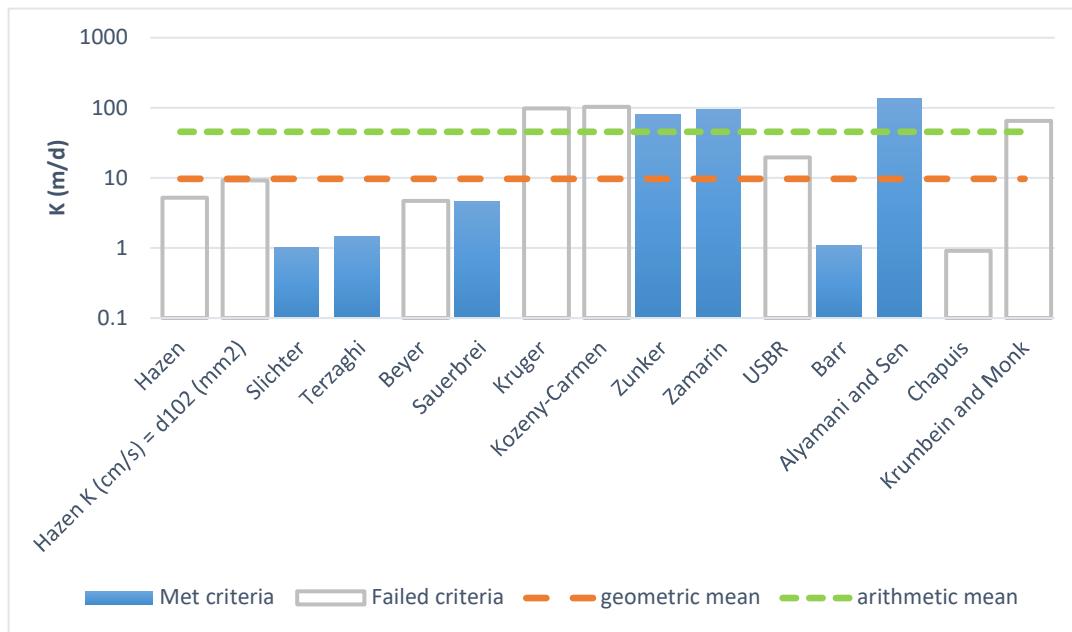
K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-6 15+Mass Sample (g): 100

T (oC)

20

Poorly sorted sandy gravel low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.602E-02	.602E-04	5.20	
Hazen $K(\text{cm/s}) = d_{10} (\text{mm})$.106E-01	.106E-03	9.18	
Slichter	.118E-02	.118E-04	1.02	3.35
Terzaghi	.169E-02	.169E-04	1.46	4.78
Beyer	.543E-02	.543E-04	4.69	
Sauerbrei	.534E-02	.534E-04	4.62	15.14
Kruger	.113E+00	.113E-02	97.81	
Kozeny-Carmen	.119E+00	.119E-02	103.09	
Zunker	.923E-01	.923E-03	79.73	261.53
Zamarin	.111E+00	.111E-02	95.52	313.31
USBR	.226E-01	.226E-03	19.53	
Barr	.127E-02	.127E-04	1.10	3.59
Alyamani and Sen	.158E+00	.158E-02	136.24	446.86
Chapuis	.104E-02	.104E-04	0.90	
Krumbein and Monk	.753E-01	.753E-03	65.04	
geometric mean	.112E-01	.112E-03	9.65	31.66
arithmetic mean	.529E-01	.529E-03	45.67	149.79



Grain Size Analysis Report

Date:

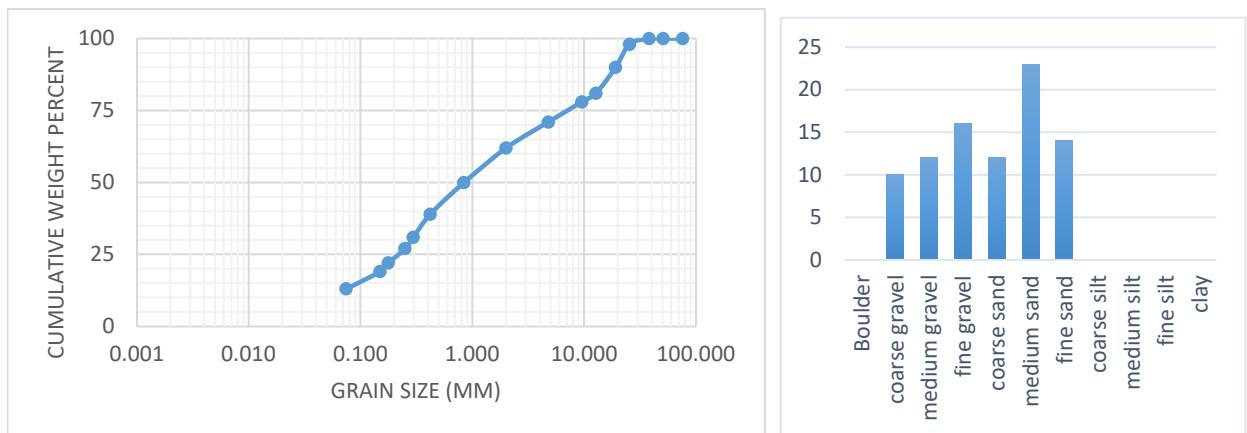
9/12/2022

Sample Name: B-9 0-9

Mass Sample (g): 100

T (oC) 20

Poorly sorted gravelly sand low in fines



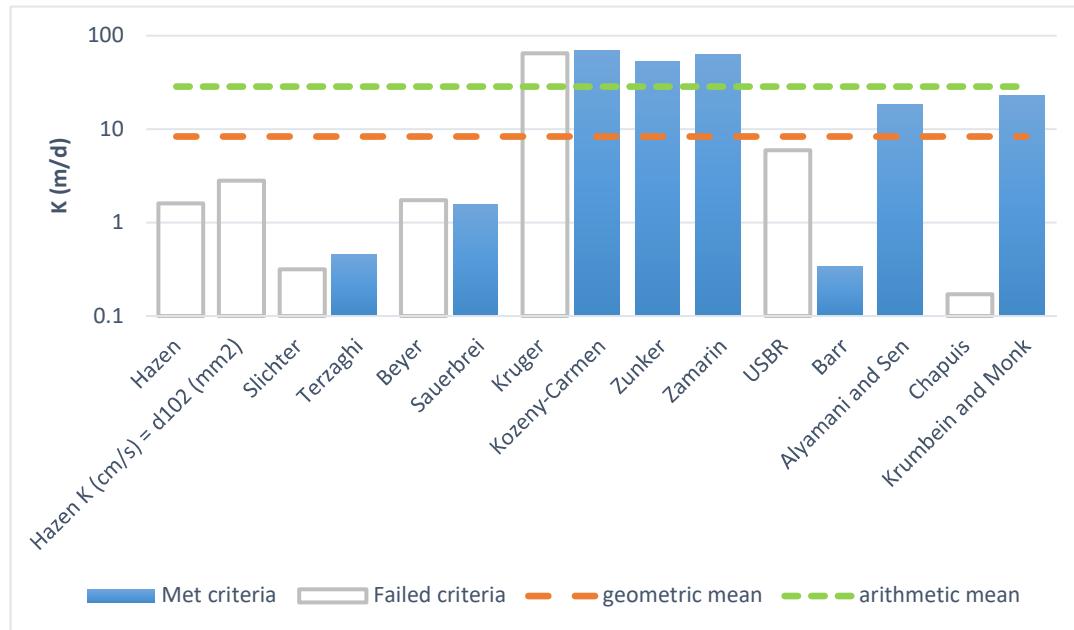
Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
76.2	0	0	100	d10	0.057	Uniformity Coef.	31.74
50.8	0	0	100	d17	0.124	n computed	0.26
38.1	0	0	100	d20	0.158	g (cm/s ²)	980.00
25.4	2	0.02	98	d50	0.841	ρ (g/cm ³)	0.9981
19.05	8	0.08	90	d60	1.807	μ (g/cm s)	0.0098
12.7	9	0.09	81	de (Kruger)	0.612	ρg/μ (1/cm s)	9.9327E+04
9.525	3	0.03	78	de (Kozeny)	0.567	tau (Sauerbrei)	1.053
4.76	7	0.07	71	de (Zunker)	0.582	d _{geometric mean}	1.554
2	9	0.09	62	de (Zamarin)	0.597	σ _ϕ	3.227
0.841	12	0.12	50	lo (Alyameni)	-0.139		
0.42	11	0.11	39	mm		0	% in sample
0.297	8	0.08	31	>64		Boulder	0
0.25	4	0.04	27	16 - 64		coarse gravel	10
0.177	5	0.05	22	8 - 16		medium gravel	12
0.149	3	0.03	19	2 - 8		fine gravel	16
0.074	6	0.06	13	0.5 - 2		coarse sand	12
				0.25 - 0.5		medium sand	23
				0.063 - 0.25		fine sand	14
				0.016 - 0.063		coarse silt	
				0.008 - 0.016		medium silt	
				0.002 - 0.008		fine silt	
				<0.002		clay	



K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-9 0-9Mass Sample (g): 100 T (oC) 20

Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.185E-02	.185E-04	1.60	
Hazen K (cm/s) = d ₁₀ (mm)	.324E-02	.324E-04	2.80	
Slichter	.364E-03	.364E-05	0.31	
Terzaghi	.520E-03	.520E-05	0.45	1.47
Beyer	.200E-02	.200E-04	1.73	
Sauerbrei	.182E-02	.182E-04	1.57	5.16
Kruger	.747E-01	.747E-03	64.55	
Kozeny-Carmen	.799E-01	.799E-03	69.04	226.46
Zunker	.615E-01	.615E-03	53.10	174.15
Zamarin	.734E-01	.734E-03	63.43	208.05
USBR	.688E-02	.688E-04	5.94	
Barr	.390E-03	.390E-05	0.34	1.11
Alyamani and Sen	.215E-01	.215E-03	18.56	60.89
Chapuis	.198E-03	.198E-05	0.17	
Krumbein and Monk	.262E-01	.262E-03	22.68	74.38
geometric mean	.965E-02	.965E-04	8.34	27.34
arithmetic mean	.332E-01	.332E-03	28.65	93.96



Grain Size Analysis Report

Date:

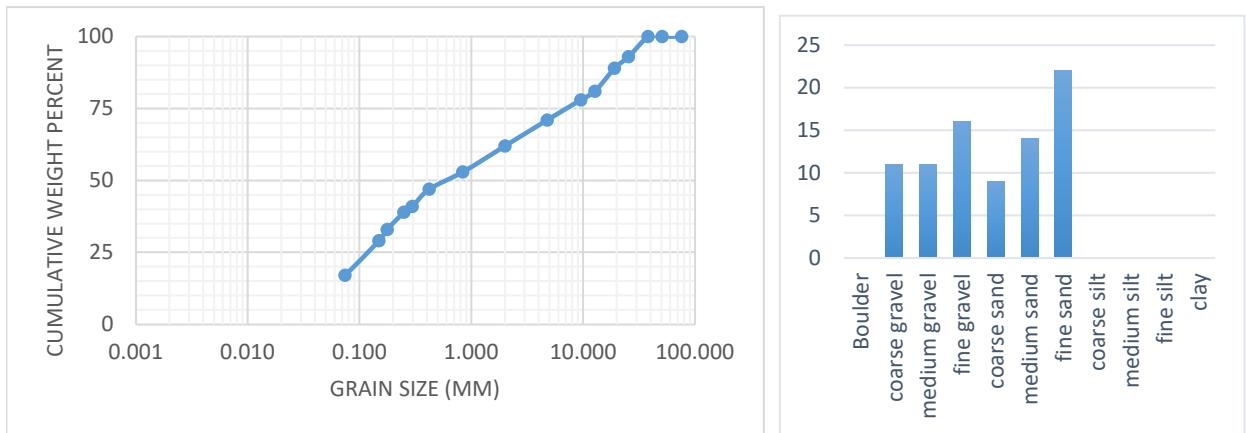
9/12/2022

Sample Name: B-9 1st

Mass Sample (g): 100

T (°C) 20

Poorly sorted gravelly sand low in fines



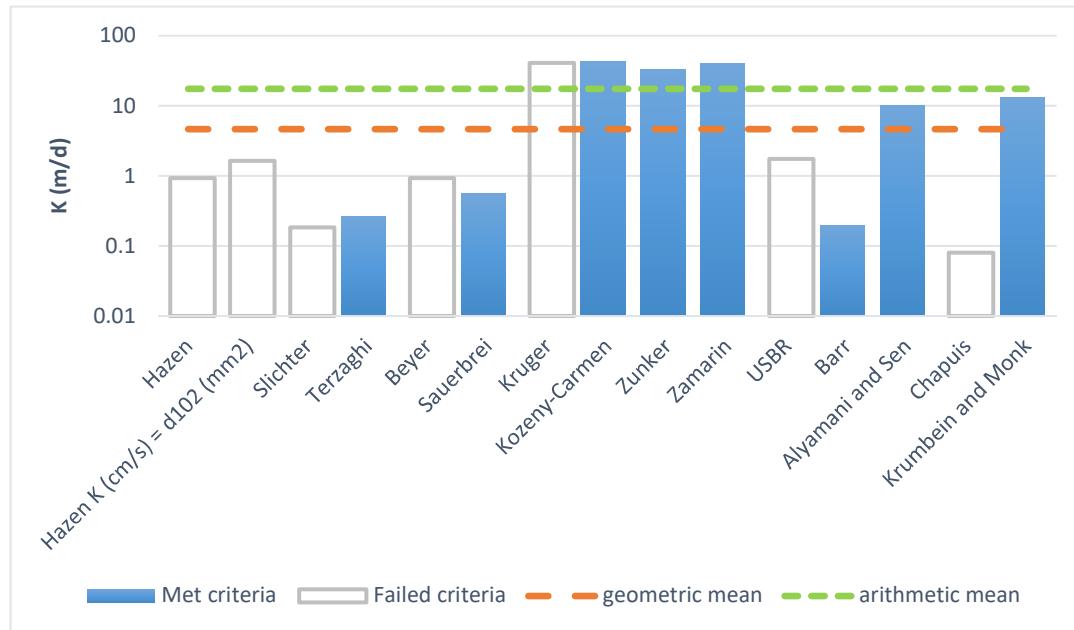
Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.044	Uniformity Coef.	40.03
76.2	0	0	100	d17	0.074	n computed	0.26
50.8	0	0	100	d20	0.093	g (cm/s ²)	980.00
38.1	0	0	100	d50	0.631	ρ (g/cm ³)	0.9981
25.4	7	0.07	93	d60	1.742	μ (g/cm s)	0.0098
19.05	4	0.04	89	de (Kruger)	0.488	ρg/μ (1/cm s)	9.9327E+04
12.7	8	0.08	81	de (Kozeny)	0.448	tau (Sauerbrei)	1.053
9.525	3	0.03	78	de (Zunker)	0.461	d _{geometric mean}	1.430
4.76	7	0.07	71	de (Zamarin)	0.474	σ _ϕ	3.513
2	9	0.09	62	lo (Alyameni)	-0.103		
0.841	9	0.09	53		mm	0	% in sample
0.42	6	0.06	47		>64	Boulder	0
0.297	6	0.06	41		16 - 64	coarse gravel	11
0.25	2	0.02	39		8 - 16	medium gravel	11
0.177	6	0.06	33		2 - 8	fine gravel	16
0.149	4	0.04	29		0.5 - 2	coarse sand	9
0.074	12	0.12	17		0.25 - 0.5	medium sand	14
					0.063 - 0.25	fine sand	22
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-9 1stMass Sample (g): 100 T (oC) 20

Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.107E-02	.107E-04	0.93	
Hazen $K \text{ (cm/s)} = d_{10} \text{ (mm)}$.189E-02	.189E-04	1.64	
Slichter	.211E-03	.211E-05	0.18	
Terzaghi	.301E-03	.301E-05	0.26	0.85
Beyer	.107E-02	.107E-04	0.93	
Sauerbrei	.643E-03	.643E-05	0.56	1.82
Kruger	.473E-01	.473E-03	40.86	
Kozeny-Carmen	.494E-01	.494E-03	42.71	140.08
Zunker	.383E-01	.383E-03	33.11	108.62
Zamarin	.461E-01	.461E-03	39.80	130.54
USBR	.201E-02	.201E-04	1.74	
Barr	.227E-03	.227E-05	0.20	0.64
Alyamani and Sen	.118E-01	.118E-03	10.19	33.43
Chapuis	.921E-04	.921E-06	0.08	
Krumbein and Monk	.153E-01	.153E-03	13.21	43.34
geometric mean	.537E-02	.537E-04	4.64	15.22
arithmetic mean	.203E-01	.203E-03	17.50	57.41



Grain Size Analysis Report

Date:

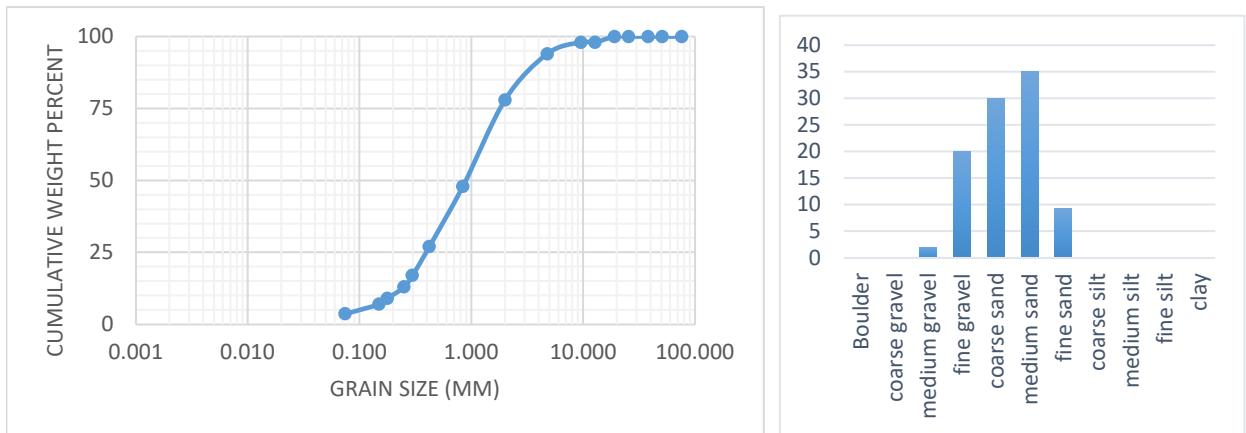
9/12/2022

Sample Name: B-9 9-14.5

Mass Sample (g): 100

T (oC) 20

Poorly sorted gravelly sand low in fines



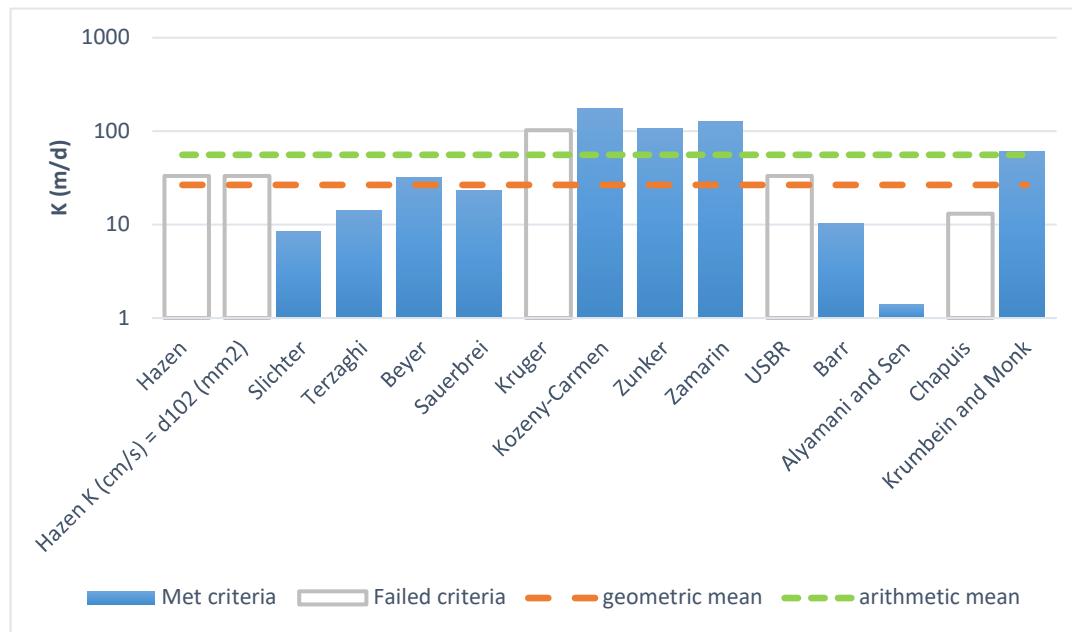
Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.195	Uniformity Coef.	6.68
76.2	0	0	100	d17	0.297	n computed	0.33
50.8	0	0	100	d20	0.334	g (cm/s ²)	980.00
38.1	0	0	100	d50	0.918	ρ (g/cm ³)	0.9981
25.4	0	0	100	d60	1.305	μ (g/cm s)	0.0098
19.05	0	0	100	de (Kruger)	0.614	ρg/μ (1/cm s)	9.9327E+04
12.7	2	0.02	98	de (Kozeny)	0.563	tau (Sauerbrei)	1.053
9.525	0	0	98	de (Zunker)	0.579	d _{geometric mean}	0.952
4.76	4	0.04	94	de (Zamarin)	0.596	σ _ϕ	1.738
2	16	0.16	78	lo (Alyameni)	0.014		
0.841	30	0.3	48		mm	0	% in sample
0.42	21	0.21	27		>64	Boulder	0
0.297	10	0.1	17		16 - 64	coarse gravel	0
0.25	4	0.04	13		8 - 16	medium gravel	2
0.177	4	0.04	9		2 - 8	fine gravel	20
0.149	2	0.02	7		0.5 - 2	coarse sand	30
0.074	3.3	0.033	3.7		0.25 - 0.5	medium sand	35
					0.063 - 0.25	fine sand	9.3
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-9 9-14.5Mass Sample (g): 100 T (oC) 20

Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.383E-01	.383E-03	33.06	
Hazen $K \text{ (cm/s)} = d_{10} \text{ (mm)}$.381E-01	.381E-03	32.94	
Slichter	.974E-02	.974E-04	8.42	27.62
Terzaghi	.163E-01	.163E-03	14.11	46.28
Beyer	.369E-01	.369E-03	31.88	104.58
Sauerbrei	.272E-01	.272E-03	23.48	77.01
Kruger	.118E+00	.118E-02	102.36	
Kozeny-Carmen	.205E+00	.205E-02	177.04	580.69
Zunker	.124E+00	.124E-02	106.71	350.01
Zamarin	.147E+00	.147E-02	126.98	416.51
USBR	.382E-01	.382E-03	33.05	
Barr	.120E-01	.120E-03	10.33	33.88
Alyamani and Sen	.160E-02	.160E-04	1.38	4.52
Chapuis	.151E-01	.151E-03	13.01	
Krumbein and Monk	.693E-01	.693E-03	59.85	196.31
geometric mean	.309E-01	.309E-03	26.67	87.47
arithmetic mean	.648E-01	.648E-03	56.02	183.74



Grain Size Analysis Report

Date:

9/12/2022

Sample Name: B-11 5-10

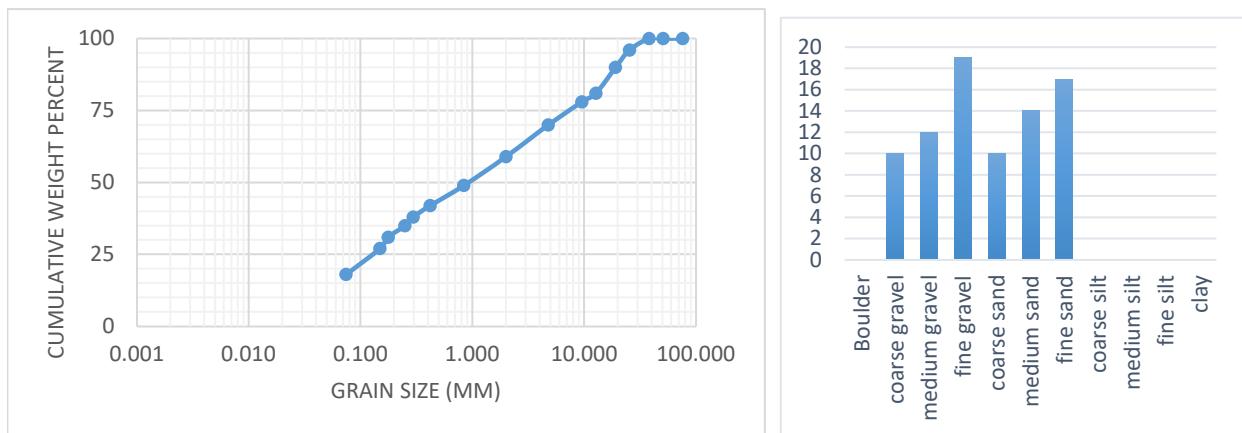
Mass Sample (g):

100

T (°C)

20

Poorly sorted gravelly sand low in fines



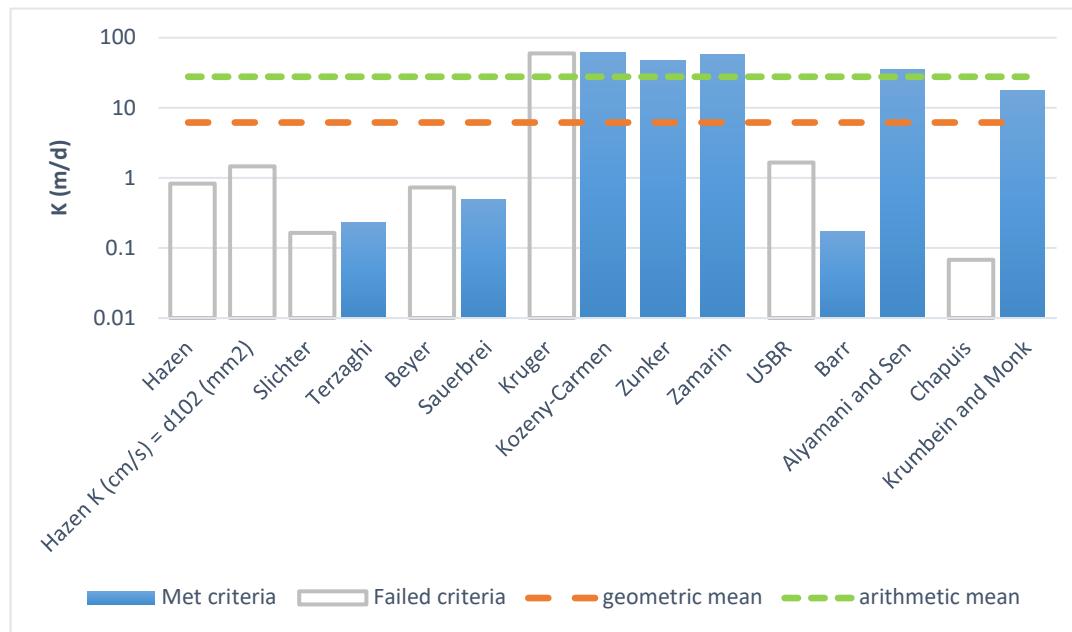
Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.041	Uniformity Coef.	54.75
76.2	0	0	100	d17	0.070	n computed	0.26
50.8	0	0	100	d20	0.091	g (cm/s ²)	980.00
38.1	0	0	100	d50	0.957	ρ (g/cm ³)	0.9981
25.4	4	0.04	96	d60	2.251	μ (g/cm s)	0.0098
19.05	6	0.06	90	de (Kruger)	0.589	ρg/μ (1/cm s)	9.9327E+04
12.7	9	0.09	81	de (Kozeny)	0.541	tau (Sauerbrei)	1.053
9.525	3	0.03	78	de (Zunker)	0.556	d _{geometric mean}	1.631
4.76	8	0.08	70	de (Zamarin)	0.573	σ _ϕ	3.501
2	11	0.11	59	lo (Alyameni)	-0.188		
0.841	10	0.1	49				
0.42	7	0.07	42				
0.297	4	0.04	38				
0.25	3	0.03	35				
0.177	4	0.04	31				
0.149	4	0.04	27				
0.074	9	0.09	18				
				mm	0	% in sample	
				>64	Boulder	0	
				16 - 64	coarse gravel	10	
				8 - 16	medium gravel	12	
				2 - 8	fine gravel	19	
				0.5 - 2	coarse sand	10	
				0.25 - 0.5	medium sand	14	
				0.063 - 0.25	fine sand	17	
				0.016 - 0.063	coarse silt		
				0.008 - 0.016	medium silt		
				0.002 - 0.008	fine silt		
				<0.002	clay		



K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-11 5-10Mass Sample (g): 100 T (oC) 20

Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.957E-03	.957E-05	0.83	
Hazen $K \text{ (cm/s)} = d_{10} \text{ (mm)}$.169E-02	.169E-04	1.46	
Slichter	.188E-03	.188E-05	0.16	
Terzaghi	.268E-03	.268E-05	0.23	0.76
Beyer	.839E-03	.839E-05	0.72	
Sauerbrei	.572E-03	.572E-05	0.49	1.62
Kruger	.688E-01	.688E-03	59.44	
Kozeny-Carmen	.720E-01	.720E-03	62.21	204.06
Zunker	.558E-01	.558E-03	48.23	158.19
Zamarin	.670E-01	.670E-03	57.91	189.96
USBR	.191E-02	.191E-04	1.65	
Barr	.202E-03	.202E-05	0.17	0.57
Alyamani and Sen	.409E-01	.409E-03	35.37	116.00
Chapuis	.782E-04	.782E-06	0.07	
Krumbein and Monk	.202E-01	.202E-03	17.46	57.28
geometric mean	.716E-02	.716E-04	6.19	20.29
arithmetic mean	.321E-01	.321E-03	27.76	91.05



Grain Size Analysis Report

Date:

9/12/2022

Sample Name: B-11 10-14

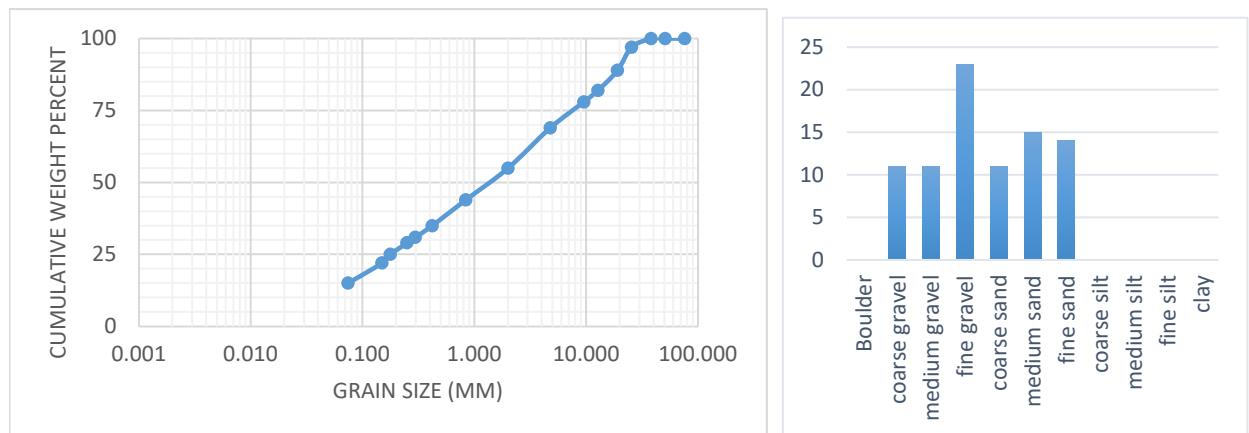
Mass Sample (g):

100

T (°C)

20

Poorly sorted gravelly sand low in fines



Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.049	Uniformity Coef.	60.52
76.2	0	0	100	d17	0.095	n computed	0.26
50.8	0	0	100	d20	0.128	g (cm/s ²)	980.00
38.1	0	0	100	d50	1.473	ρ (g/cm ³)	0.9981
25.4	3	0.03	97	d60	2.986	μ (g/cm s)	0.0098
19.05	8	0.08	89	de (Kruger)	0.680	ρg/μ (1/cm s)	9.9327E+04
12.7	7	0.07	82	de (Kozeny)	0.623	tau (Sauerbrei)	1.053
9.525	4	0.04	78	de (Zunker)	0.641	d _{geometric mean}	1.835
4.76	9	0.09	69	de (Zamarin)	0.661	σ _ϕ	3.357
2	14	0.14	55	lo (Alyameni)	-0.307		
0.841	11	0.11	44		mm	0	% in sample
0.42	9	0.09	35		>64	Boulder	0
0.297	4	0.04	31		16 - 64	coarse gravel	11
0.25	2	0.02	29		8 - 16	medium gravel	11
0.177	4	0.04	25		2 - 8	fine gravel	23
0.149	3	0.03	22		0.5 - 2	coarse sand	11
0.074	7	0.07	15		0.25 - 0.5	medium sand	15
					0.063 - 0.25	fine sand	14
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



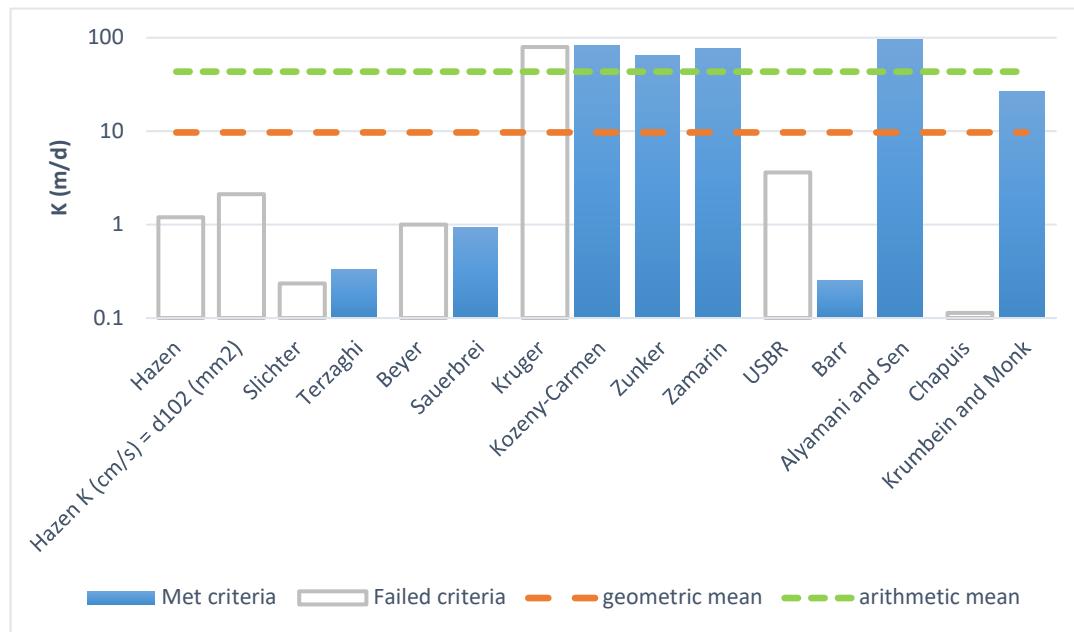
K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-11 10-14Mass Sample (g): 100

T (oC)

20

Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.138E-02	.138E-04	1.19	
Hazen K (cm/s) = d ₁₀ (mm)	.243E-02	.243E-04	2.10	
Slichter	.271E-03	.271E-05	0.23	
Terzaghi	.386E-03	.386E-05	0.33	1.09
Beyer	.115E-02	.115E-04	1.00	
Sauerbrei	.107E-02	.107E-04	0.92	3.02
Kruger	.918E-01	.918E-03	79.30	
Kozeny-Carmen	.956E-01	.956E-03	82.58	270.87
Zunker	.742E-01	.742E-03	64.13	210.34
Zamarin	.893E-01	.893E-03	77.14	253.01
USBR	.418E-02	.418E-04	3.61	
Barr	.290E-03	.290E-05	0.25	0.82
Alyamani and Sen	.111E+00	.111E-02	95.50	313.23
Chapuis	.131E-03	.131E-05	0.11	
Krumbein and Monk	.309E-01	.309E-03	26.67	87.48
geometric mean	.113E-01	.113E-03	9.73	31.91
arithmetic mean	.503E-01	.503E-03	43.44	142.48



Grain Size Analysis Report

Date:

9/12/2022

Sample Name: B-11 10-14

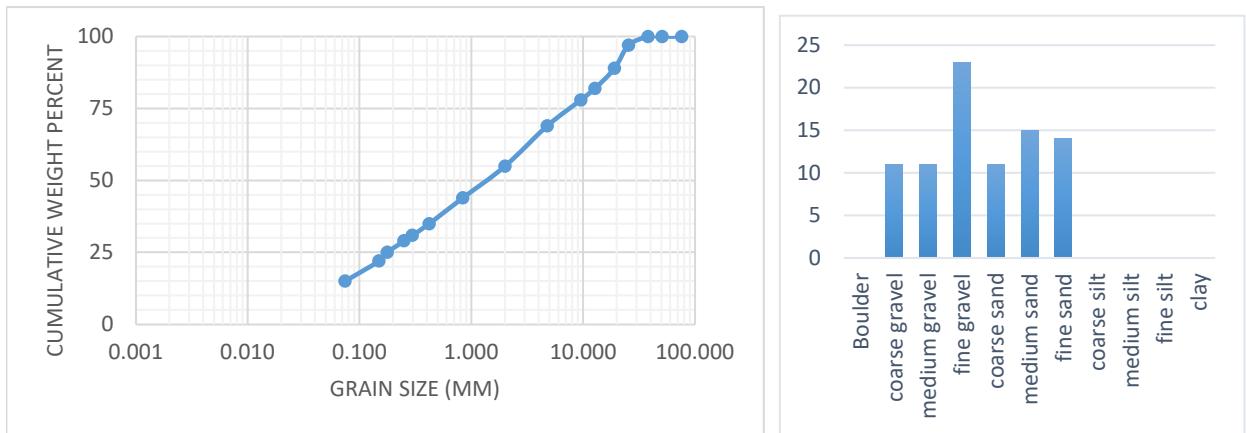
Mass Sample (g):

100

T (°C)

20

Poorly sorted gravelly sand low in fines



Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.049	Uniformity Coef.	60.52
76.2	0	0	100	d17	0.095	n computed	0.26
50.8	0	0	100	d20	0.128	g (cm/s ²)	980.00
38.1	0	0	100	d50	1.473	ρ (g/cm ³)	0.9981
25.4	3	0.03	97	d60	2.986	μ (g/cm s)	0.0098
19.05	8	0.08	89	de (Kruger)	0.680	ρg/μ (1/cm s)	9.9327E+04
12.7	7	0.07	82	de (Kozeny)	0.623	tau (Sauerbrei)	1.053
9.525	4	0.04	78	de (Zunker)	0.641	d _{geometric mean}	1.835
4.76	9	0.09	69	de (Zamarin)	0.661	σ _ϕ	3.357
2	14	0.14	55	lo (Alyameni)	-0.307		
0.841	11	0.11	44		mm	0	% in sample
0.42	9	0.09	35		>64	Boulder	0
0.297	4	0.04	31		16 - 64	coarse gravel	11
0.25	2	0.02	29		8 - 16	medium gravel	11
0.177	4	0.04	25		2 - 8	fine gravel	23
0.149	3	0.03	22		0.5 - 2	coarse sand	11
0.074	7	0.07	15		0.25 - 0.5	medium sand	15
					0.063 - 0.25	fine sand	14
					0.016 - 0.063	coarse silt	
					0.008 - 0.016	medium silt	
					0.002 - 0.008	fine silt	
					<0.002	clay	



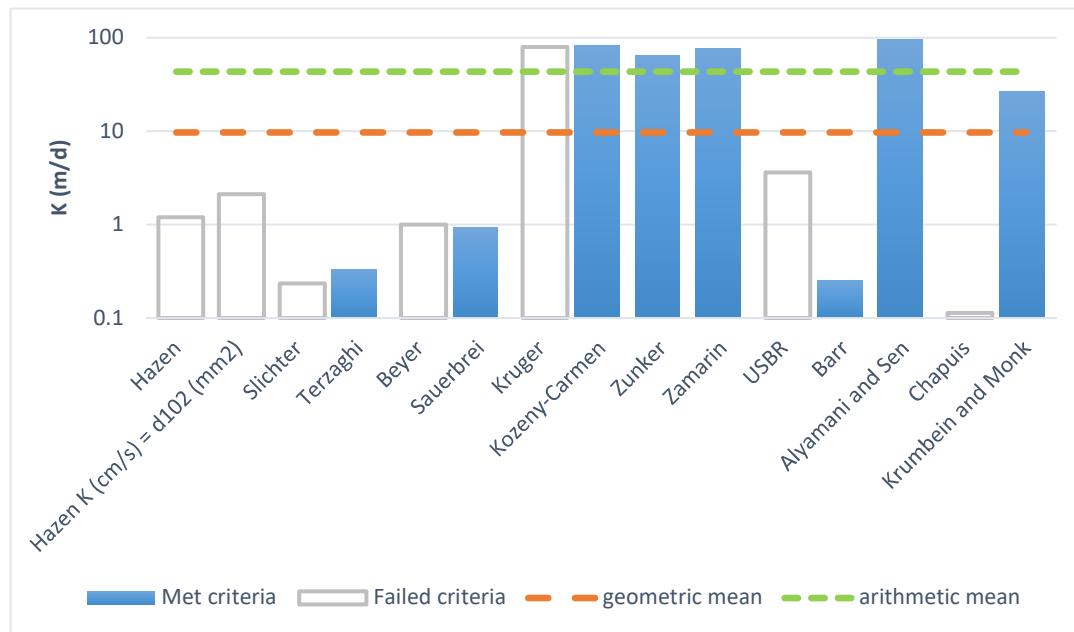
K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-11 10-14Mass Sample (g): 100

T (oC)

20

Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.138E-02	.138E-04	1.19	
Hazen K (cm/s) = d ₁₀ (mm)	.243E-02	.243E-04	2.10	
Slichter	.271E-03	.271E-05	0.23	
Terzaghi	.386E-03	.386E-05	0.33	1.09
Beyer	.115E-02	.115E-04	1.00	
Sauerbrei	.107E-02	.107E-04	0.92	3.02
Kruger	.918E-01	.918E-03	79.30	
Kozeny-Carmen	.956E-01	.956E-03	82.58	270.87
Zunker	.742E-01	.742E-03	64.13	210.34
Zamarin	.893E-01	.893E-03	77.14	253.01
USBR	.418E-02	.418E-04	3.61	
Barr	.290E-03	.290E-05	0.25	0.82
Alyamani and Sen	.111E+00	.111E-02	95.50	313.23
Chapuis	.131E-03	.131E-05	0.11	
Krumbein and Monk	.309E-01	.309E-03	26.67	87.48
geometric mean	.113E-01	.113E-03	9.73	31.91
arithmetic mean	.503E-01	.503E-03	43.44	142.48



Grain Size Analysis Report

Date:

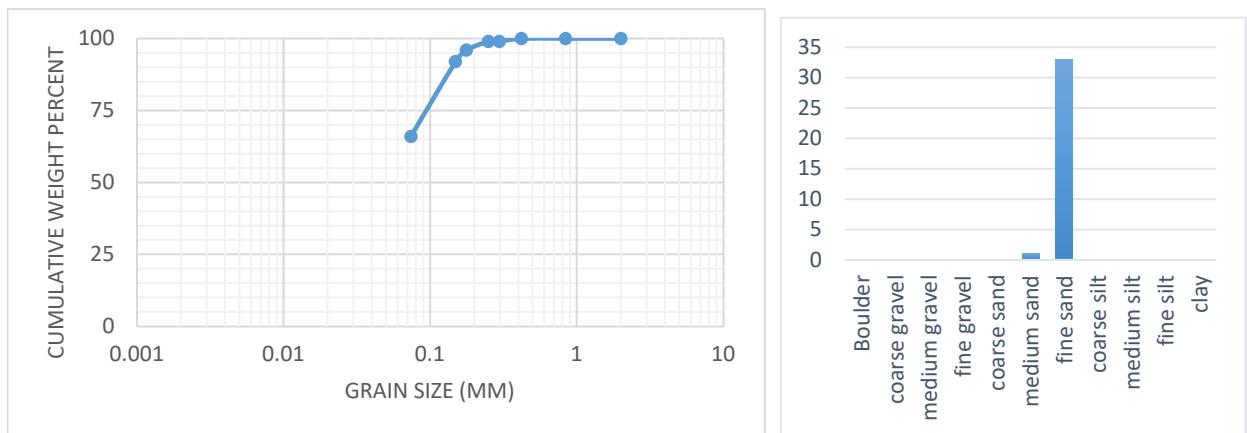
9/12/2022

Sample Name: B-13 from 5' to 15'

Mass Sample (g): 100

T (oC) 20

Poorly sorted sandy silt low in fines



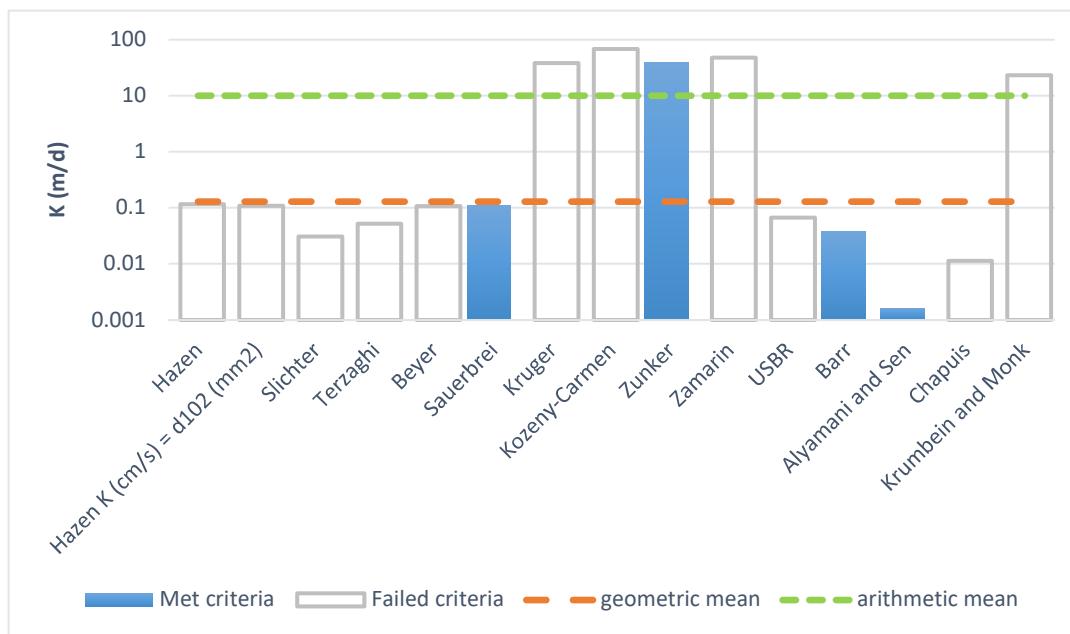
Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.011	Uniformity Coef.	6.00
2	0	0	100	d17	0.019	n computed	0.34
0.841	0	0	100	d20	0.022	g (cm/s ²)	980.00
0.42	0	0	100	d50	0.056	ρ (g/cm ³)	0.9981
0.297	1	0.01	99	d60	0.067	μ (g/cm s)	0.0098
0.25	0	0	99	de (Kruger)	0.364	ρg/μ (1/cm s)	9.9327E+04
0.177	3	0.03	96	de (Kozeny)	0.328	tau (Sauerbrei)	1.053
0.149	4	0.04	92	de (Zunker)	0.340	d _{geometric mean}	0.489
0.074	26	0.26	66	de (Zamarin)	0.352	σ _φ	1.449
				Io (Alyameni)	0.000		
				mm	0	% in sample	
				>64	Boulder		
				16 - 64	coarse gravel		
				8 - 16	medium gravel		
				2 - 8	fine gravel	0	
				0.5 - 2	coarse sand	0	
				0.25 - 0.5	medium sand	1	
				0.063 - 0.25	fine sand	33	
				0.016 - 0.063	coarse silt		
				0.008 - 0.016	medium silt		
				0.002 - 0.008	fine silt		
				<0.002	clay		



K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-13 from 5' to 15'Mass Sample (g): 100 T (oC) 20

Poorly sorted sandy silt low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.134E-03	.134E-05	0.12	
Hazen $K \text{ (cm/s)} = d_{10} \text{ (mm)}$.126E-03	.126E-05	0.11	
Slichter	.354E-04	.354E-06	0.03	
Terzaghi	.600E-04	.600E-06	0.05	
Beyer	.125E-03	.125E-05	0.11	
Sauerbrei	.126E-03	.126E-05	0.11	0.36
Kruger	.443E-01	.443E-03	38.28	
Kozeny-Carmen	.784E-01	.784E-03	67.77	
Zunker	.464E-01	.464E-03	40.11	131.55
Zamarin	.554E-01	.554E-03	47.89	
USBR	.767E-04	.767E-06	0.07	
Barr	.444E-04	.444E-06	0.04	0.13
Alyamani and Sen	.189E-05	.189E-07	0.00	0.01
Chapuis	.130E-04	.130E-06	0.01	
Krumbein and Monk	.267E-01	.267E-03	23.05	
geometric mean	.149E-03	.149E-05	0.13	0.42
arithmetic mean	.116E-01	.116E-03	10.06	33.01



Grain Size Analysis Report

Date:

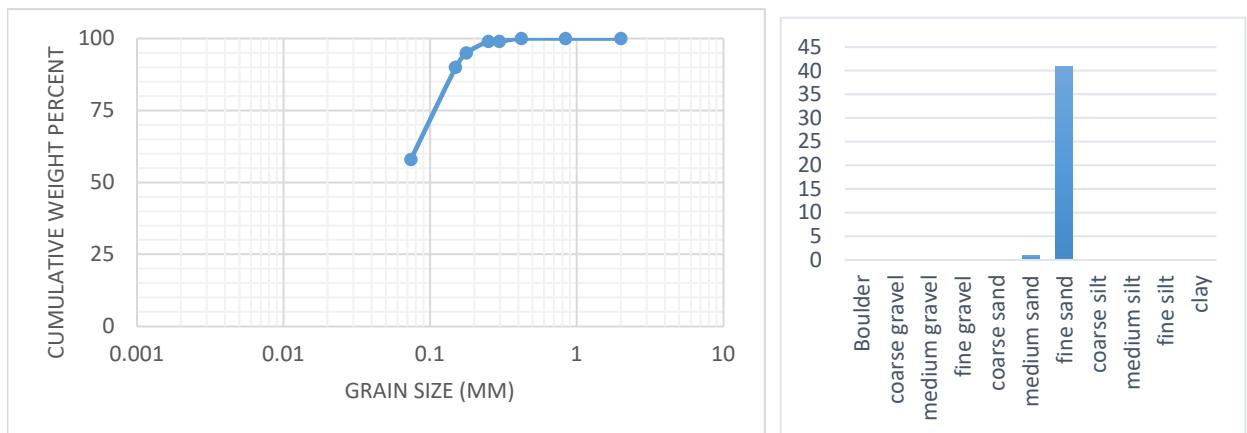
9/12/2022

Sample Name: B-13 from 15' to 20'

Mass Sample (g): 100

T (oC) 20

Poorly sorted sand low in fines



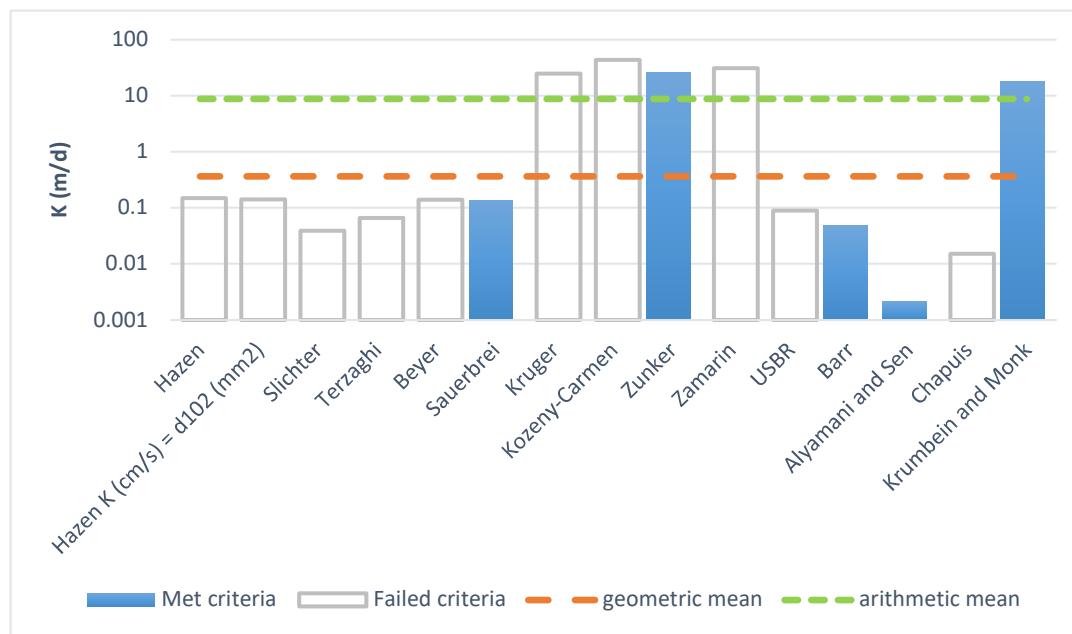
Sieve opening (ps) di (mm)	Mass of retained (mr) (g)	mass fraction (mf)	Percent Passing (pp)	Effective Grain Diameters (mm)		Other Useful Parameters	
				d10	0.013	Uniformity Coef.	6.09
2	0	0	100	d17	0.022	n computed	0.34
0.841	0	0	100	d20	0.026	g (cm/s ²)	980.00
0.42	0	0	100	d50	0.064	ρ (g/cm ³)	0.9981
0.297	1	0.01	99	d60	0.078	μ (g/cm s)	0.0098
0.25	0	0	99	de (Kruger)	0.295	ρg/μ (1/cm s)	9.9327E+04
0.177	4	0.04	95	de (Kozeny)	0.265	tau (Sauerbrei)	1.053
0.149	5	0.05	90	de (Zunker)	0.275	d _{geometric mean}	0.413
0.074	32	0.32	58	de (Zamarin)	0.285	σ _ϕ	1.393
				Io (Alyameni)	0.000		
				mm	0	% in sample	
				>64	Boulder		
				16 - 64	coarse gravel		
				8 - 16	medium gravel		
				2 - 8	fine gravel	0	
				0.5 - 2	coarse sand	0	
				0.25 - 0.5	medium sand	1	
				0.063 - 0.25	fine sand	41	
				0.016 - 0.063	coarse silt		
				0.008 - 0.016	medium silt		
				0.002 - 0.008	fine silt		
				<0.002	clay		



K from Grain Size Analysis Report

Date: 9/12/2022Sample Name: B-13 from 15' to 20'Mass Sample (g): 100 T (oC) 20

Poorly sorted sand low in fines

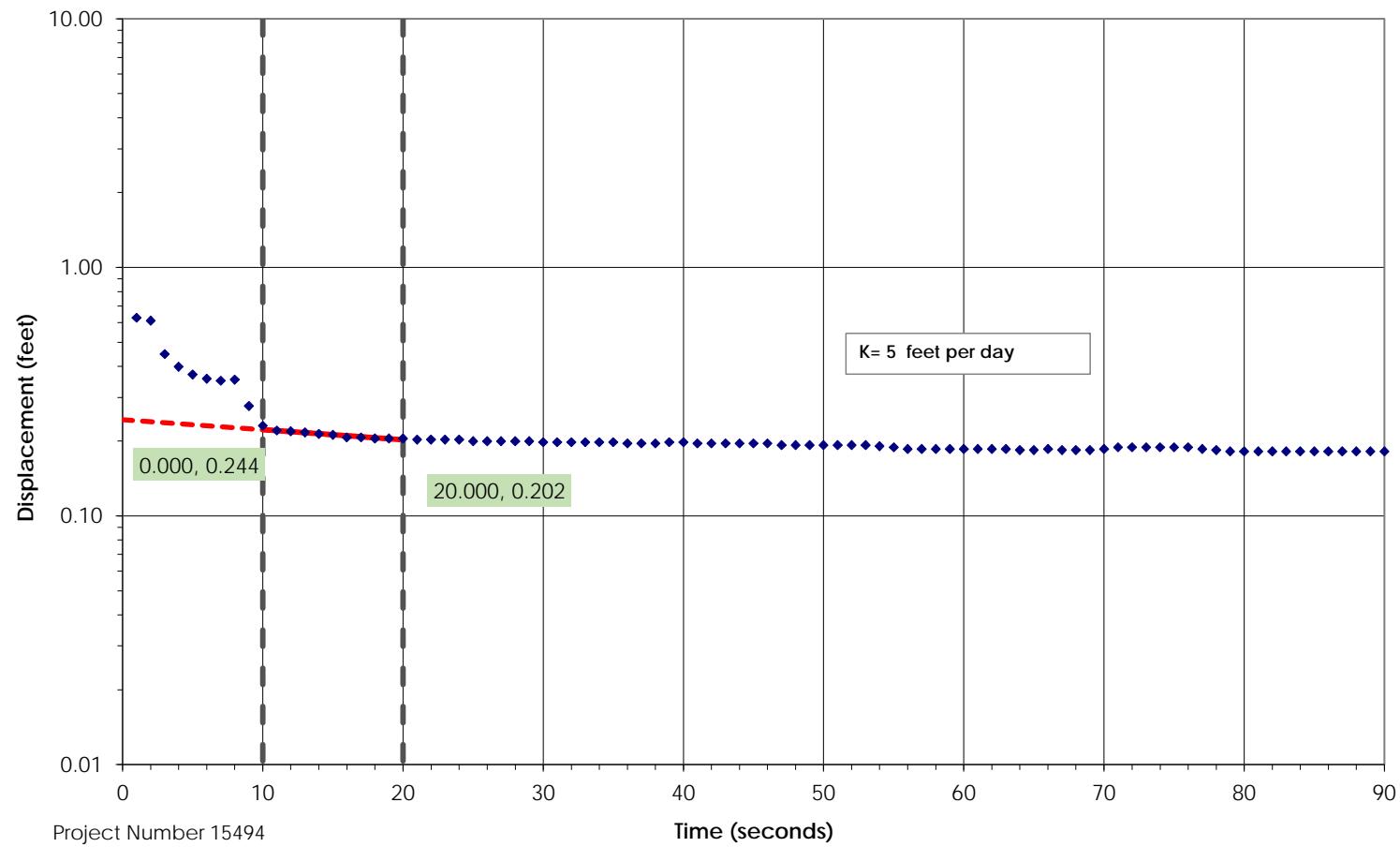


Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	ft/day
Hazen	.172E-03	.172E-05	0.15	
Hazen K (cm/s) = d_{10} (mm)	.163E-03	.163E-05	0.14	
Slichter	.453E-04	.453E-06	0.04	
Terzaghi	.766E-04	.766E-06	0.07	
Beyer	.161E-03	.161E-05	0.14	
Sauerbrei	.161E-03	.161E-05	0.14	0.46
Kruger	.288E-01	.288E-03	24.88	
Kozeny-Carmen	.506E-01	.506E-03	43.73	
Zunker	.301E-01	.301E-03	25.98	85.20
Zamarin	.359E-01	.359E-03	31.05	
USBR	.103E-03	.103E-05	0.09	
Barr	.566E-04	.566E-06	0.05	0.16
Alyamani and Sen	.245E-05	.245E-07	0.00	0.01
Chapuis	.175E-04	.175E-06	0.02	
Krumbein and Monk	.205E-01	.205E-03	17.67	57.97
geometric mean	.424E-03	.424E-05	0.37	1.20
arithmetic mean	.101E-01	.101E-03	8.77	28.76

ATTACHMENT 2

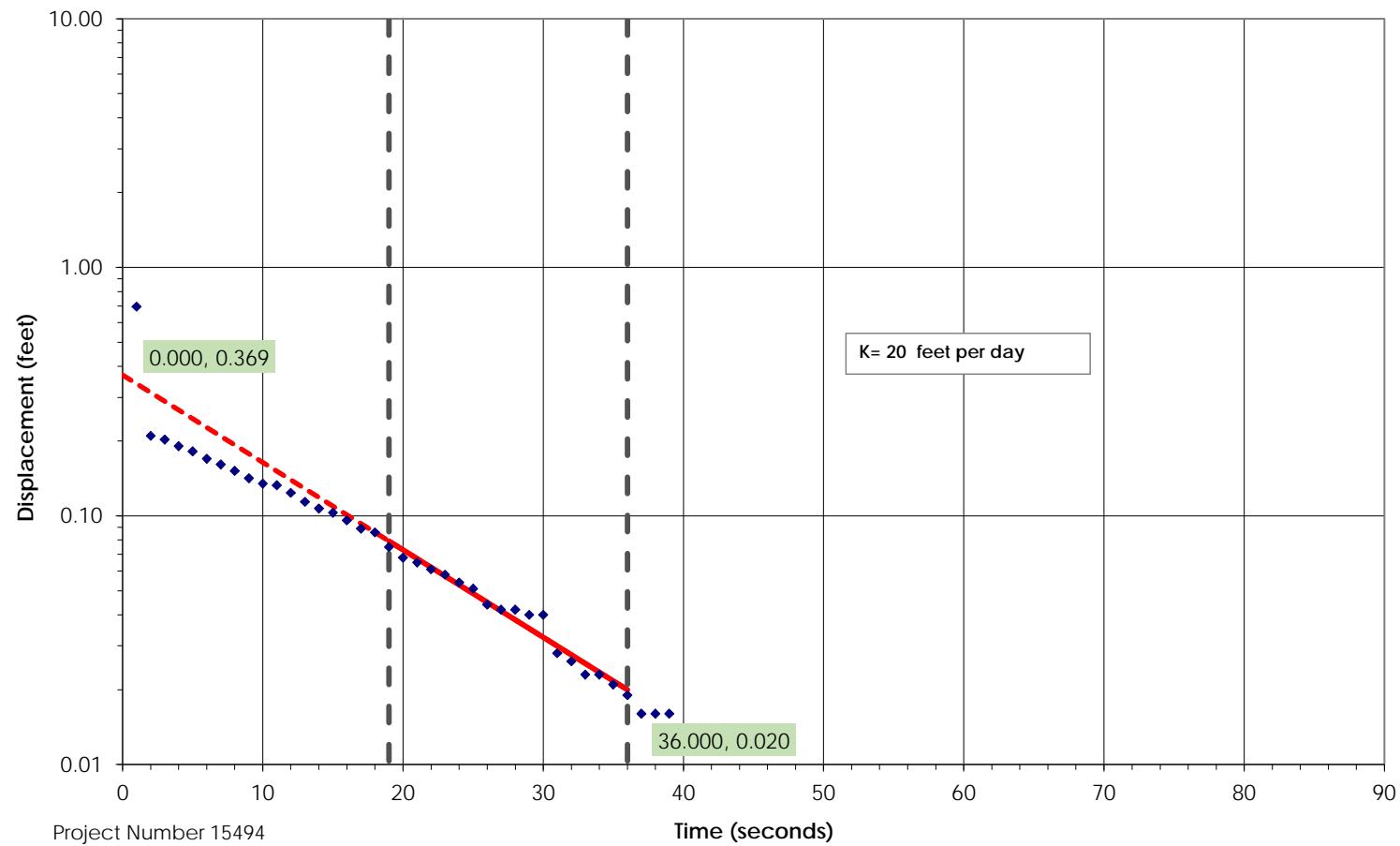
SLUG TEST ANALYSIS

MW-1 SLUG TEST ANALYSIS (Trial 1)
ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA



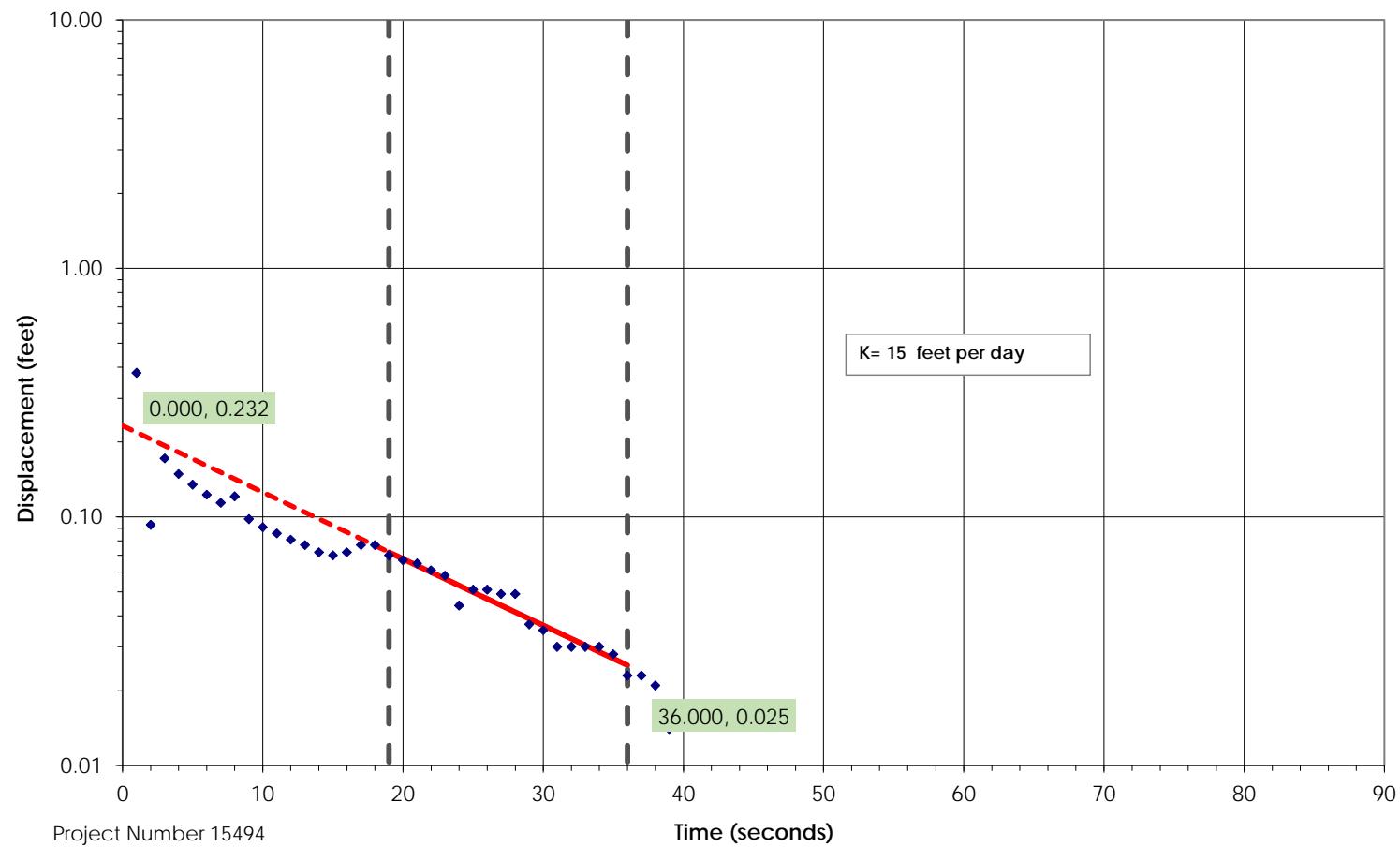
MW-2 SLUG TEST ANALYSIS (Trial 1)

ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA



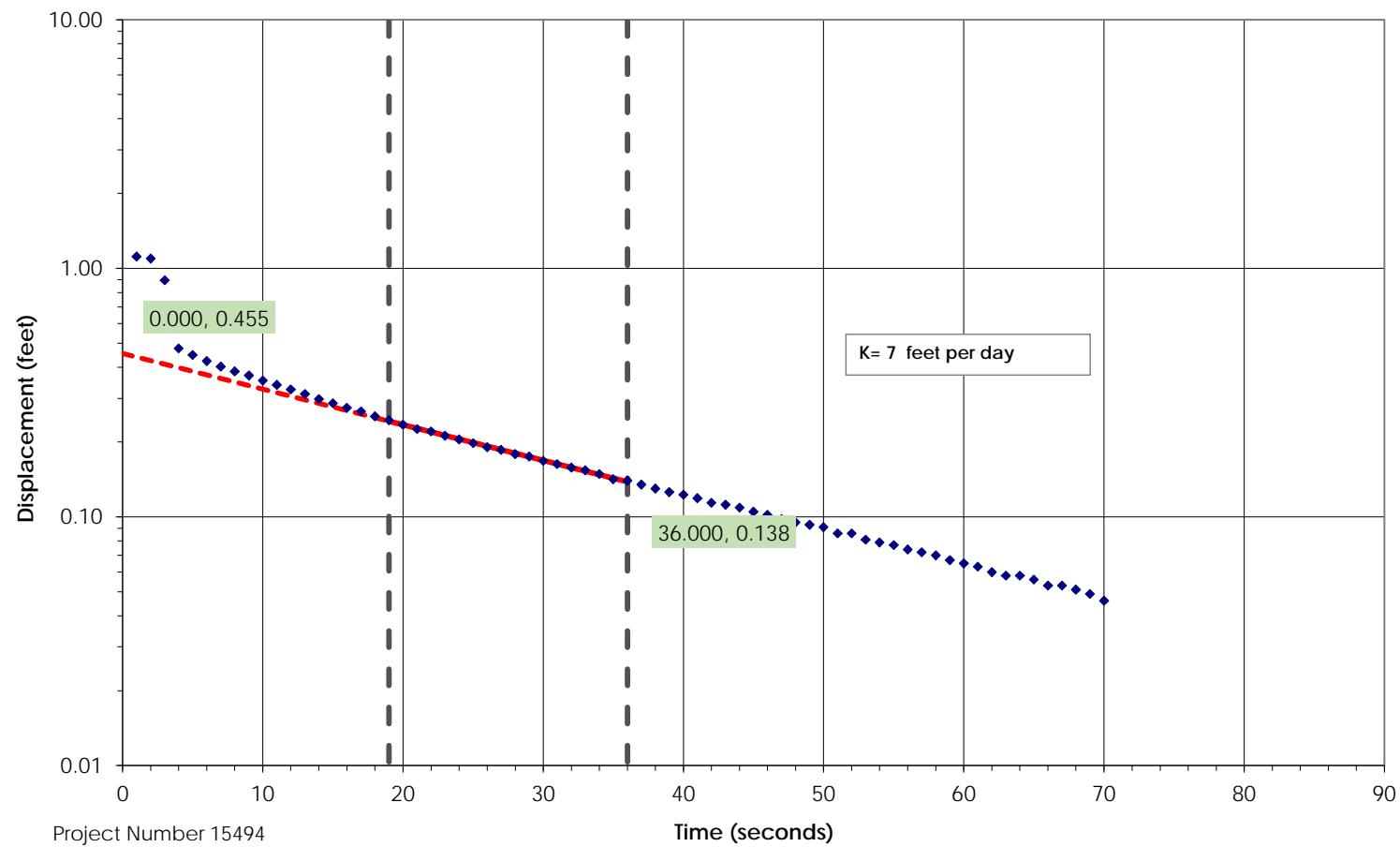
MW-2 SLUG TEST ANALYSIS (Trial 2)

ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA

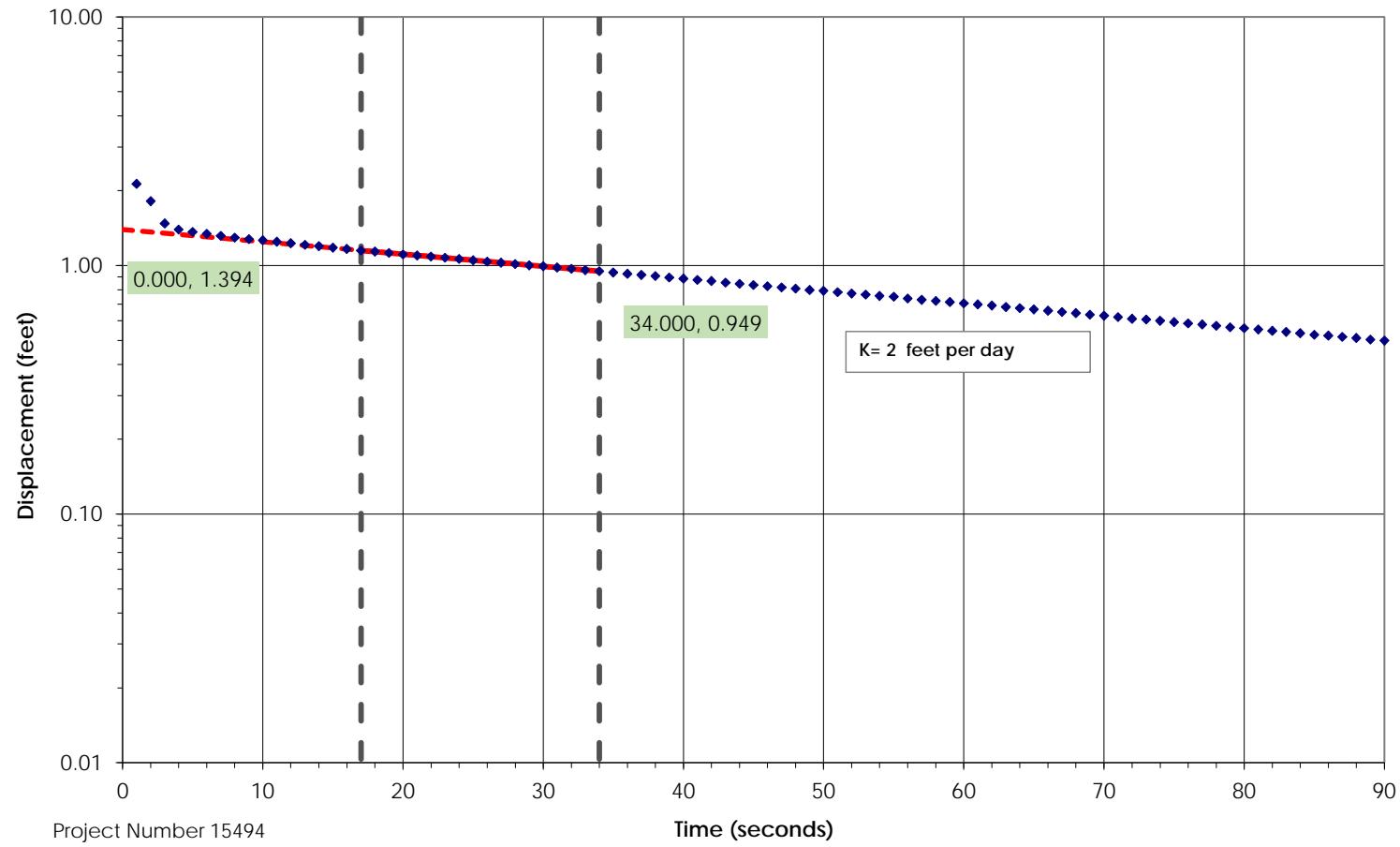


MW-3 SLUG TEST ANALYSIS (Trial 1)

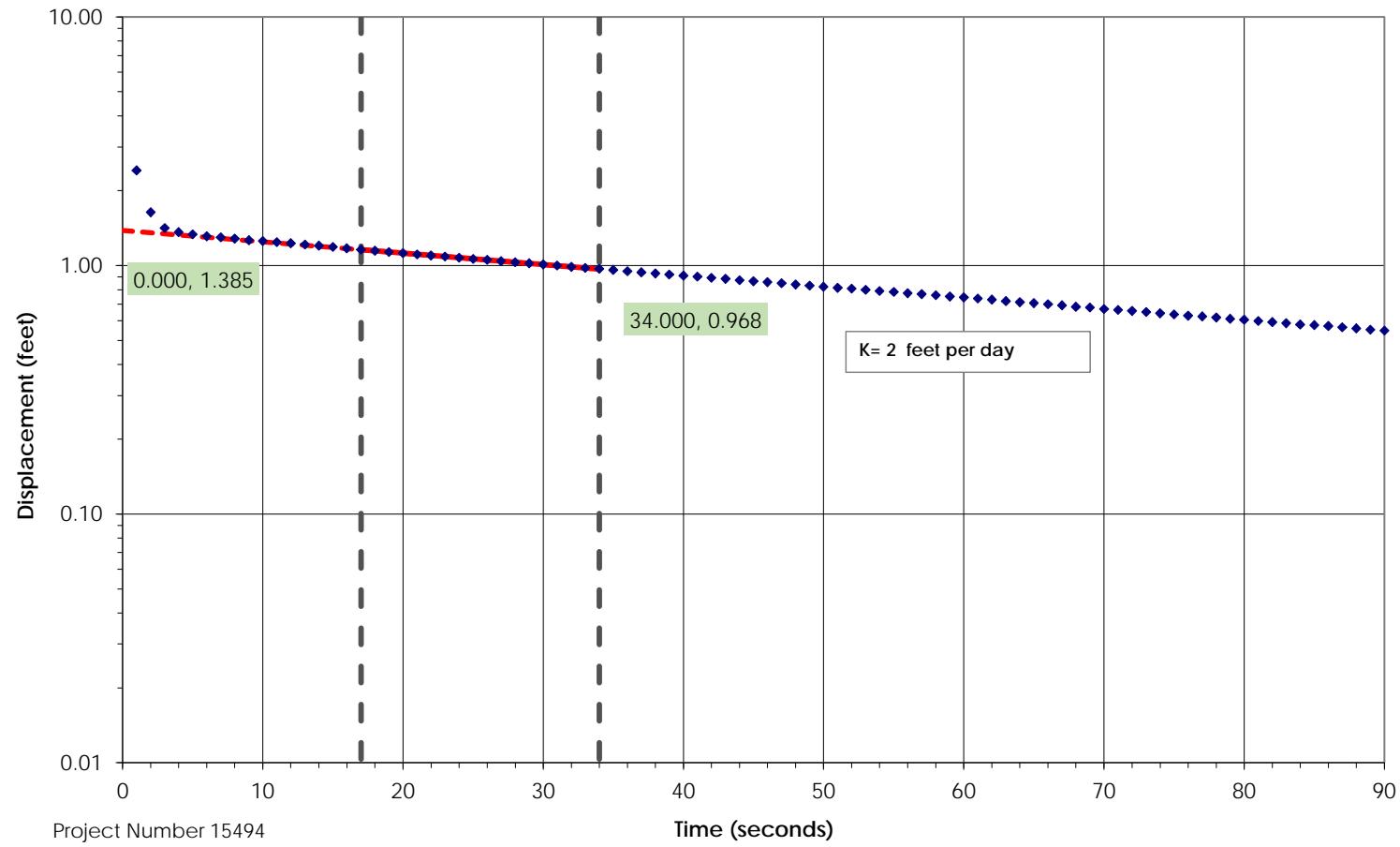
ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA



MW-5 SLUG TEST ANALYSIS (Trial 1)
ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA

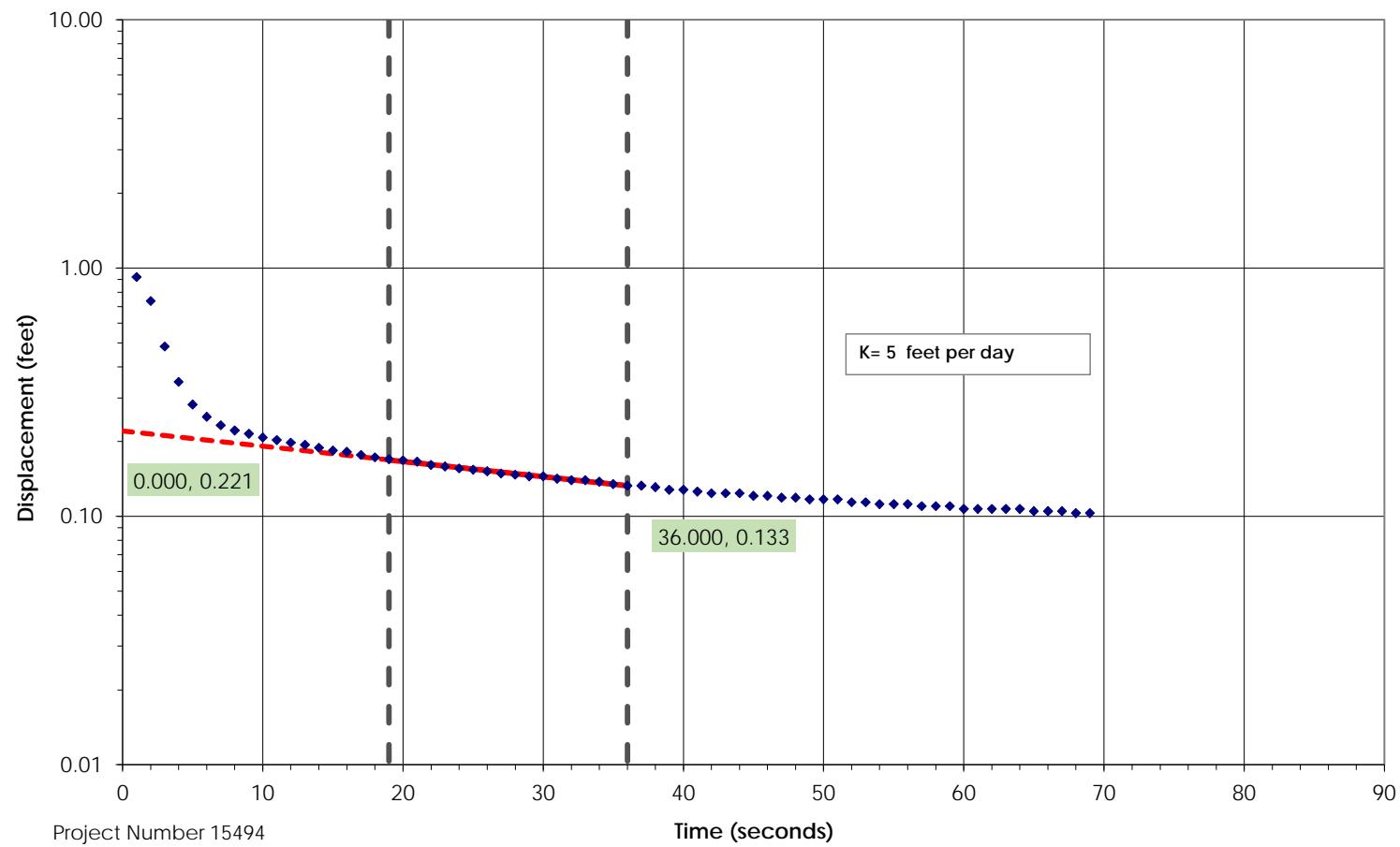


MW-5 SLUG TEST ANALYSIS (Trial 2)
ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA



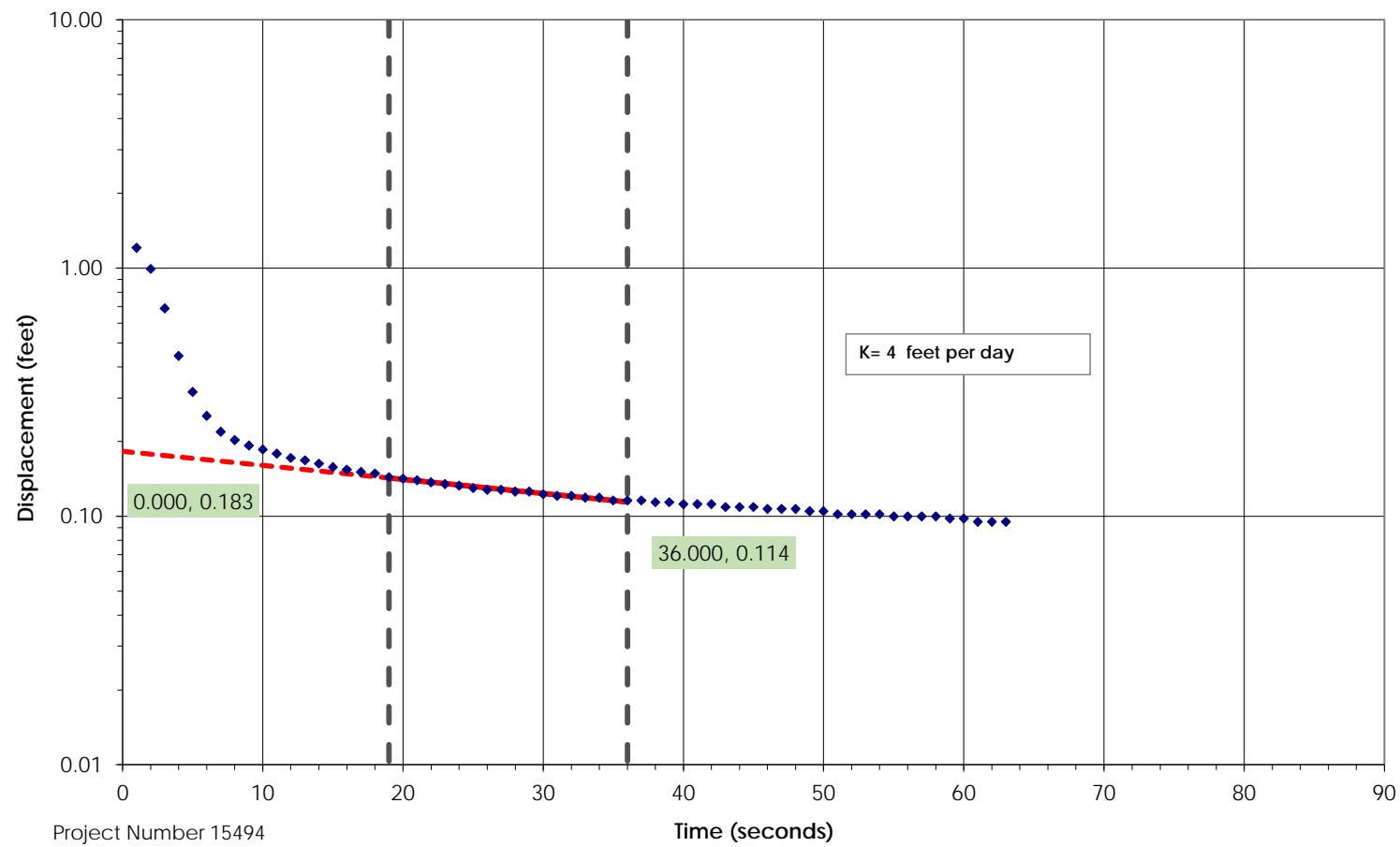
MW-7 SLUG TEST ANALYSIS (Trial 1)

ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA

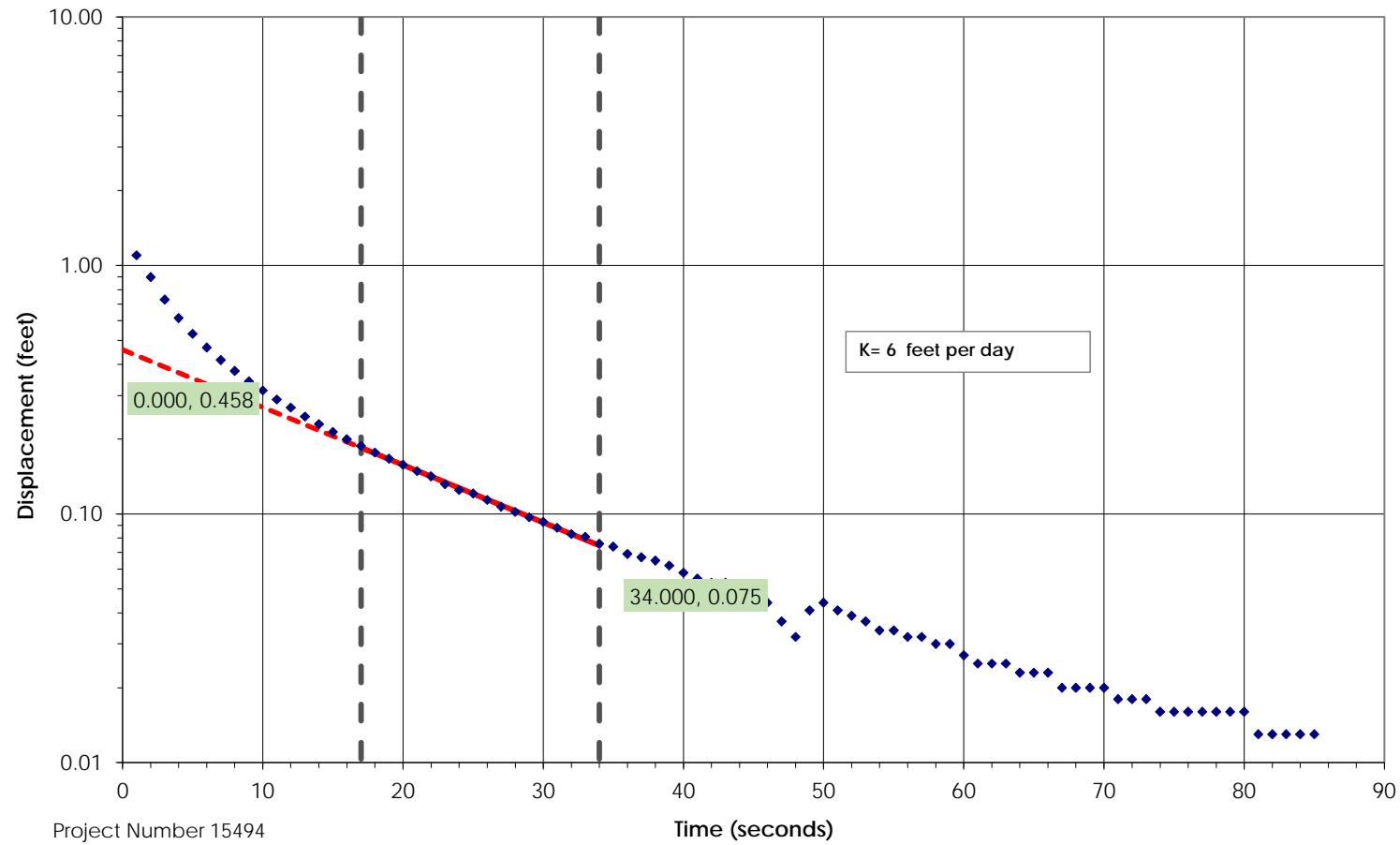


MW-7 SLUG TEST ANALYSIS (Trial 2)

ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA

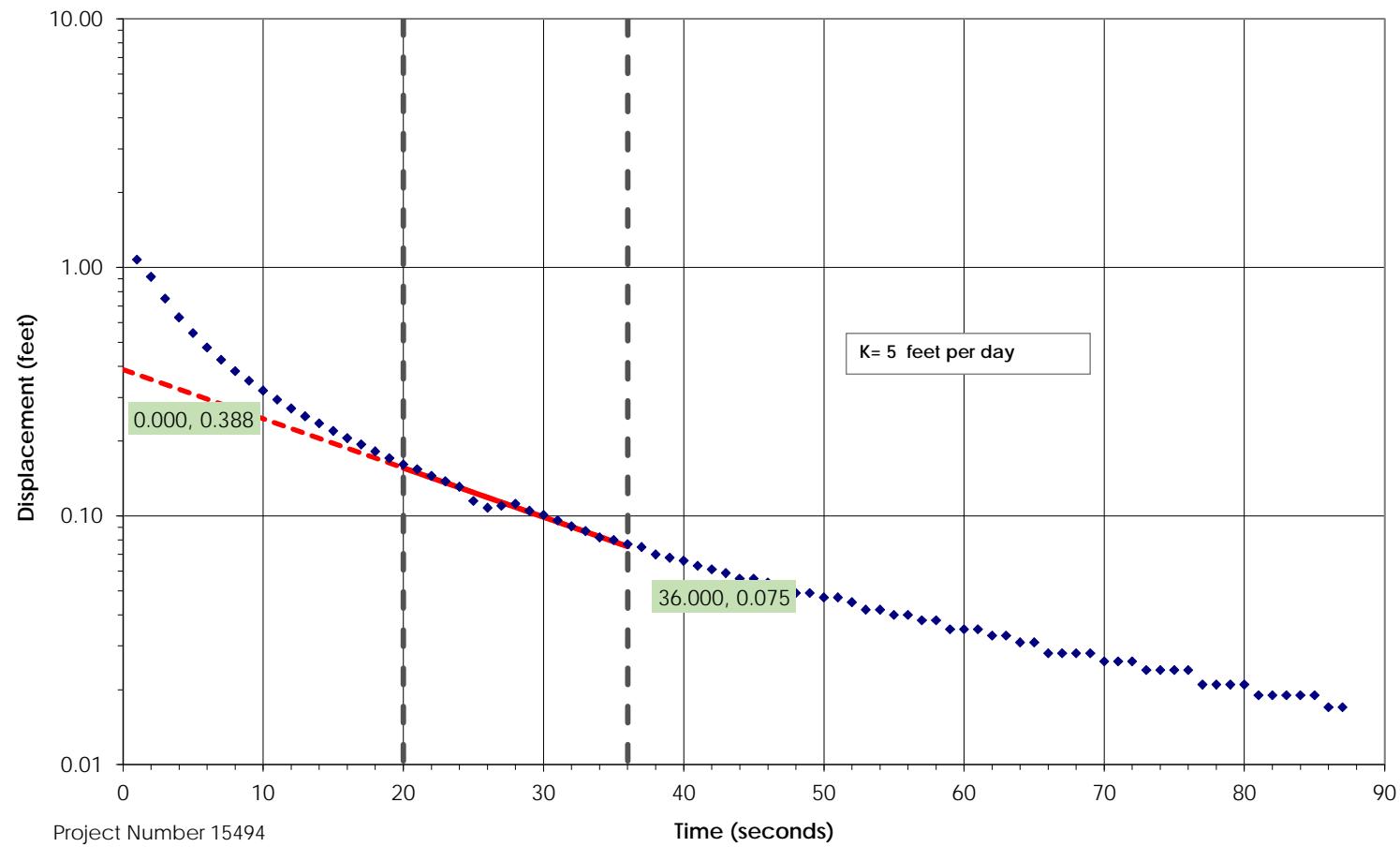


MW-8 SLUG TEST ANALYSIS (Trial 1)
ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA



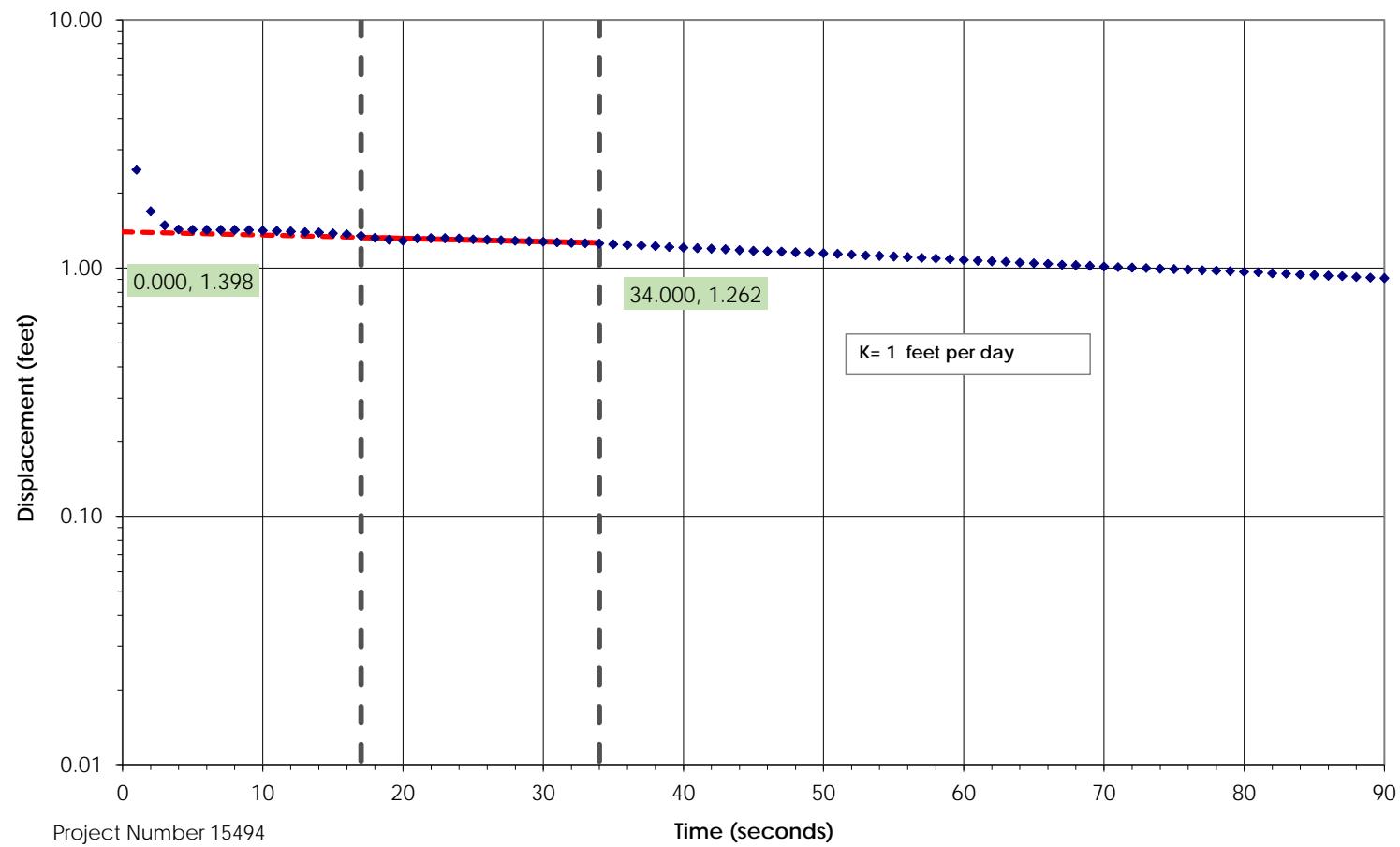
MW-8 SLUG TEST ANALYSIS (Trial 2)

ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA



USGS SLUG TEST ANALYSIS (Trial 1)

ADAMS SCHOOL WASTEWATER INVESTIGATION HOLLISTON, MA



APPENDIX C RAIN DATA

Rainfall Data Collected from Milford Weather Station

Day	Oct-22	Nov-22	Dec-22	Jan-23	Feb-23	Mar-23
1	0	0.14	0.73	0.38	0	0.23
2	0.06	0.03	0	0	0	0.28
3	0.01	0	0	0.01	0	0.09
4	0	0	0.35	0.7	0	0.48
5	0.9	0	0	0.33	0	0.18
6	0.68	0	0	0.04	0	0
7	0	0	0.61	0.18	0	0
8	0.02	0	0.3	0	0.04	0
9	0	0	0	0	0	0
10	0	0	0	0	0	0
11	0	0	0	0	0	0.16
12	0	0.68	0.09	0	0	0
13	0	0.15	0	0.79	0	0
14	0.81	0.21	0	0.07	0	1.67
15	0.77	0	0	0.03	0	1.08
16	0	0.67	0.53	0.09	0	0
17	0.02	0.15	1.34	0.1	0.02	0
18	0.83	0	0.05	0.05	0.11	0
19	0.08	0	0	0	0	0
20	0	0	0	1.24	0	0
21	0	0	0	0.19	0.07	0
22	0	0	0	0	0.07	0
23	0	0	1.7	0.91	0.6	0
24	0.09	0	0.77	0.38	0.14	0
25	0.16	0	0	0	0	0
26	0.42	0.03	0	1.4	0.05	0.16
27	0.07	0	0	0.03	0.01	0
28	0	0.77	0	0	0.18	0.5
29	0	0	0	0		0.07
30	0	0	0	0		0
31	0		0	0.06		0
Total	4.92	2.83	6.47	6.98	1.29	4.9
Cum. Total	4.92	7.75	14.22	21.2	22.49	27.39
Normal	4.98	3.96	4.73	3.73	3.27	
Cum. Normal	4.98	8.94	13.67	17.4	20.67	20.67

Figures showing early 2023 Rainfall and Ground Water Elevation

These figures show how the ground water elevations respond to rainfall. There is a significant rise in the elevation when there is heavy rainfall. The water level rises by about a foot and a half after a week with several rainy days in late January. There is a similar response in the ground water elevation from heavy rain on March 13th. There is a small time delay in this response, as the water continues to rise for a couple days after the rain stops. The water elevations proceed to drop during the periods of little rain, causing the elevation to fall from where it has risen to from the rain.

